



**Addendum #4  
Lakeland School Corporation  
Maintenance Building**

**Date:** May 05, 2026  
**Project:** Maintenance Building  
**Project #:** 25082  
**Pages:** 1 of 5 (8-1/2x11) pages  
**Bid Dates:** **THURSDAY, May 14th, 2026 at 11:30 am (prevailing local time)**

**General Note:**

The original Specifications and Drawings dated April 10, 2026 for the project referenced above are amended as noted in this Addendum No. 4. Receipt of this Addendum and any subsequent Addenda must be acknowledged on the Bid Form. Items changed or added by this addendum are to take precedence over the items or descriptions of the work in the project manual and the drawings. Items not mentioned in this addendum are to remain as described in the original plans and specifications.

There was a question regarding Form AIA305. You may omit that form from your submission. It is not required for this project.

**Clarification Items:**

***Owner Clarification: This building does not need to be connected to the Niagara Controls System.***

***Clarification from Addendum #3 – PEX tubing is acceptable in uses for domestic water piping.***

*Q: 13 34 19 1.01 – Section H. An alternate bid to add one more bay to the west is to be bid as an alternate. This expanded alternate bay is also to have an expandable end wall to allow for additional future expansions. \*Please clarify\**

***A: The Intent is for PEMB (west end wall) to NOT have ability the to add bays in the future. The section was to say EAST WALL – this was in the event the Alternate 1 ADD wash bay was not accepted. Lakeland would have the ability to add this in the future to the east.***

*Q: 01 23 00 1.04 A: ALTERNATE 1: “deduct wash bay” contradicts Bid form 96 p2 “alternate bid no. 1 – “Add wash bay”. \*Please clarify base bid\**

***A: Bid form 96 is accurate. Alternate 1 is to ADD wash bay scope.***

*Q: Does there need to be a railing on the sides of the stairwell leading to the mezzanine?*

***A: Yes railing on both sides.***

*Q: Wall types show 5/8” drywall on both sides but you also have those same wall types with note 9. So do you want both layers of 5/8” drywall and then the 5/8” plywood on top of the drywall? If so, does that then make those door jambs 5/8” wider.*

***A: Intent is for plywood 5/8” plywood to be on outside of the entire office and 5/8” gyp. bd. for the entire interior of the office.***



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*Q: Please provide wall details at mower lift in column line 4D.*

**A: No special details.**

*Q: Please confirm that the East Endwall is to be expandable as per specifications.*

**A: If alternate for wash bay is taken, there will be no need for expansion. If wash bay is not taken, we will want the East endwall to be expandable.**

*Q: Are value engineering numbers acceptable for volunteer alternates with proposals?*

**A: No**

*Q: PL-2 is called out in the list of material finishes, but I do not see where this is utilized in the drawings. Also in lieu of the Corian tops, would a 2cm thick quartz be acceptable?*

**A: yes, quartz is an acceptable substitute.**

*Q: Is fire alarm required?*

**A: No**

*Q: Is a Knox Box Required?*

**A: Yes, it will be Owner Provided, Contractor Installed.**

*Q: Provide Clarification for Resinous Epoxy Flooring.*

**A: Please see attached Addendum #4 Document for cutsheet on Resinous Flooring Product Specified on Finish Schedule Sheet A11-1. The chip blend will be a blend of Lakeland's colors selected from manufactures' standard list of colors.**

**01 23 00 – Alternates: See attached spec section for language updates on alternates.**

**00 41 00 – Bid Form 96 – Alternates language updated to match alternates spec section.**

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**Drawing Set Items:**

**Sheet C1-010, Sheet C1-020, Sheet C1-100, Sheet C2-100:**

1. Replace with entire sheets.
2. Only changes are relative to Bid Alternate No. 5 – Removing Remaining Berm

**Sheet S0-1 Structural General Notes**

1. Add this sheet in entirety. See new sheet attached to this addendum.

**Sheet S1-0 Foundation Plan**

1. Replace with entire sheet.
2. Add notes: FOUNDATION GENERAL NOTES
3. Add 6/S1-0 detail: TYP SLEEVE DETAIL AT WALL FOOTING
4. Add detail: SLAB ON GRADE CONSTRUCTION
5. Add detail: TYPICAL SLAB JOINTS



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6. Changed WF-2 to WF-1 on plan.
7. Remove WF-2.
8. Add details for mechanical enclosure footings.
9. Added thickened slab for bearing walls.
10. Wash bay alternate – change slope to ¼” per 12” min. slope
11. Replace note 3, to read: TYPICAL SAWED IN CONTROL JOINT, TYPICAL, PER CONTROL JOINT DETAIL ON THIS SHEET. FILL WITH SEALANT.
12. Add note 11 – Add (2) coats of sealer to wash bay slab only.

**Sheet S1-1 Framing Plan**

1. Replace with entire sheet.
2. Changed TJs and bearing wall layout.
3. Changed stair system.
4. Mezzanine design loads: 125 PSF Live Load and 10 PSF Dead Load.

**Sheet A1-1 Architectural Floor Plan**

1. Add detail section cut to interior window.
2. Change note 3.
3. Change note 10.
4. Change note 7.
5. Add “BRACED” to note 13.
6. Changed mezzanine loading access to 5’-0” wide gates (2).
7. Added details for mechanical enclosure.
8. Added detail for interior window.
9. Remove “PAINT” from note 6.
10. Add note 14 for non-occupied space.
11. Insulate Restroom 22 and Meeting room 21 with acoustic batts. The office perimeter PEMB walls will be the contractor's choice for furring on girts and 5/8” gyp. All exterior walls are to be insulated per PEMB details. The entire mezzanine ceiling is to be insulated.
12. Add Note 15 for “STAINLESS STEEL 2 BASIN STANDALONE SINK. SEE MEP SHEETS FOR PLUMBING COORDINATION”
13. All office area is to have 5/8” gyp. bd. up 4” above ACT.
14. Intent is for surface mounted fire extinguishers on wall without a cabinet.

**Sheet A2-1 Reflected Ceiling Plan**

1. ACT basis of design tile: 2x2x3/4” USG Frost Basic (NRC .55, CAC 35, White, SLB Profile)

**Sheet A3-1 Roof Plan**

2. Removed general note C.

**Sheet A4-1 Exterior Elevations**



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1. Changed window above mechanical enclosure to be AF-04.
2. Added note 16 “NON-OPERABLE ALUMINUM FRAME STOREFRONT W/ INSULATED GLAZING. 9'-0" W x 2'-6" H” and labeled windows.
3. Added note 17 “ NON-OPERABLE ALUMINUM FRAME STOREFRONT W/ INSULATED GLAZING. 3'-0" W x 2'-6" H and labeled windows.
4. All exterior metal wall panels to be Nucor Reverse R panel. Both the light color and blue color will be non-custom colors, selected from the metal suppliers list of color selections.
5. L Graphic – Specs provided for High Performance Exterior Vinyl. Similar Grade Product can be provided as a substitute. Architect will provide Vector file for signage subcontractor.

**Sheet A6-1 Wall Sections & Details**

1. Add detail 5 for mechanical enclosure.
2. All exterior metal wall panels to be Nucor Reverse R panel. Both the light color and blue color will be non-custom colors, selected from the metal suppliers list of color selections.
3. Exterior bump out on upper perimeter walls: Contractor’s choice for using PEMB components or plywood and lumber for construction. Dimensions and design must remain the same regardless of construction methods.
4. All Treated Blocking used to support exterior metal panels should be KDAT treated lumber.

**Sheet A6-2 Wall Sections & Details**

1. Add liner panel to ceiling as shown on RCP.
2. Liner panel on walls to go to ceiling.
3. Remove “J” trim and add trim for liner panel joint at ceiling.
4. All exterior metal wall panels to be Nucor Reverse R panel. Both the light color and blue color will be non-custom colors, selected from the metal suppliers list of color selections.

**Sheet A8-1 Door/Frame Schedules**

1. Change Door 01 to be Alum.
2. Remove translucent windows in all OHVD doors.
3. Changed AF-03 to be 9'-0" W x 2'-6" tall with IGU-1.
4. All Alum. frames to be clear anodized.
5. Intent is for glazing to be clear insulated glazing units.

**Sheet A9-1 Enlarged Floor Plan & Details**

6. Add interior window sill detail to sheet.
7. Changed details 10 and 11.



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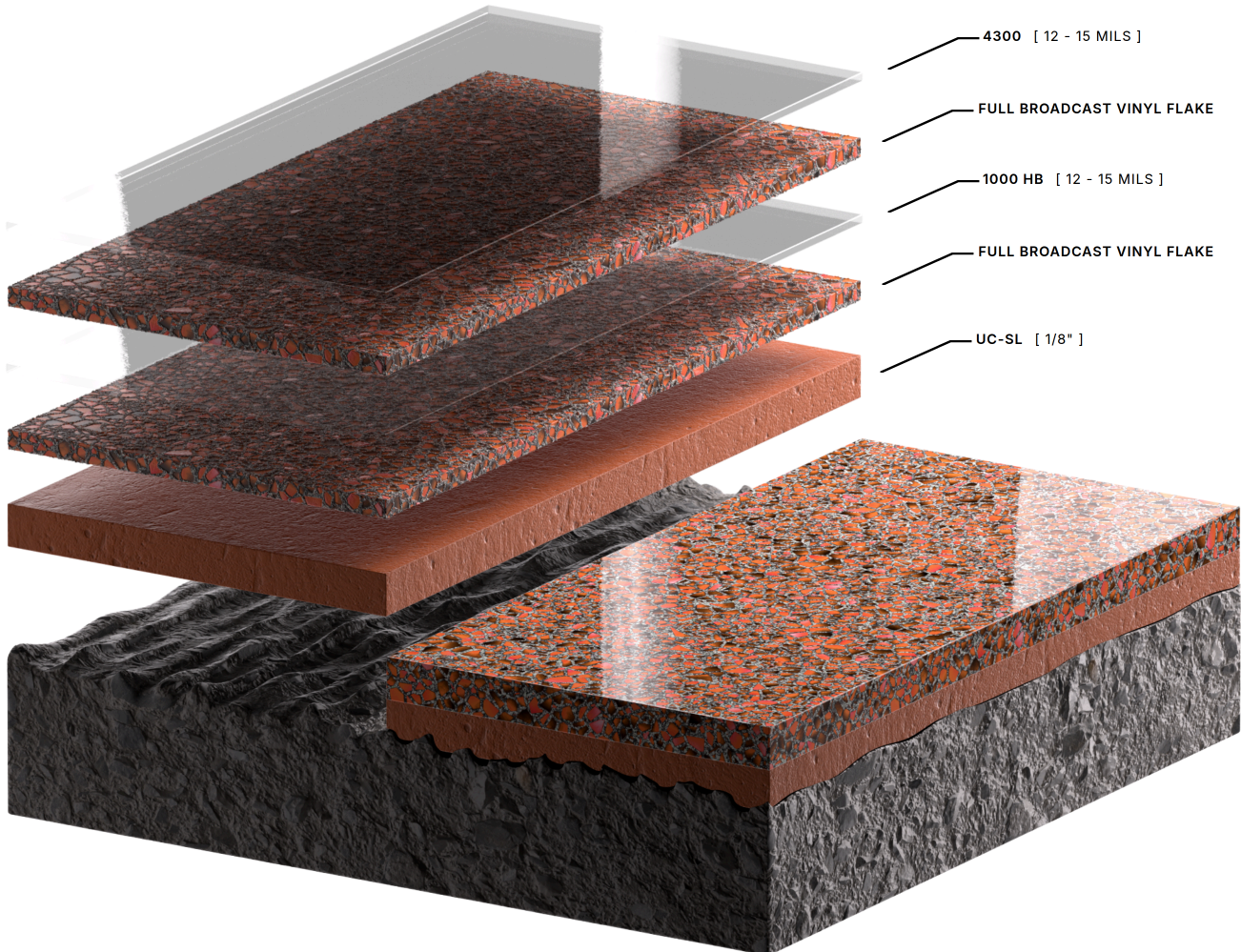
**Attachments:**

1. Resinous Flooring Cutsheet
2. Geotech Report – Refer to Site 1 – Lakeland Int. School. See pg 29 / boring B-1 to B-5.
3. Spec Section – 00 41 00 – Bid Form 96
4. Spec Section – 01 23 00 – Alternates
5. C1-010 – Erosion Control Plan
6. C1-020 – Site Demolition Plan
7. C1-100 – Site Layout Overall Plan
8. C2-100 – Site Grading and Drainage Plan
9. S0-1
10. S1-0
11. S1-1
12. A1-1
13. A3-1
14. A4-1
15. A6-1
16. A6-2
17. A9-1

END

# Protect SL-245

High-Performance Slurry Flooring System



## FINISH DESIGN

SL-245 displays a decorative flake film with a smooth textured finish with a UV stable gloss appearance. The displayed aggregate can create a CoF up to .8 (ASTM D2047)

## SYSTEM STRUCTURE

SL-245 relies on the stratification of aggregate to push excess resin to the surface. The available resin forms a textured topping that encapsulates the flake aggregate. The addition of an additional encapsulated flake layer greatly and improves physical properties.

## BOND APPROACH

SL-245 allows mechanical surface profiling to form a complete mechanical bond. This allowable profile combined with resin resilience resists damage from elevated physical and thermal exposure.

## SPECIALTY PERFORMANCE

SL-245 does not provide any specialty static discharge or electrical performance. The system does not offer any active antimicrobial function but systems are available with 3rd party verified, fully antimicrobial properties.



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## GEOTECHNICAL REPORT

### PROPOSED IMPROVEMENTS AT SEVERAL EXISTING LAKELAND SCHOOL CORPORATION SCHOOL SITES

Various Locations,  
LaGrange, IN

**GME TESTING PROJECT NO.  
G26-033379**

PREPARED FOR:

**krM Architecture**  
1515 North Pennsylvania Street  
Indianapolis, IN 46202  
**Attn: Kyle Keaffaber, AIA, Associate Principal**

April 24, 2026

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April 24, 2026  
G26-033379

**krM Architecture**  
1515 North Pennsylvania Street  
Indianapolis, IN 46202  
**Attn: Kyle Keaffaber, AIA, Associate Principal**

**REF: SUBSURFACE EXPLORATION AND RECOMMENDATIONS**  
**Proposed Improvements at Several Existing**  
**Lakeland School Corporation School Sites**  
Various Locations,  
LaGrange, IN

Dear Mr. Keaffaber:

In compliance with your request and authorization, **GME Testing** is pleased to submit this report on our subsurface exploration and recommendations for the above-referenced project. Our services were performed according to our proposal (GMEP 26-030067) dated March 16, 2026.

We appreciate the opportunity to be of service on this project. Should you have any questions related to this report, please contact us at your convenience.

Sincerely,  
**GME Testing**

S M Naziur Mahmud, P.E.

Khaled AlAkhras, PhD, PE



Rami M. Anabtawi, P.E., BC.GE

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**APPENDIX A**

I. Field Exploration, II. Laboratory Testing, Exhibit A - Site Vicinity and Boring Location Map, Exhibit B - Undercut Excavation Below Footings, Exhibit C - Placement of Adjacent Footings

**APPENDIX B**

Boring Logs, General Notes, ASCE Design Hazards Report, Custom Soil Resource Report

## 1.0 INTRODUCTION

### 1.1 Project Location

This report presents the results of our geotechnical investigation for the proposed project, which consists of various improvements at several existing Lakeland School Corporation school sites in LaGrange, Indiana. The planned locations include:

- **Site 1:** Lakeland Intermediate School
- **Site 2:** Lakeland Junior/Senior High School
- **Site 3:** Wolcott Mills Elementary School
- **Site 4:** Lakeland Primary School

### 1.2 Scope of Geotechnical Investigation

The general scope of this investigation was to evaluate the existing subsurface conditions encountered in the borings and develop geotechnical recommendations for use by the owner's design engineers and architects in preparing the design plans for the proposed construction.

Twelve (12) vertical soil test borings were requested and drilled at the locations provided in this report to evaluate subsurface conditions at the different sites. Table 1 summarizes the designated boring areas.

<b>Table 1: Summary of Borings</b>		
<b>Site Designation</b>	<b>Proposed Construction</b>	<b>Boring Number</b>
Site 1: Lakeland Intermediate School	Potential Borrow (Existing Berm Area)	B-01
	Outside Building (Grass Area)	B-02
	Maintenance Barn	B-03 through B-05
Site 2: Lakeland Junior/Senior High School	Building Addition	B-06, B-08, B-09
	Outside Building (Grass Area)	B-07

Table 1: Summary of Borings		
Site Designation	Proposed Construction	Boring Number
Site 3: Wolcott Mills Elementary School	Playground	B-12
Site 4: Lakeland Primary School	Playground	B-10 and B-11

### 1.3 Field Coordination

GME Testing coordinated their fieldwork logistics, site access, utilities marking, and the geotechnical drilling program schedule with the client representative to conduct this geotechnical engineering investigation.

### 1.4 Site Characteristics

#### 1.4.1 Site Conditions

The project sites consist of existing developed school properties owned by Lakeland School Corporation.

Each site is currently in active use and includes a combination of school buildings, parking areas, athletic fields, playgrounds, landscaped areas, sidewalks, and associated site infrastructure. Existing overhead and underground utilities are also present at each site, consistent with developed school campus conditions.

The proposed improvements will be constructed within these developed areas and are expected to require only limited site disturbance. Construction activities will generally involve localized grading, minor utility adjustments as needed, and integration with existing site drainage patterns.

Figure 2 provides selected photographs representative of the site at the time of this investigation.



*Selected Photographs of the Site 1: Lakeland Intermediate School*



*Selected Photographs of the Site 2: Lakeland Junior/Senior High School*



*Selected Photographs of the Site 3: Wolcott Mills Elementary School*



*Selected Photographs of the Site 4: Lakeland Primary School*

**Figure 2 – Selected Photographs of the Site**

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### **1.4.2 Site Soil Survey**

According to the USDA Web Soil Survey, the sites are mapped as follows:

- **Site 1:** Conover loam, Hillsdale sandy loam, and Oshtemo loamy sand
- **Site 2:** Oshtemo loamy sand
- **Site 3:** Boyer loamy sand and Udorthents, loamy
- **Site 4:** Hillsdale sandy loam, Oshtemo–Hillsdale–Chelsea complex, and Rensselaer loam

A copy of the USDA Web Soil Survey *Custom Soil Resource Report for LaGrange County, Indiana*, is included in Appendix B of this report for reference.

### **1.5 Project Details**

Our proposal outlined our initial understanding of the project based on information and a conceptual site plan provided by Mr. Kyle Keaffaber, AIA, Associate Principal with krM Architecture. We understand that the proposed project will consist of the following:

**a) Site 1: Lakeland Intermediate School (Borings B-01 through B-05)**

The proposed project at Lakeland Intermediate School consists of a maintenance barn incorporating office space, storage areas, and a wash bay. The structure is planned to have an approximate first-floor footprint of 9,730 square feet, with an additional mezzanine level of approximately 1,542 square feet, for a total building area of about 11,272 square feet. The structure height is anticipated to be approximately 16 feet.

Boring B-01, located within the existing berm area, may represent a potential on-site borrow source; however, details regarding its use were not available at the time of this report. Grading plans for the pavement and adjacent areas were also not available.

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Parking and driveway areas are planned to the west of the proposed maintenance building. It is anticipated that these areas will accommodate passenger vehicles and occasional heavy truck traffic. The client indicated that the parking areas will generally consist of standard-duty asphalt pavement sections with curbs and typical details for passenger vehicles, with localized heavy-duty asphalt sections incorporated where needed. The proposed access drive is anticipated to consist of a heavy-duty asphalt pavement section.

Based on the topographic survey prepared by ForeSight Consulting, LLC (dated January 26, 2026), the proposed finished floor elevation is approximately 930 feet. Relative to existing site grades, the project will require general fill placement and site grading, with estimated grade raises ranging from approximately 1 to 5 feet across the site.

**b) Site 2: Lakeland Junior/Senior High School (Borings B-06 through B-09)**

The proposed project at Lakeland Junior/Senior High School consists of building additions to be constructed immediately south of the existing building. The additions are anticipated to be slab-on-grade with no basement and will consist of a combination of masonry and steel-frame construction similar to the existing building. As-built plans for the existing building are not available at this time. The proposed additions are anticipated to be structurally connected to the existing building, as determined by the project's structural engineer, and architectural and/or structural modifications may be required to accommodate new doorways or entrances/exits.

According to the topographic survey prepared by ForeSight Consulting, LLC (dated January 26, 2026), the finished floor elevation of the existing building is approximately 909 feet. For preliminary planning purposes, the finished floor elevation of the proposed additions is anticipated to match the existing building. Based on current site grades, approximately 1 to 2 feet or less of grade-raise fill is expected above the existing ground surface.

**c) Site 3: Wolcott Mills Elementary School (Boring B-12) and Site 4:  
Lakeland Primary School (Borings B-10 and B-11)**

The proposed improvements at Wolcott Mills Elementary School and Lakeland Primary School are anticipated to include the development of typical playground facilities consisting of at-grade play structures and impact-attenuating surfacing systems commonly used for playground applications.

In addition to playground facilities, the project is expected to include the construction of an asphalt-paved basketball court along with associated site improvements. These improvements may include sidewalks, accessible pedestrian routes, and localized grading modifications required to accommodate proposed equipment, facilitate site drainage, and integrate the improvements with existing site conditions.

Playground equipment is anticipated to be supported on shallow foundation systems such as embedded post systems or concrete piers designed to provide stability for the installed structures. No building foundations are anticipated as part of the proposed playground, basketball court, or associated site improvements.

**General Project Information**

Structural loading information and building footprints for Lakeland Intermediate School and Lakeland Junior/Senior High School were requested but were not available at the time of this evaluation. For preliminary geotechnical evaluation only, we assumed loading conditions include column loads of approximately 100 kips, wall loads of approximately 4 kips per linear foot, and floor slab loads of approximately 125 psf.

It is anticipated that the floor slabs will be “floating,” meaning they will be isolated from the footings except in areas intentionally thickened to support interior load-bearing elements.

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The structural engineer will be responsible for the final foundation design and detailing of the floor slabs, including slab thickness, reinforcement, and jointing, to ensure adequate performance under the intended building use.

GME Testing should be contacted to review any design information that differs from the assumptions presented herein. Based on that review, we will evaluate whether modifications to this report or its recommendations are warranted.

## **2.0 SUBSURFACE INVESTIGATION AND LABORATORY TESTING**

Twelve (12) vertical soil test borings were drilled to depths of approximately 15 feet (for playground areas) to 25 feet (other borings) below the existing ground surface (bgs), as shown on the individual borehole logs. GME Testing personnel established boring locations in the field based on the site plans provided to us by the client (see Exhibit A.1 through A.4).

Our subsurface exploration was performed in accordance with the Standard Penetration Test, ASTM D-1586. Representative soils collected using a split spoon sampler. Our laboratory testing program included performing **a)** visual soil classifications according to ASTM D-2487 and ASTM D-2488, **b)** natural moisture content tests according to ASTM D-2216 on all samples, and **c)** unconfined compressive strength tests with calibrated spring hand penetration tests in general accordance with ASTM D-2166.

### **Boring Results**

This overview reflects the Site's geotechnical conditions based on our field investigation and laboratory testing. Detailed descriptions and data for each test boring are available in the Borehole Logs in Appendix B. Conditions not represented at boring locations may be encountered during construction and could affect the project.

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The ground surface at the site was generally covered with approximately 5 to 18 inches of topsoil, and occasional mulch and pea gravel.

**Fill Materials:** Fill materials were encountered in select borings (B-01, B-07, B-08, B-09, and B-10) underlying the topsoil. These materials generally consist of brown to dark brown clayey sand, silty sand, and fine clayey silty sand with trace fine gravel. Fill thicknesses ranged from approximately 2 to 5 feet below ground surface. The lateral extent and depth of existing fill materials will vary and will not be known until the time of construction.

**Native Soils:** Underlying the fill and/or topsoil, native soils encountered in the borings generally consist of sandy silty clay, silty clay, clayey sand, silty sand, and silt. These soils are typically brown to gray, moist to very moist, and locally contain trace fine gravel and occasional sand or silt seams.

In several borings (e.g., B-01, B-02, B-05, and B-06), sandy silty clay extends to depths ranging from approximately 10 to 15 feet below ground surface, underlain by silty clay and/or granular soils.

Granular soils consisting of fine to medium coarse sand, silty sand, and sand and gravel mixtures were encountered in multiple borings (including B-01, B-02, B-08, and B-09) at depths generally below approximately 10 to 15 feet, extending to the explored depths of approximately 25 feet. These materials are typically moist to wet and may exhibit variable density.

Localized silt layers and clayey silt or sandy silt zones were also encountered in borings such as B-04, B-07, and B-08.

The **General Notes** sheet, which follows the boring logs included in Appendix B, explains the consistency and relative density correlations.

---

## Groundwater Measurements

Short-term groundwater measurements were taken during our field operations by noting the depth of water on the rods and in open boreholes following the withdrawal of the drilling augers after the completion of drilling activities in test borings.

In summary, groundwater was encountered during or following our drilling program in most of the borings, as shown in Table 2.

Table 2: Groundwater Depths in the Borings at Time of Drilling			
Site Designation	Boring Number	*Groundwater Depth, feet (Elevation, feet)	
		During Drilling	At the Completion of Drilling
Site 1: Lakeland Intermediate School	B-01	±21 (±900)	±15 (±906)
	B-02	±8 (±920)	±7 (±921)
	B-03	±3.5 (±926.5)	±19 (±911)
	B-04	±3 (±926)	±3 (±926)
	B-05	±3.5 (±922)	±7 (±918.5)
Site 2: Lakeland Junior/Senior High School	B-06	±13.5 (±895.5)	±15 (±894)
	B-07	±10 (±898)	±16.5 (±891.5)
	B-08	±10.5 (±898)	±17.5 (±891)
	B-09	±18.5 (±890.5)	±18.5 (±890.5)
Site 4: Lakeland Primary School	B-10	† NO	NO
	B-11	NO	NO
Site 3: Wolcott Mills Elementary School	B-12	±6 (±924)	±5 (±925)

\*Depths referenced below the existing ground surface.

†Not Observed (NO)

The groundwater depths shown on the boring logs reflect groundwater levels **only** for the date on which the borings were drilled. Fluctuations in the level and rate of seepage of groundwater will occur due to variations in rainfall and other factors.

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### **3.0 GEOTECHNICAL RECOMMENDATIONS**

The following geotechnical recommendations are based on our field and laboratory testing, as well as our evaluation of the subsurface materials encountered in the test borings. These recommendations apply specifically to the proposed project as currently described. Any significant changes in building configuration, location, assumed loading, or assumed grades should be brought to our attention for further evaluation, as they may affect our conclusions.

#### **3.1 Geotechnical Summary**

The proposed construction is considered geotechnically feasible, provided that site preparation is performed in accordance with the recommendations of this report.

Site soils, which consist of moisture-sensitive clayey sands, are susceptible to disturbance. Where loose, disturbed, or otherwise unsuitable conditions are encountered, subgrade improvement will be required. Acceptable measures include removal and replacement with engineered fill, moisture conditioning and re-compaction, or chemical stabilization where field conditions warrant.

Fine, dry sands within the subgrade shall be recompacted to at least 95 percent of the maximum dry density in accordance with ASTM D1557.

Due to the potential for sloughing in sandy soils, conventional open-trench excavation may not be suitable for footing construction. Temporary shoring or alternative excavation and stabilization methods may be required to maintain stable conditions during construction.

Groundwater should be anticipated in excavations extending to or below the depths at which groundwater was encountered during the subsurface exploration.

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Construction should be performed during favorable (dry) weather conditions whenever possible. The subgrade should be properly graded for drainage and protected from disturbance prior to placement of fill or structural elements. Where necessary, undercutting, moisture conditioning, or replacement with engineered fill may be required to restore suitable subgrade conditions.

Construction traffic should be restricted to designated haul routes to prevent disturbance or softening of the subgrade. Care shall be taken during grading and fill placement to maintain stability.

Continued geotechnical observation during construction is recommended. GME Testing should observe subgrade preparation, evaluate fill materials, monitor compaction, and verify compliance with this report.

### **3.2 General Earthwork Recommendations**

The following earthwork recommendations are based on the subsurface conditions described in **Section 3.1**.

#### **3.2.1 Site Preparation**

Site stripping within the proposed construction areas should include the removal of vegetation, unsuitable fill, organics, and other deleterious materials. These areas should be observed by the geotechnical engineer during grading to confirm that the recommendations of the geotechnical engineer are followed.

Existing below-ground utilities that are abandoned or conflict with the proposed improvements should be removed, demolished, or properly relocated.

Following this, the exposed subgrade shall be proofrolled in the presence of GME Testing using medium-weight equipment to evaluate stability.

---

The moisture content of the supporting soils should be maintained within 2 percent of optimum moisture contents as determined by ASTM D-1557.

Earthwork operations should be scheduled during favorable weather conditions where practicable, as wet conditions and construction traffic can degrade subgrade performance. The exposed subgrade should be graded to promote positive drainage and protected from moisture intrusion and disturbance.

GME Testing should be retained to observe subgrade preparation, evaluate borrow and fill materials, monitor compaction, and confirm general conformance with these recommendations to support long-term performance of the proposed construction.

### **3.2.2 Engineered Fill**

Wherever new grade-raise fill is required, it shall be approved and consist of engineered fill materials and compacted using suitable equipment to a dry density of 95 percent or more of the maximum density obtained in accordance with ASTM D-1557. The compaction moisture content should generally be within 2 percent of the optimum.

To achieve the recommended compaction limit of the structural fill, the fill material should be placed and compacted in layers not exceeding 8 inches in loose thickness (the loose lift thickness should be reduced to 6 inches when utilizing small hand compactors). GME Testing should be retained to provide continuous inspection and documentation of the earthwork process.

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### **3.2.3 Site Drainage and Groundwater Control**

Positive surface drainage away from structures should be maintained at all times to prevent water infiltration and potential damage. Roof runoff and surface water, including downspouts, should be directed to approved off-site drainage locations (e.g., streets or designated outfalls) using properly graded swales or other positive drainage measures.

Water encountered during excavation or subgrade preparation should be promptly removed and discharged away from active construction areas and existing structures. If excavations extend to groundwater, dewatering will be required. Sumps and pumps may be used to control water in shallow excavations; however, the contractor is responsible for selecting and implementing appropriate dewatering methods to maintain suitable construction conditions.

### **3.2.4 Excavations and Trenches**

All excavation and trenching must comply with OSHA 29 CFR Part 1926, Subpart P, covering excavation safety. Trenches and excavations for utilities and other construction activities are subject to caving sides and can expose workers to engulfment hazards. A “Competent Person” must monitor all excavations per OSHA requirements. Appropriate shoring, sloping, or benching must be used to prevent cave-ins.

## **3.3 Foundation Recommendations**

### **3.3.1 Maintenance Barn at Site 1: Lakeland Intermediate School**

Following proper earthwork, site preparation, and placement of engineered fill as previously discussed, the proposed maintenance building may be supported on conventional foundations.

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Conventional foundations may include isolated spread (column) footings and continuous strip (wall) footings, provided they bear on approved native soils or properly compacted engineered fill extending to approved native soils.

In areas with soft or weak soils in the upper 10 feet (notably borings B-03 through B-05), foundations may need to extend to depths of approximately 5 to 6 feet or more below the existing grade. Final bearing depths should be verified in the field. Deeper embedment may be required where soils at the base of footing excavations exhibit moisture contents exceeding 2 percent above the optimum moisture content (OMC), as determined by ASTM D1557. GME Testing should be retained on-site to evaluate the extent of remediation required.

Provided that our recommendations provided in this report are completely followed, a maximum net allowable soil bearing pressure of **1,500 pounds per square foot (psf)** for column (square type) and strip (wall type) foundations may be used for dimensioning the footings at the recommended bearing depths mentioned above.

### **3.3.2 Building Addition at Site 2: Lakeland Junior/Senior High School**

Following earthwork, site preparation, and placement of engineered fill as previously described, the proposed building additions may be supported on conventional spread (column) and strip (wall) footings bearing on competent native soils and/or properly compacted engineered fill.

All foundations should extend below existing fill and any unsuitable subsurface materials and may require deeper embedment, where necessary, to achieve suitable bearing conditions and minimize settlement risk. Foundations shall not bear on unsuitable materials, including weak or compressible soils, existing foundation backfill, undocumented fill, or organic soils.

Clayey silty sands and clayey sands were encountered at typical footing depths in all borings. These soils are moisture- and disturbance-sensitive and may experience a reduction in strength upon exposure to construction activities, precipitation, or groundwater. Therefore, subgrade conditions should be evaluated at the time of construction, and stabilization measures, such as undercutting and replacement, may be required where disturbed or softened conditions are observed.

Footing subgrades should consist of approved firm native soils and/or properly compacted engineered fill. Loose and very loose sands encountered in the borings should be densified using appropriate compaction methods to achieve a minimum of 95 percent of the maximum dry density in accordance with ASTM D1557.

Provided that our recommendations provided in this report are completely followed, a maximum net allowable soil bearing pressure of **1,500 pounds per square foot (psf)** for column (square type) and strip (wall type) foundations may be used for dimensioning the footings and at the recommended bearing depths.

Care must be taken when positioning new foundations for the proposed building additions adjacent to existing foundations to avoid overlapping zones of influence and to prevent compromising the existing foundations due to excavation below their bearing elevations. Depending on the excavation depth, proximity to the existing structure, and other factors, it may be necessary to provide bracing or shoring for the excavation sides. Footprints of the proposed structures were not provided to GME. All footings should be placed to ensure a clear horizontal distance between any two footings that is at least equal to the difference in their bearing elevations, as illustrated in Exhibit C in Appendix A of this report. If maintaining this distance is not feasible, the lower footing should be designed to accommodate the load from the upper footing. Expansion joints should be provided between the existing building and the proposed additions to accommodate potential differential movements between the structures.

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### **3.3.3 General Discussions**

When using net pressure, the weight of the footing and backfill over the footing, including the weight of the floor slab, need not be considered. Hence, only loads applied at or above the finished floor must be used to dimension the footings.

New fill soil below, above, and surrounding footings shall consist of approved materials and then be placed and compacted in accordance with this report.

If any water infiltration is encountered in the footing excavations, it should be removed by adequate sumps placed outside the limits of the main footing excavations.

Provided that our recommendations in this report and project specifications are followed, total foundation settlements are not expected to exceed about (1) inch with differential settlements of up to (1/2) inch. Field control and proper footing proportions will contribute substantially to minimizing total and differential settlements.

The foundation sizes and reinforcement should be determined by the professional structural engineer.

### **Foundation Frost Depth**

All approved foundations resting on competent-bearing soils must be protected during and post-construction. All exterior footings and footings in unheated areas should be located at a depth of 3.5 feet or more below the final exterior grade for frost protection. Interior footings in heated areas can be located at nominal depths below the finished floor, provided that they bear on firm materials and provided that other recommendations in this report are followed.

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### **Foundation Inspection**

It can be challenging to accurately distinguish between loose and compacted soils based on visual inspection alone. Therefore, performing in-situ testing, such as the Dynamic Cone Penetration (DCP) test, is recommended.

It is strongly recommended that GME Testing be retained to perform continuous inspections of all earth-connected phases of the project and monitor and document them. This will ensure that each footing excavation is checked during the construction and installation of all foundations and slabs, allowing adherence to the recommendations outlined in this report, plans, and project specifications.

To reduce the bearing disturbance caused by inclement weather, sloughing, etc., foundation excavations should be backfilled or concreted immediately following satisfactory evaluation by GME Testing. If this is not practical, soils exposed at the bases of all excavations must be protected against any detrimental change in conditions, such as from disturbance, rain, and freezing.

### **Seismicity Classification**

Based on the soil characteristics encountered in test borings, the site is classified as ASCE 7-10 Site Class D, with a Risk Category III and a Seismic Design Category B, as summarized in the seismic report included in Appendix B. Considering the above, seismic design parameters  $S_s = 0.099g$ ;  $S_1 = 0.055g$ ;  $S_{DS} = 0.105$ ; and  $S_{D1} = 0.088$  may be used in the foundation design.

Seismic parameters are based on available subsurface data and published hazard values; final design should be confirmed by the structural engineer in accordance with the governing building code.

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### 3.4 Ground-Supported Floor Slabs

This section is applicable to the Maintenance Barn at Site 1 (Lakeland Intermediate School) and the Building Addition at Site 2 (Lakeland Junior/Senior High School).

Prior to placement of new fill, the existing subgrade should be prepared in accordance with **Sections 3.1 and 3.2** to provide uniform and stable support for slab-on-grade construction.

Floor slabs are assumed to be floating and isolated from footings, except at thickened sections supporting interior load-bearing elements.

Subgrade soils should be properly aerated, conditioned, and recompact as needed to reduce moisture content and improve uniformity prior to placing the subbase.

Within slab areas, existing materials should be proof-rolled to assess their suitability for support. Any yielding, pumping, or otherwise unstable soils should be remediated to achieve a firm, unyielding subgrade.

Ground-supported slabs should be designed for anticipated loading conditions using established methodologies (e.g., PCA or ACI), with final slab thickness, reinforcement, and joint detailing determined by the structural engineer.

The upper 6 to 8 inches of slab subbase should consist of approved crushed limestone aggregate (e.g., INDOT No. 53) to provide a leveling course and capillary break. A modulus of subgrade reaction (k) of 100 pci may be used for the design of ground-supported floor slabs, provided that the slab is constructed on properly prepared and compacted subgrade soils with a suitable granular base and as mentioned in this report.

The slab subgrade should be proofrolled immediately prior to concrete placement. The final subbase thickness and composition should be determined by the structural engineer in coordination with the geotechnical engineer.

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Care should be taken when placing and compacting backfill adjacent to foundations and walls, as inadequate compaction may result in slab edge settlement and cracking.

Depending on the selected floor finish, a vapor retarder beneath the slab may be appropriate. The need for a vapor retarder should be evaluated by the design team in accordance with ACI guidance and the intended floor use.

### **3.5 Parking and Driveway Subgrade Areas**

Pavement subgrades and new fill should be prepared and placed in accordance with the recommendations presented in **Section 3.2** of this report. The following general minimum recommendations are offered.

Based on subsurface conditions encountered in the parking and driveway borings, and provided the subgrade is properly prepared, a California Bearing Ratio (CBR) value of approximately 2 percent (corresponding to a resilient modulus of approximately 3,000 psi) may be used for preliminary pavement design and estimating.

Elevated moisture conditions can significantly reduce subgrade strength and pavement support. Where practicable, earthwork should be performed during seasonally dry conditions. If proper conditioning cannot be achieved, unsuitable soils may require undercutting and replacement with approved fill.

Localized unstable areas encountered during construction may require additional undercutting, placement of a granular stone base, and/or geotextile reinforcement to achieve stable support.

Pavement design, materials, and construction should conform to current Indiana Department of Transportation (INDOT) Standards and Specifications.

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### 3.6 Playground Subgrade Areas

The proposed playground improvements at Wolcott Mills Elementary School (Site 3) and Lakeland Primary School (Site 4), including play structures and impact-attenuating surfacing systems, are considered geotechnically feasible provided proper subgrade preparation is performed in accordance with **Section 3.2** of this report.

Playground design and surfacing should be consistent with applicable guidance from the U.S. Consumer Product Safety Commission Public Playground Safety Handbook and relevant provisions of ASTM International standards, including ASTM F1487 and ASTM F1292.

All playground areas should be proofrolled to identify soft, loose, wet, or otherwise unsuitable soils. Unsuitable materials, including organic soils or undocumented fill, should be undercut and replaced with properly compacted engineered fill. Subgrades should be moisture-conditioned and compacted to at least 95 percent of the maximum dry density in accordance with ASTM D1557.

Subgrades beneath impact-attenuating surfacing should be firm, uniform, and stable to minimize post-construction settlement. Positive drainage should be maintained to prevent water accumulation within playground areas.

Playground equipment foundations (e.g., embedded posts or piers) should bear on competent native soils or properly compacted engineered fill, with final embedment verified in the field.

GME Testing should observe subgrade preparation, proofrolling, and compaction to confirm compliance with these recommendations.

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#### **4.0 GENERAL COMMENTS**

This field evaluation, laboratory testing, and geotechnical analyses presented in this geotechnical investigation report have been conducted in general accordance with current practice and the standard of care exercised by geotechnical consultants performing similar tasks in the project area. Although individual test borings are representative of the subsurface conditions at the boring locations on the dates drilled, they are not necessarily representative of the subsurface conditions between boring locations or subsurface conditions during other seasons of the year.

The lines of demarcation shown on the logs represent approximate boundaries between the various classifications. The stratification of soils, as shown on the accompanying test borehole logs, represents the soil conditions at the drilled borehole locations, and variations may occur between the boreholes. In-situ strata changes could occur gradually or at different levels. Also, it should be noted that the boreholes depict conditions at the particular locations and times indicated.

The report was prepared by GME Testing solely for the use of the Client in accordance with an executed contract. The Client's use of or reliance on this report is limited by the terms and conditions of the contract and by the qualifications and limitations stated in the report. It is also acknowledged that the Client's use of and reliance of this report is limited for reasons which include actual site conditions that may change with time; hidden conditions, not discoverable within the scope of the assessment may exist at the site; and the scope of the investigation may have been limited by time, budget and other constraints imposed by the client.

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Neither the report nor its contents, conclusions or recommendations are intended for the use of any party other than the Client. GME Testing and the Client assume no liability for any reliance placed on this report by such party. The rights of the client under contract may not be assigned to any person or entity, without the consent of GME Testing which shall not be unreasonably withheld.

Our services have been provided consistent with its professional standards of care. No other warranties are made, either expressed or implied.

We wish to remind you that we will store the samples for 30 days, after which time they will be discarded unless you request otherwise.

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# **APPENDIX A**

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## I. FIELD EXPLORATION

### Drilling and Sampling Procedures

The test borings were drilled using conventional augers to advance the holes and representative samples of the soils were obtained employing split-barrel sampling techniques in accordance with ASTM procedures D-1586-84. After completion of the borings and water level readings, the auger holes were backfilled with auger cuttings.

The description and depths of soil strata encountered and levels at which samples were recovered are indicated on the accompanying borehole log sheets in the Appendix B. In the column "Soil/Material Description" on the drill borehole log, the horizontal lines represent stratum changes. A solid line represents an observed change, and a dashed line represents an estimated change. An explanation of the symbols and terms used on the boring log sheets is given in Appendix B of this report.

### Field Tests and Measurements

**Standard Penetration Test:** During the sampling procedures, Standard Penetration Test (SPT) was performed at regular intervals through the depth of the borings. The SPT value ("N"-value) is defined as the number of blows required to advance a 2-inch O.D., split-barrel sampler a distance of one foot by a 140-pound hammer falling 30-inches. These values provide a useful preliminary indication of the consistency or relative density of most soil deposits and are included on the Borehole Logs in Appendix B.

**Water Level Measurements:** Groundwater level observations were made in the boring holes during and upon completion of the boring operations. The groundwater level measurements are noted on the boring logs presented herein.

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All recovered samples were returned to GME Testing laboratory for visual examination and subsequent laboratory testing.

## II. LABORATORY TESTING

Selected soil samples obtained from the drilling and sampling program were tested in the laboratory to evaluate additional pertinent engineering characteristics of the foundation materials necessary in estimating the engineering properties of these materials.

### Soil Laboratory Tests and Measurements

**Visual Classification:** All samples were visually classified by a geotechnical engineer in general accordance with ASTM D-2488, and on the Borehole Logs, which are located in Appendix B of this report.

**Moisture Content Tests:** The natural moisture content of selected samples was determined by ASTM method D-2216 and is recorded on the Borehole Logs as a percentage of dry weight of soil under the “MC”.

**Hand Penetration Tests:** Samples of cohesive soils obtained from the split spoon sampler were tested with a calibrated hand penetrometer to aid in evaluating the soil strength characteristics. The results from this testing are tabulated on the Borehole Logs under the heading “Q<sub>P</sub>”.

**Unconfined Compressive Strength Tests:** The undrained shear strengths of the cohesive soils were evaluated utilizing unconfined compressive tests on specimens obtained from the split-barrel and/or thin wall tube sampler. The values of strength tests performed on soil samples obtained from the split-barrel sampler are considered approximate recognizing that the sampler provides a representative but somewhat disturbed sample. The test results are tabulated on the Borehole Logs under the heading “Q<sub>u</sub>”.

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VICINITY MAP (NOT TO SCALE)



NOTES

1. All boring locations are approximate.
2. Vicinity map generated using imagery from google.com/maps.

EXHIBIT A.1 – APPROXIMATE BORING LOCATION MAP

**Project Name:** Proposed Improvements at Several Existing Lakeland School Corporation School Sites

**Location:** Various Locations, LaGrange, IN

**Client Name:** krM Architecture

**GME Project Number:** G26-033379



N

LEGEND



B-1

Test Boring Location and Designation



A UES Company



VICINITY MAP (NOT TO SCALE)



NOTES

1. All boring locations are approximate.
2. Vicinity map generated using imagery from google.com/maps.

EXHIBIT A.2 – APPROXIMATE BORING LOCATION MAP

**Project Name:** Proposed Improvements at Several Existing Lakeland School Corporation School Sites

**Location:** Various Locations, LaGrange, IN

**Client Name:** krM Architecture

**GME Project Number:** G26-033379



LEGEND



B-6

Test Boring Location and Designation



A UES Company



VICINITY MAP (NOT TO SCALE)



NOTES

1. The boring location is approximate.
2. Vicinity map generated using imagery from google.com/maps.



LEGEND

 B-12  
Test Boring Location and Designation

EXHIBIT A.3 – APPROXIMATE BORING LOCATION MAP

**Project Name:** Proposed Improvements at Several Existing Lakeland School Corporation School Sites

**Location:** Various Locations, LaGrange, IN

**Client Name:** krM Architecture

**GME Project Number:** G26-033379



A UES Company



**VICINITY MAP (NOT TO SCALE)**



**NOTES**

1. All boring locations are approximate.
2. Vicinity map generated using imagery from google.com/maps.



**LEGEND**

 B-10  
Test Boring Location and Designation

**EXHIBIT A.4 – APPROXIMATE BORING LOCATION MAP**

**Project Name:** Proposed Improvements at Several Existing Lakeland School Corporation School Sites

**Location:** Various Locations, LaGrange, IN

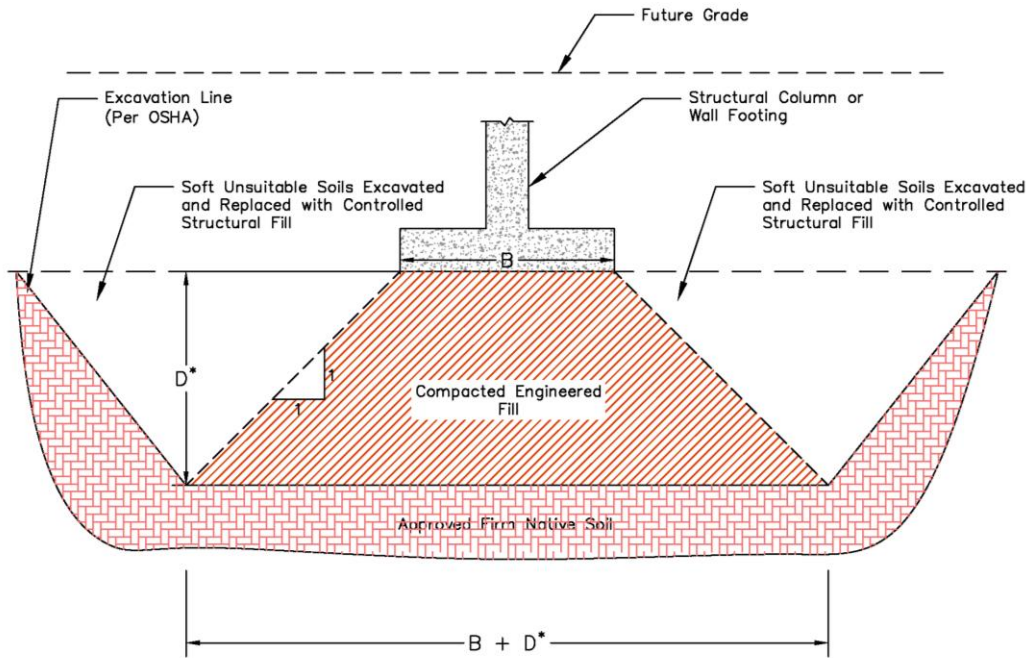
**Client Name:** krM Architecture

**GME Project Number:** G26-033379

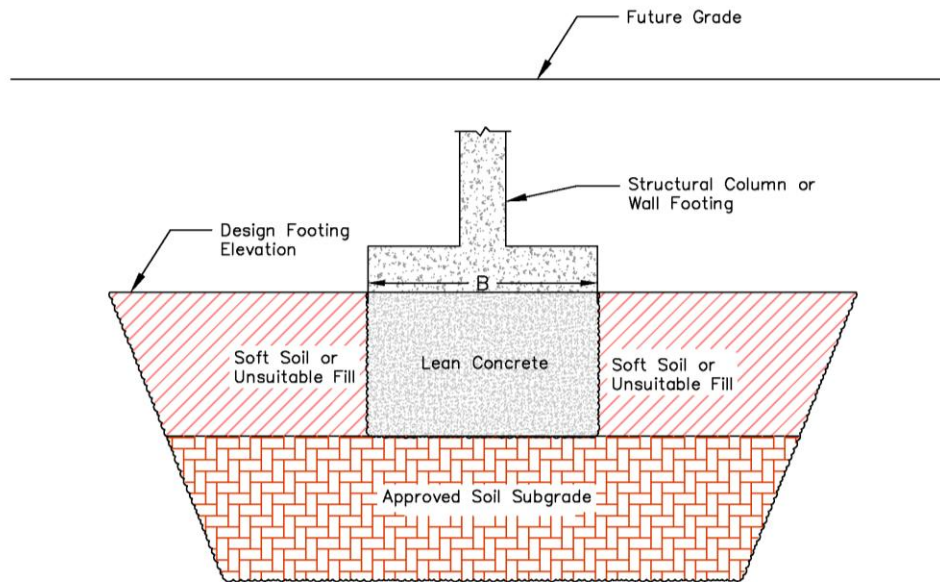


A UES Company

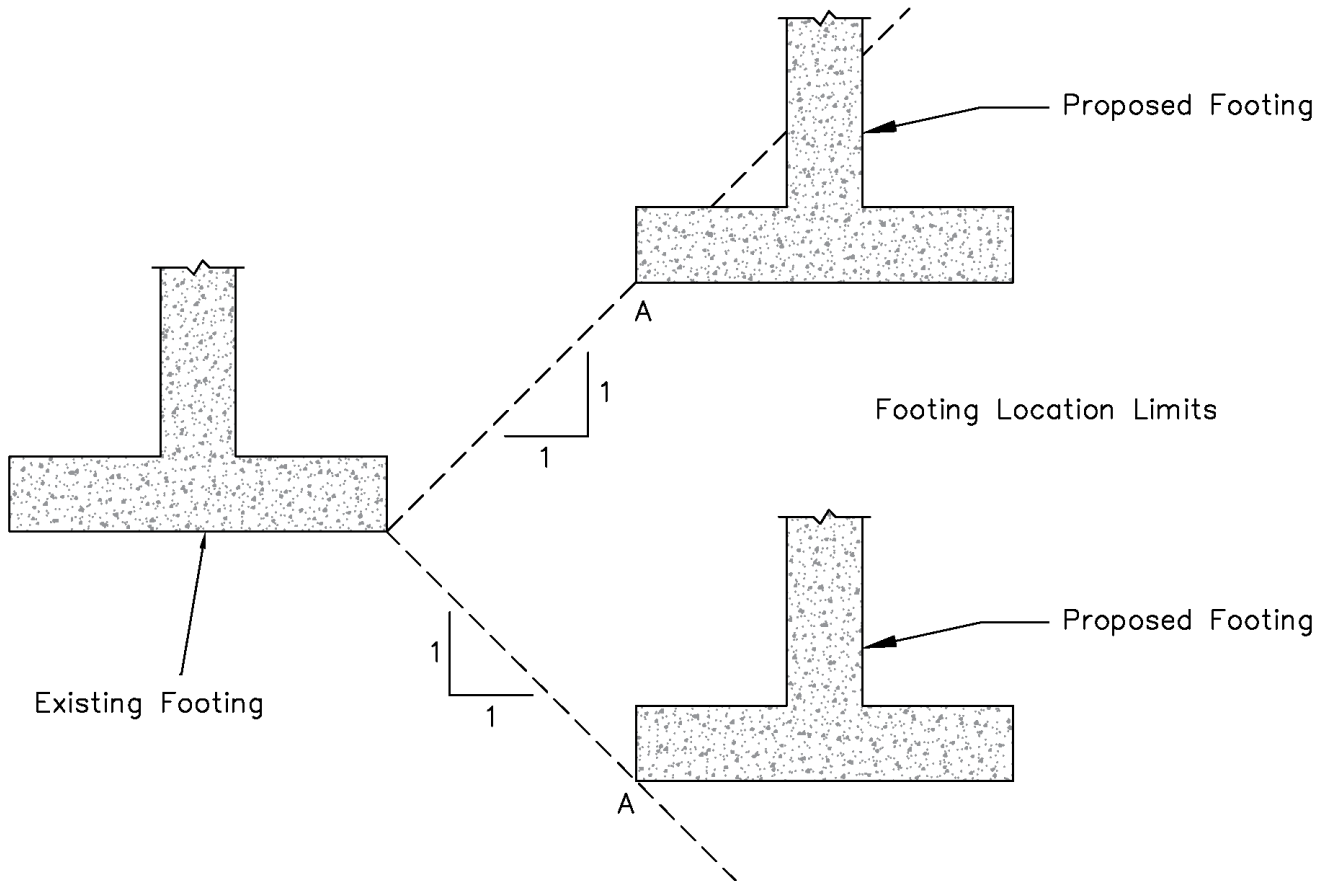
UNDERCUT EXCAVATION FOR FOOTINGS IN UNSUITABLE MATERIALS  
REPLACED WITH COMPACTED STRUCTURAL FILL



UNDERCUT EXCAVATION FOR FOOTINGS  
IN UNSUITABLE MATERIALS REPLACED WITH  
LEAN CONCRETE



## PLACEMENT OF ADJACENT FOOTINGS



Note: Point 'A' of the proposed footing must be within the dashed lines showing the footing location limits

# **APPENDIX B**



# TEST BORING LOG

BORING NO.: **B-01**  
 SHEET 1 OF 1  
**GME PROJECT NO: G26-033379**  
 STRUCTURE \_\_\_\_\_  
 DATUM : \_\_\_\_\_  
 DATE STARTED : 04-17-26  
 DRILLER/INSP : RS/AB

CLIENT: krM Architecture  
 PROJECT TYPE: Proposed Improvements at Several Existing Lakeland School Corporation School Sites  
 LOCATION: Various Locations, LaGrange, IN

ELEVATION : <u>921.0</u>	BORING METHOD : <u>ASTM D-1586</u>	LATITUDE : <u>41.653286</u>
STATION : _____	RIG TYPE : <u>Skid</u>	LONGITUDE : <u>-85.405317</u>
OFFSET : _____	CASING DIA. : <u>3.3 in</u>	
LINE : _____	HAMMER : <u>Auto</u>	
DEPTH : <u>25.0 ft</u>		

GROUNDWATER:  $\nabla$  Encountered at 21.0 ft  $\blacktriangledown$  At completion 15.0 ft

STRATUM ELEVATION	SAMPLE DEPTH	SOIL/MATERIAL DESCRIPTION	SAMPLE NUMBER	SPT per 6" (N)	% RECOVERY	% MOISTURE CONTENT	UNCONF. COMP., tsf	Qp (tsf)	REMARKS
920.3	0.7	±8" Dark Brown, Silty Clayey TOPSOIL.							
	2.5	FILL: Brown, Moist, Fine Clayey Silty Sand, Trace Fine Gravel.	SS 1	5-3-2 (5)	100	11.0			
	5.0		SS 2	2-1-1 (2)	100	18.5			
915.5	5.5	Brown, Moist, SANDY SILTY CLAY.	SS 3	4-6-7 (13)	100	13.2		2.5	
	10.0		SS 4	5-6-8 (14)	100	12.4		4.0	
908.5	12.5		SS 5	7-8-9 (17)	100	12.7		2.0	
906.0	15.0	Brown, Moist to Very Moist, Fine to Medium Coarse SAND, Trace Fine Gravel.	SS 6	7-9-12 (21)	100	14.8			
	25.0		SS 7	8-11-14 (25)	100	17.3			
	25.0	Bottom of Boring at 25.0 ft							



# TEST BORING LOG

BORING NO.: **B-02**  
 SHEET 1 OF 1  
 GME PROJECT NO: **G26-033379**  
 STRUCTURE \_\_\_\_\_  
 DATUM : \_\_\_\_\_  
 DATE STARTED : 04-17-26  
 DRILLER/INSP : RS/AB

CLIENT: krM Architecture  
 PROJECT TYPE: Proposed Improvements at Several Existing Lakeland School Corporation School Sites  
 LOCATION: Various Locations, LaGrange, IN

ELEVATION : <u>928.0</u>	BORING METHOD : <u>ASTM D-1586</u>	LATITUDE : <u>41.653117</u>
STATION : _____	RIG TYPE : <u>Skid</u>	LONGITUDE : <u>-85.404317</u>
OFFSET : _____	CASING DIA. : <u>3.3 in</u>	
LINE : _____	HAMMER : <u>Auto</u>	
DEPTH : <u>25.0 ft</u>		

GROUNDWATER:  Encountered at 8.0 ft       At completion 7.0 ft

STRATUM ELEVATION	SAMPLE DEPTH	SOIL/MATERIAL DESCRIPTION	SAMPLE NUMBER	SPT per 6" (N)	% RECOVERY	% MOISTURE CONTENT	UNCONF. COMP., tsf	Qp (tsf)	REMARKS
927.4	0.6	±7" Dark Brown, Sandy Clayey TOPSOIL.							
	2.5		SS 1	5-2-2 (4)	100	14.5	1.8	2.0	
	5.0		SS 2	2-2-3 (5)	100	15.2		0.5	
	7.5	Brown, Moist to Very Moist, SANDY SILTY CLAY.	SS 3	3-4-4 (8)	100	11.7		1.0	
	10.0		SS 4	5-6-6 (12)	100	12.0	1.6	1.5	
916.0	12.0								
	15.0	Brown, Moist, SILTY CLAY.	SS 5	8-8-9 (17)	100	16.4		3.0	
911.0	17.0								
	20.0	Brown, Moist, Fine to Medium Coarse SAND, Trace Fine Gravel.	SS 6	12-18-21 (39)	100	8.8			
903.0	25.0		SS 7	28-31-35 (66)	100	7.9			
	25.0	Bottom of Boring at 25.0 ft							





# TEST BORING LOG

BORING NO.: **B-04**  
 SHEET 1 OF 1  
**GME PROJECT NO: G26-033379**  
 STRUCTURE \_\_\_\_\_  
 DATUM : \_\_\_\_\_  
 DATE STARTED : 04-17-26  
 DRILLER/INSP : RS/AB

CLIENT: krM Architecture  
 PROJECT TYPE: Proposed Improvements at Several Existing Lakeland School Corporation School Sites  
 LOCATION: Various Locations, LaGrange, IN

ELEVATION : <u>929.0</u>	BORING METHOD : <u>ASTM D-1586</u>	LATITUDE : <u>41.653011</u>
STATION : _____	RIG TYPE : <u>Skid</u>	LONGITUDE : <u>-85.403961</u>
OFFSET : _____	CASING DIA. : <u>3.3 in</u>	
LINE : _____	HAMMER : <u>Auto</u>	
DEPTH : <u>25.0 ft</u>		

GROUNDWATER:  Encountered at 3.0 ft       At completion 3.0 ft

STRATUM ELEVATION	SAMPLE DEPTH	SOIL/MATERIAL DESCRIPTION	SAMPLE NUMBER	SPT per 6" (N)	% RECOVERY	% MOISTURE CONTENT	UNCONF. COMP., tsf	Qp (tsf)	REMARKS
927.5	1.5	±18" Dark Brown, Silty Clayey TOPSOIL.	SS 1	1-2-2 (4)	100	13.1			
924.0	5.0	Brown, Moist, SILTY CLAYEY SAND.	SS 2	2-2-1 (3)	100	16.7			
	7.5	Brown, Moist, SANDY SILTY CLAY, Trace Fine Gravel.	SS 3	4-5-6 (11)	100	13.0		2.5	
919.0	10.0		SS 4	5-6-6 (12)	100	12.0			
	12.5	Brown, Moist, Fine CLAYEY SANDY SILT, Trace Fine Gravel.	SS 5	10-12-16 (28)	100	10.8			
913.0	16.0		SS 6	8-10-12 (22)	100	14.5		3.0	
	20.0	Brown and Gray, Moist, SANDY SILTY CLAY, Trace Fine Gravel, Occasional Sand Seams.	SS 7	9-11-14 (25)	100	14.0		3.5	
904.0	25.0	Bottom of Boring at 25.0 ft							













# TEST BORING LOG

BORING NO.: **B-10**  
 SHEET 1 OF 1  
**GME PROJECT NO: G26-033379**  
 STRUCTURE \_\_\_\_\_  
 DATUM : \_\_\_\_\_  
 DATE STARTED : 04-17-26  
 DRILLER/INSP : JS/MT/GR

CLIENT: krM Architecture  
 PROJECT TYPE: Proposed Improvements at Several Existing Lakeland School Corporation School Sites  
 LOCATION: Various Locations, LaGrange, IN

ELEVATION : <u>927.0</u>	BORING METHOD : <u>ASTM D-1586</u>	LATITUDE : <u>41.642636</u>
STATION : _____	RIG TYPE : <u>Skid</u>	LONGITUDE : <u>-85.411261</u>
OFFSET : _____	CASING DIA. : <u>3.3 in</u>	
LINE : _____	HAMMER : <u>Auto</u>	
DEPTH : <u>15.0 ft</u>		

GROUNDWATER:  Encountered at Dry       At completion Dry

STRATUM ELEVATION	SAMPLE DEPTH	SOIL/MATERIAL DESCRIPTION	SAMPLE NUMBER	SPT per 6" (N)	% RECOVERY	% MOISTURE CONTENT	UNCONF. COMP., tsf	Qp (tsf)	REMARKS
926.2		Mulch/ ±10" PEA GRAVEL.							
924.5	2.5	POSSIBLE FILL: Dark Brown, Moist, Fine Clayey Sand, Trace Fine Gravel.	SS 1	2-2-3 (5)	75	17.5			
921.0	5.0	Brown, Moist, Mottled SANDY CLAY, Trace Fine Gravel.	SS 2	2-3-4 (7)	75	21.2		1.5	
	7.5		SS 3	7-8-9 (17)	75	12.6		<0.5	
	10.0	Brown, Very Moist, SANDY SILTY CLAY, Trace Fine Gravel.	SS 4	4-3-2 (5)	50	12.2		<0.5	
912.0	15.0		SS 5	5-7-9 (16)	50	13.2		<0.5	
		Bottom of Boring at 15.0 ft							





# GENERAL NOTES

## SAMPLE IDENTIFICATION

Visual soil classifications are made in general accordance with the United States Soil Classification System on the basis of textural and particle size categorization, and various soil behavior and characteristics. Visual classifications should be made by appropriate laboratory testing when more exact soil identification is required to satisfy specific project applications criteria.

### RELATIVE PROPORTIONS OF COHESIONLESS SOILS

<u>Term</u>	<u>Defining Range by % of Weight</u>
Trace	1-10 %
Little	11-20 %
Some	21-35 %
And	36-50 %

### WATER LEVEL MEASUREMENT

NE	No Water Encountered
BF	Backfilled upon Completion

### ORGANIC CONTENT BY COMBUSTION METHOD

<u>Soil Description</u>	<u>LOI</u>
w/ organic matter	4-15 %
Organic Soil (A-8)	16-30 %
Peat (A-8)	More than 30%

### LABORATORY TESTS

Qp	Penetrometer Reading, tsf
Qu	Unconfined Strength, tsf
MC	Moisture Content, %
LL	Liquid Limit, %
PL	Plastic Limit, %
PI	Plastic Index
SL	Shrinkage Limit, %
pH	Measure of Soil Alkalinity/Acidity
$\gamma$	Dry Unit Weight, pcf
LOI	Loss of Ignition, %

### DRILLING AND SAMPLING SYMBOLS

AS	Auger Sample
BS	Bag Sample
PID	Photo ionization Detector (Hnu meter) volatile vapor level,(PPM)
COA	Clean-Out Auger
CS	Continuous Sampling
FA	Flight Auger
HA	Hand Auger
HAS	Hollow Stem Auger
NR	No Recovery
PT	3" O.D. Piston Tube Sample
RB	Rock Bit
RC	Rock Coring
REC	Recovery
RQD	Rock Quality Designation
RS	Rock Sounding
S	Soil Sounding
SS	2" O.D. Split-Barrel Sample
2ST	2" O.D. Tin-Walled Tube Sample
3ST	3" O.D. Thin-Walled Tube Sample
VS	Vane Shear Test
DB	Diamond Bit
WS	Wash Sample
RB	Roller Bit
ST	Shelby Tube, 2" O.D. or 3" O.D.
CB	Carbide Bit
WOH	Weight of the Hammer

### GRAIN SIZE TERMINOLOGY

### RELATIVE DENSITY

### CONSISTENCY

### PLASTICITY

<u>Soil fraction</u>	<u>Particle size</u>	<u>Us standard sieve size</u>	<u>Term</u>	<u>"N" Value</u>	<u>Term</u>	<u>"N" Value</u>	<u>Term</u>	<u>Plastic Index</u>
Boulders	larger than 75 mm	Larger than 3"	Very Loose	0-5	Very Soft	0-3	None to Slight	0-4
Gravel	2mm to 75 mm	#10 to 75 mm	Loose	6-10	Soft	4-5	Slight	5-7
Coarse Sand	0.425 mm to 2 mm	#40 to #10	Medium Dense	11-30	Medium Stiff	6-10	Medium	8-22
Fine Sand	0.075mm to 0.425 mm	#200 to #40	Dense	31-50	Stiff	11-15	High/Very High	Over 22
Silt	0.002 mm to 0.075 mm	Smaller than #200	Very Dense	51+	Very Stiff	16-30		
Clay	Smaller than 0.002 mm	Smaller than #200			Hard	31+		

Note(s):

The penetration resistance, "N" Value, is the summation of the number of blows required to effect two successive 6-inch penetrations of the 2-inch split-barrel sampler. The sampler is driven with a 140-lb. weight falling 30-inches and is seated to a depth of 6-inches before commencing the standard penetration test.

Water level measurements shown on the boring logs represent conditions at the time indicated and may not reflect static levels, especially in cohesive soils

## GME TESTING

3517 Focus Drive  
Fort Wayne, IN 46818

(260) 497- 8127 • (877) 660-4GME • (260) 497- 0826 fax  
Division of **GEOTECHNICAL & MATERIALS ENGINEERS, INC.**

[www.gmetesting.com](http://www.gmetesting.com)

# SOIL CLASSIFICATION CHART

MAJOR DIVISIONS			SYMBOLS		TYPICAL DESCRIPTIONS
			GRAPH	LETTER	
<b>COARSE GRAINED SOILS</b>  MORE THAN 50% OF MATERIAL IS LARGER THAN NO. 200 SIEVE SIZE	GRAVEL AND GRAVELLY SOILS  MORE THAN 50% OF COARSE FRACTION RETAINED ON NO. 4 SIEVE	CLEAN GRAVELS  (LITTLE OR NO FINES)		<b>GW</b>	WELL-GRADED GRAVELS, GRAVEL - SAND MIXTURES, LITTLE OR NO FINES
		GRAVELS WITH FINES  (APPRECIABLE AMOUNT OF FINES)		<b>GP</b>	POORLY-GRADED GRAVELS, GRAVEL - SAND MIXTURES, LITTLE OR NO FINES
		GRAVELS WITH FINES  (APPRECIABLE AMOUNT OF FINES)		<b>GM</b>	SILTY GRAVELS, GRAVEL - SAND - SILT MIXTURES
	SAND AND SANDY SOILS  MORE THAN 50% OF COARSE FRACTION PASSING ON NO. 4 SIEVE	CLEAN SANDS  (LITTLE OR NO FINES)		<b>SW</b>	WELL-GRADED SANDS, GRAVELLY SANDS, LITTLE OR NO FINES
		SANDS WITH FINES  (LITTLE OR NO FINES)		<b>SP</b>	POORLY-GRADED SANDS, GRAVELLY SAND, LITTLE OR NO FINES
		SANDS WITH FINES  (APPRECIABLE AMOUNT OF FINES)		<b>SM</b>	SILTY SANDS, SAND - SILT MIXTURES
<b>FINE GRAINED SOILS</b>  MORE THAN 50% OF MATERIAL IS SMALLER THAN NO. 200 SIEVE SIZE	SILTS AND CLAYS  LIQUID LIMIT LESS THAN 50		<b>ML</b>	INORGANIC SILTS AND VERY FINE SANDS, ROCK FLOUR, SILTY OR CLAYEY FINE SANDS OR CLAYEY SILTS WITH SLIGHT PLASTICITY	
			<b>CL</b>	INORGANIC CLAYS OF LOW TO MEDIUM PLASTICITY, GRAVELLY CLAYS, SANDY CLAYS, SILTY CLAYS, LEAN CLAYS	
			<b>OL</b>	ORGANIC SILTS AND ORGANIC SILTY CLAYS OF LOW PLASTICITY	
	SILTS AND CLAYS  LIQUID LIMIT GREATER THAN 50		<b>MH</b>	INORGANIC SILTS, MICACEOUS OR DIATOMACEOUS FINE SAND OR SILTY SOILS	
			<b>CH</b>	INORGANIC CLAYS OF HIGH PLASTICITY	
			<b>OH</b>	ORGANIC CLAYS OF MEDIUM TO HIGH PLASTICITY, ORGANIC SILTS	
HIGHLY ORGANIC SOILS				<b>PT</b>	PEAT, HUMUS, SWAMP SOILS WITH HIGH ORGANIC CONTENTS

NOTE: DUAL SYMBOLS ARE USED TO INDICATE BORDERLINE SOIL CLASSIFICATIONS

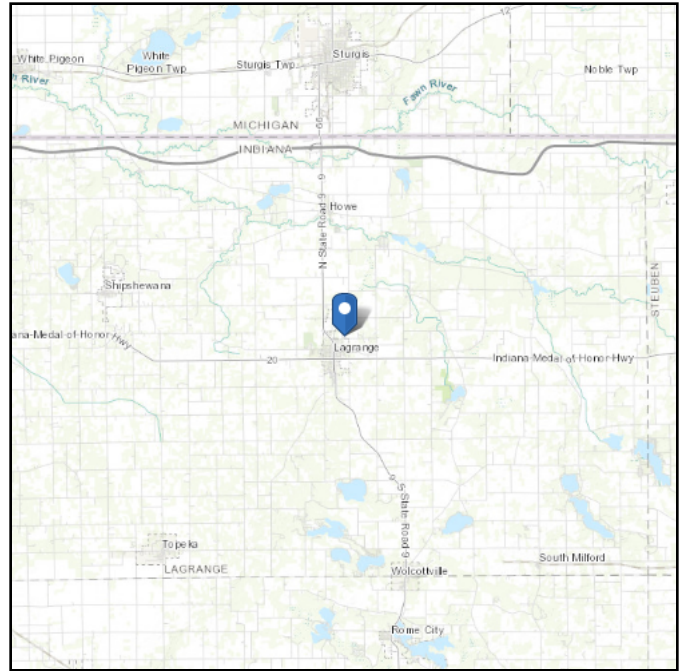
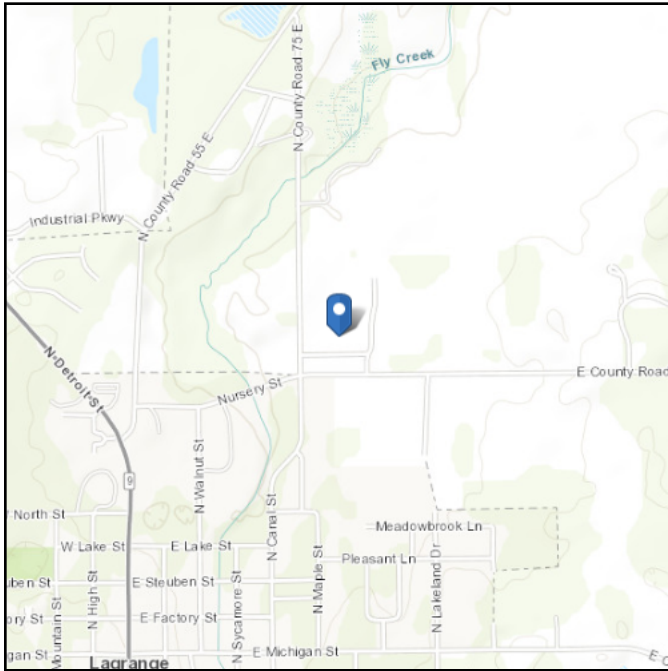


# ASCE Hazards Report

**Address:**  
No Address at This Location

**Standard:** ASCE/SEI 7-10  
**Risk Category:** III  
**Soil Class:** D - Stiff Soil

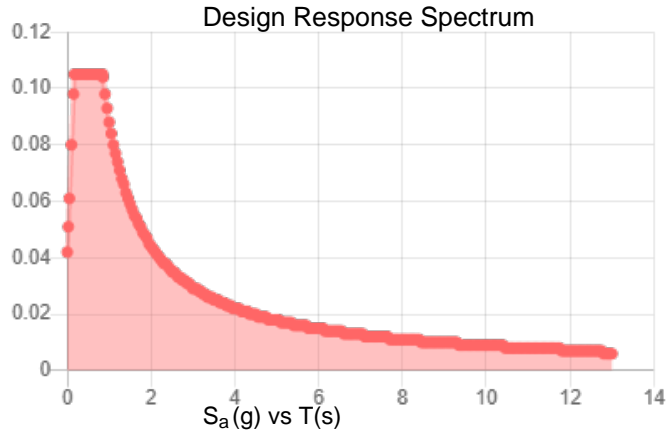
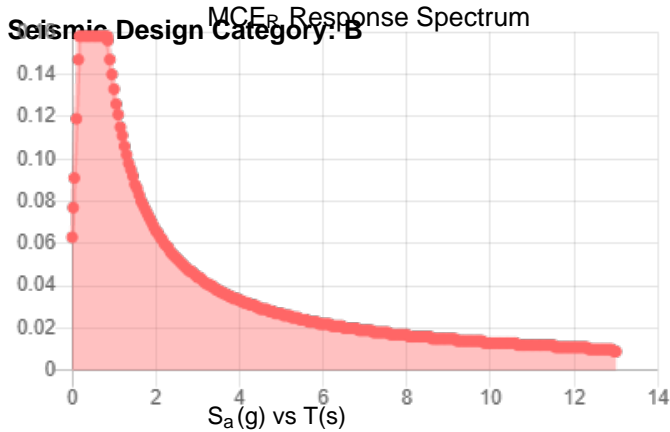
**Latitude:** 41.653339  
**Longitude:** -85.410483  
**Elevation:** 908.6916556004417 ft (NAVD 88)



**Site Soil Class:** D - Stiff Soil

**Results:**

$S_s$ :	0.099	$S_{D1}$ :	0.088
$S_1$ :	0.055	$T_L$ :	12
$F_a$ :	1.6	PGA :	0.046
$F_v$ :	2.4	PGA <sub>M</sub> :	0.073
$S_{MS}$ :	0.158	$F_{PGA}$ :	1.6
$S_{M1}$ :	0.133	$I_e$ :	1.25
$S_{DS}$ :	0.105		



**Data Accessed:** Thu Apr 23 2026

**Date Source:**

USGS Seismic Design Maps based on ASCE/SEI 7-10, incorporating Supplement 1 and errata of March 31, 2013, and ASCE/SEI 7-10 Table 1.5-2. Additional data for site-specific ground motion procedures in accordance with ASCE/SEI 7-10 Ch. 21 are available from USGS.

The ASCE Hazard Tool is provided for your convenience, for informational purposes only, and is provided “as is” and without warranties of any kind. The location data included herein has been obtained from information developed, produced, and maintained by third party providers; or has been extrapolated from maps incorporated in the ASCE standard. While ASCE has made every effort to use data obtained from reliable sources or methodologies, ASCE does not make any representations or warranties as to the accuracy, completeness, reliability, currency, or quality of any data provided herein. Any third-party links provided by this Tool should not be construed as an endorsement, affiliation, relationship, or sponsorship of such third-party content by or from ASCE.

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United States  
Department of  
Agriculture

**NRCS**

Natural  
Resources  
Conservation  
Service

A product of the National  
Cooperative Soil Survey,  
a joint effort of the United  
States Department of  
Agriculture and other  
Federal agencies, State  
agencies including the  
Agricultural Experiment  
Stations, and local  
participants

# Custom Soil Resource Report for **Lagrange County, Indiana**



# Custom Soil Resource Report Soil Map



Soil Map may not be valid at this scale.


Map Scale: 1:1,710 if printed on A landscape (11" x 8.5") sheet.



Map projection: Web Mercator Corner coordinates: WGS84 Edge ticks: UTM Zone 16N WGS84

### MAP LEGEND

**Area of Interest (AOI)**

 Area of Interest (AOI)




















**Soils**







 Soil Map Unit Polygons

 Soil Map Unit Lines


 Soil Map Unit Points

**Special Point Features**






-  Blowout
-  Borrow Pit
-  Clay Spot
-  Closed Depression
-  Gravel Pit
-  Gravelly Spot
-  Landfill
-  Lava Flow
-  Marsh or swamp
-  Mine or Quarry
-  Miscellaneous Water
-  Perennial Water
-  Rock Outcrop
-  Saline Spot
-  Sandy Spot
-  Severely Eroded Spot
-  Sinkhole
-  Slide or Slip
-  Sodic Spot

-  Spoil Area
-  Stony Spot
-  Very Stony Spot
-  Wet Spot
-  Other
-  Special Line Features

**Water Features**

 Streams and Canals

**Transportation**

-  Rails
-  Interstate Highways
-  US Routes
-  Major Roads
-  Local Roads

**Background**

 Aerial Photography

### MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:20,000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service  
 Web Soil Survey URL:  
 Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Lagrange County, Indiana  
 Survey Area Data: Version 29, Sep 3, 2025

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Jun 18, 2022—Jun 21, 2022

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

## Map Unit Legend

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
CrA	Conover loam, 0 to 3 percent slopes	0.2	1.5%
HdB	Hillsdale sandy loam, 2 to 6 percent slopes	9.1	65.7%
HdC	Hillsdale sandy loam, 6 to 12 percent slopes	1.7	12.0%
OsA	Oshtemo loamy sand, 0 to 2 percent slopes	2.9	20.8%
<b>Totals for Area of Interest</b>		<b>13.9</b>	<b>100.0%</b>

## Map Unit Descriptions

The map units delineated on the detailed soil maps in a soil survey represent the soils or miscellaneous areas in the survey area. The map unit descriptions, along with the maps, can be used to determine the composition and properties of a unit.

A map unit delineation on a soil map represents an area dominated by one or more major kinds of soil or miscellaneous areas. A map unit is identified and named according to the taxonomic classification of the dominant soils. Within a taxonomic class there are precisely defined limits for the properties of the soils. On the landscape, however, the soils are natural phenomena, and they have the characteristic variability of all natural phenomena. Thus, the range of some observed properties may extend beyond the limits defined for a taxonomic class. Areas of soils of a single taxonomic class rarely, if ever, can be mapped without including areas of other taxonomic classes. Consequently, every map unit is made up of the soils or miscellaneous areas for which it is named and some minor components that belong to taxonomic classes other than those of the major soils.

Most minor soils have properties similar to those of the dominant soil or soils in the map unit, and thus they do not affect use and management. These are called noncontrasting, or similar, components. They may or may not be mentioned in a particular map unit description. Other minor components, however, have properties and behavioral characteristics divergent enough to affect use or to require different management. These are called contrasting, or dissimilar, components. They generally are in small areas and could not be mapped separately because of the scale used. Some small areas of strongly contrasting soils or miscellaneous areas are identified by a special symbol on the maps. If included in the database for a given area, the contrasting minor components are identified in the map unit descriptions along with some characteristics of each. A few areas of minor components may not have been observed, and consequently they are not mentioned in the descriptions, especially where the pattern was so complex that it was impractical to make enough observations to identify all the soils and miscellaneous areas on the landscape.

## Custom Soil Resource Report

The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The objective of mapping is not to delineate pure taxonomic classes but rather to separate the landscape into landforms or landform segments that have similar use and management requirements. The delineation of such segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, however, onsite investigation is needed to define and locate the soils and miscellaneous areas.

An identifying symbol precedes the map unit name in the map unit descriptions. Each description includes general facts about the unit and gives important soil properties and qualities.

Soils that have profiles that are almost alike make up a *soil series*. Except for differences in texture of the surface layer, all the soils of a series have major horizons that are similar in composition, thickness, and arrangement.

Soils of one series can differ in texture of the surface layer, slope, stoniness, salinity, degree of erosion, and other characteristics that affect their use. On the basis of such differences, a soil series is divided into *soil phases*. Most of the areas shown on the detailed soil maps are phases of soil series. The name of a soil phase commonly indicates a feature that affects use or management. For example, Alpha silt loam, 0 to 2 percent slopes, is a phase of the Alpha series.

Some map units are made up of two or more major soils or miscellaneous areas. These map units are complexes, associations, or undifferentiated groups.

A *complex* consists of two or more soils or miscellaneous areas in such an intricate pattern or in such small areas that they cannot be shown separately on the maps. The pattern and proportion of the soils or miscellaneous areas are somewhat similar in all areas. Alpha-Beta complex, 0 to 6 percent slopes, is an example.

An *association* is made up of two or more geographically associated soils or miscellaneous areas that are shown as one unit on the maps. Because of present or anticipated uses of the map units in the survey area, it was not considered practical or necessary to map the soils or miscellaneous areas separately. The pattern and relative proportion of the soils or miscellaneous areas are somewhat similar. Alpha-Beta association, 0 to 2 percent slopes, is an example.

An *undifferentiated group* is made up of two or more soils or miscellaneous areas that could be mapped individually but are mapped as one unit because similar interpretations can be made for use and management. The pattern and proportion of the soils or miscellaneous areas in a mapped area are not uniform. An area can be made up of only one of the major soils or miscellaneous areas, or it can be made up of all of them. Alpha and Beta soils, 0 to 2 percent slopes, is an example.

Some surveys include *miscellaneous areas*. Such areas have little or no soil material and support little or no vegetation. Rock outcrop is an example.

## Lagrange County, Indiana

### CrA—Conover loam, 0 to 3 percent slopes

#### Map Unit Setting

*National map unit symbol:* 94ng  
*Elevation:* 600 to 940 feet  
*Mean annual precipitation:* 34 to 40 inches  
*Mean annual air temperature:* 47 to 51 degrees F  
*Frost-free period:* 140 to 185 days  
*Farmland classification:* Prime farmland if drained

#### Map Unit Composition

*Conover and similar soils:* 90 percent  
*Minor components:* 10 percent  
*Estimates are based on observations, descriptions, and transects of the mapunit.*

#### Description of Conover

##### Setting

*Landform:* Moraines  
*Landform position (two-dimensional):* Footslope, summit  
*Landform position (three-dimensional):* Interfluve  
*Down-slope shape:* Linear  
*Across-slope shape:* Linear  
*Parent material:* Loamy till

##### Typical profile

*Ap - 0 to 9 inches:* loam  
*E - 9 to 13 inches:* sandy loam  
*Bw - 13 to 16 inches:* sandy loam  
*Bt1 - 16 to 21 inches:* clay loam  
*Bt2 - 21 to 32 inches:* clay loam  
*BCt - 32 to 36 inches:* loam  
*C - 36 to 60 inches:* loam

##### Properties and qualities

*Slope:* 0 to 3 percent  
*Depth to restrictive feature:* More than 80 inches  
*Drainage class:* Somewhat poorly drained  
*Runoff class:* Medium  
*Capacity of the most limiting layer to transmit water (Ksat):* Moderately high (0.20 to 0.60 in/hr)  
*Depth to water table:* About 6 to 24 inches  
*Frequency of flooding:* None  
*Frequency of ponding:* None  
*Calcium carbonate, maximum content:* 35 percent  
*Available water supply, 0 to 60 inches:* High (about 9.1 inches)

##### Interpretive groups

*Land capability classification (irrigated):* None specified  
*Land capability classification (nonirrigated):* 2w  
*Hydrologic Soil Group:* C/D  
*Ecological site:* R111XC006IN - Flat Glacial Ridge  
*Hydric soil rating:* No

### Minor Components

#### Whitaker

*Percent of map unit:* 7 percent  
*Hydric soil rating:* No

#### Rensselaer

*Percent of map unit:* 3 percent  
*Landform:* Depressions  
*Hydric soil rating:* Yes

## HdB—Hillsdale sandy loam, 2 to 6 percent slopes

### Map Unit Setting

*National map unit symbol:* 94nn  
*Elevation:* 600 to 940 feet  
*Mean annual precipitation:* 34 to 40 inches  
*Mean annual air temperature:* 47 to 51 degrees F  
*Frost-free period:* 140 to 185 days  
*Farmland classification:* All areas are prime farmland

### Map Unit Composition

*Hillsdale and similar soils:* 100 percent  
*Estimates are based on observations, descriptions, and transects of the mapunit.*

### Description of Hillsdale

#### Setting

*Landform:* Moraines, Till plains  
*Landform position (two-dimensional):* Backslope, shoulder  
*Landform position (three-dimensional):* Side slope  
*Down-slope shape:* Linear  
*Across-slope shape:* Linear  
*Parent material:* Loamy till

#### Typical profile

*Ap - 0 to 10 inches:* sandy loam  
*E - 10 to 13 inches:* sandy loam  
*Bw - 13 to 17 inches:* sandy loam  
*Bt1 - 17 to 35 inches:* sandy loam  
*Bt2 - 35 to 41 inches:* sandy loam  
*Bt3 - 41 to 50 inches:* sandy loam  
*BC - 50 to 70 inches:* sandy loam  
*C - 70 to 80 inches:* sandy loam

#### Properties and qualities

*Slope:* 2 to 6 percent  
*Depth to restrictive feature:* More than 80 inches  
*Drainage class:* Well drained  
*Runoff class:* Medium

## Custom Soil Resource Report

*Capacity of the most limiting layer to transmit water (Ksat):* Moderately high to high (0.60 to 2.00 in/hr)

*Depth to water table:* More than 80 inches

*Frequency of flooding:* None

*Frequency of ponding:* None

*Available water supply, 0 to 60 inches:* Moderate (about 8.3 inches)

### Interpretive groups

*Land capability classification (irrigated):* None specified

*Land capability classification (nonirrigated):* 2e

*Hydrologic Soil Group:* A

*Ecological site:* F111XC007IN - Glacial Ridge

*Hydric soil rating:* No

## HdC—Hillsdale sandy loam, 6 to 12 percent slopes

### Map Unit Setting

*National map unit symbol:* 94np

*Elevation:* 600 to 940 feet

*Mean annual precipitation:* 34 to 40 inches

*Mean annual air temperature:* 47 to 51 degrees F

*Frost-free period:* 140 to 185 days

*Farmland classification:* Not prime farmland

### Map Unit Composition

*Hillsdale and similar soils:* 100 percent

*Estimates are based on observations, descriptions, and transects of the mapunit.*

### Description of Hillsdale

#### Setting

*Landform:* Moraines, Till plains

*Landform position (two-dimensional):* Backslope, shoulder

*Landform position (three-dimensional):* Side slope

*Down-slope shape:* Linear

*Across-slope shape:* Linear

*Parent material:* Loamy till

#### Typical profile

*Ap - 0 to 10 inches:* sandy loam

*E - 10 to 13 inches:* sandy loam

*Bw - 13 to 17 inches:* sandy loam

*Bt1 - 17 to 35 inches:* sandy loam

*Bt2 - 35 to 41 inches:* sandy loam

*Bt3 - 41 to 50 inches:* sandy loam

*BC - 50 to 70 inches:* sandy loam

*C - 70 to 80 inches:* sandy loam

#### Properties and qualities

*Slope:* 6 to 12 percent

*Depth to restrictive feature:* More than 80 inches

*Drainage class:* Well drained

## Custom Soil Resource Report

*Runoff class:* Medium

*Capacity of the most limiting layer to transmit water (Ksat):* Moderately high to high (0.60 to 2.00 in/hr)

*Depth to water table:* More than 80 inches

*Frequency of flooding:* None

*Frequency of ponding:* None

*Available water supply, 0 to 60 inches:* Moderate (about 8.3 inches)

### **Interpretive groups**

*Land capability classification (irrigated):* None specified

*Land capability classification (nonirrigated):* 3e

*Hydrologic Soil Group:* A

*Ecological site:* F111XC007IN - Glacial Ridge

*Hydric soil rating:* No

## **OsA—Oshtemo loamy sand, 0 to 2 percent slopes**

### **Map Unit Setting**

*National map unit symbol:* 94p2

*Elevation:* 600 to 940 feet

*Mean annual precipitation:* 34 to 40 inches

*Mean annual air temperature:* 47 to 51 degrees F

*Frost-free period:* 140 to 185 days

*Farmland classification:* Not prime farmland

### **Map Unit Composition**

*Oshtemo and similar soils:* 90 percent

*Minor components:* 10 percent

*Estimates are based on observations, descriptions, and transects of the mapunit.*

### **Description of Oshtemo**

#### **Setting**

*Landform:* Outwash plains

*Landform position (two-dimensional):* Summit

*Landform position (three-dimensional):* Interfluve

*Down-slope shape:* Linear

*Across-slope shape:* Linear

*Parent material:* Loamy outwash over sandy and gravelly outwash

#### **Typical profile**

*Ap - 0 to 9 inches:* loamy sand

*BE, Bt1, Bt2 - 9 to 36 inches:* gravelly sandy loam

*Bt3, BC - 36 to 53 inches:* gravelly sandy loam

*2C - 53 to 60 inches:* stratified very gravelly coarse sand to sand

#### **Properties and qualities**

*Slope:* 0 to 2 percent

*Depth to restrictive feature:* 24 to 60 inches to strongly contrasting textural stratification

*Drainage class:* Well drained

*Runoff class:* Very low

## Custom Soil Resource Report

*Capacity of the most limiting layer to transmit water (Ksat):* High (2.00 to 6.00 in/hr)

*Depth to water table:* More than 80 inches

*Frequency of flooding:* None

*Frequency of ponding:* None

*Calcium carbonate, maximum content:* 40 percent

*Available water supply, 0 to 60 inches:* Moderate (about 6.6 inches)

### **Interpretive groups**

*Land capability classification (irrigated):* None specified

*Land capability classification (nonirrigated):* 3s

*Hydrologic Soil Group:* A

*Ecological site:* F098XA015MI - Dry Loamy Drift Plains

*Hydric soil rating:* No

### **Minor Components**

#### **Boyer**

*Percent of map unit:* 7 percent

*Hydric soil rating:* No

#### **Gilford**

*Percent of map unit:* 3 percent

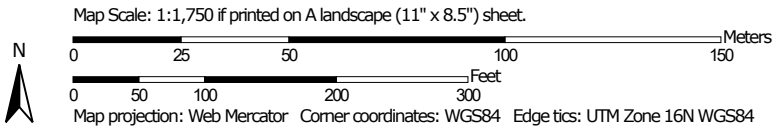
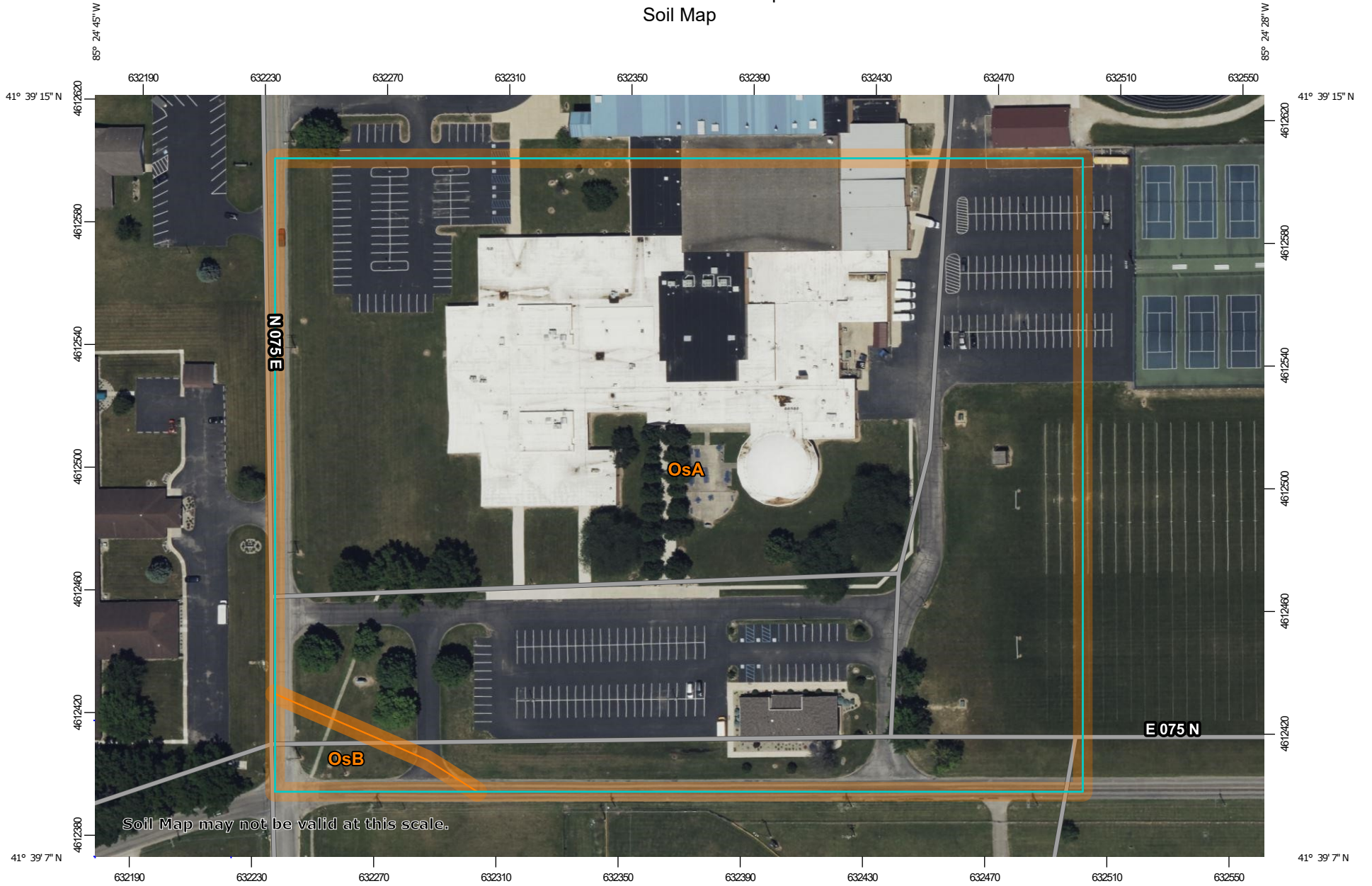
*Landform:* Depressions

*Hydric soil rating:* Yes

# Custom Soil Resource Report for Lagrange County, Indiana




# Custom Soil Resource Report Soil Map




### MAP LEGEND

**Area of Interest (AOI)**

 Area of Interest (AOI)




















**Soils**







 Soil Map Unit Polygons

 Soil Map Unit Lines


 Soil Map Unit Points

**Special Point Features**






-  Blowout
-  Borrow Pit
-  Clay Spot
-  Closed Depression
-  Gravel Pit
-  Gravelly Spot
-  Landfill
-  Lava Flow
-  Marsh or swamp
-  Mine or Quarry
-  Miscellaneous Water
-  Perennial Water
-  Rock Outcrop
-  Saline Spot
-  Sandy Spot
-  Severely Eroded Spot
-  Sinkhole
-  Slide or Slip
-  Sodic Spot

-  Spoil Area
-  Stony Spot
-  Very Stony Spot
-  Wet Spot
-  Other
-  Special Line Features


**Water Features**

 Streams and Canals

**Transportation**

-  Rails
-  Interstate Highways
-  US Routes
-  Major Roads
-  Local Roads

**Background**

 Aerial Photography

### MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:20,000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service  
 Web Soil Survey URL:  
 Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Lagrange County, Indiana  
 Survey Area Data: Version 29, Sep 3, 2025

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Jun 18, 2022—Jun 21, 2022

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

## Map Unit Legend

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
OsA	Oshtemo loamy sand, 0 to 2 percent slopes	13.3	97.9%
OsB	Oshtemo loamy sand, 2 to 6 percent slopes	0.3	2.1%
<b>Totals for Area of Interest</b>		<b>13.6</b>	<b>100.0%</b>

## Map Unit Descriptions

The map units delineated on the detailed soil maps in a soil survey represent the soils or miscellaneous areas in the survey area. The map unit descriptions, along with the maps, can be used to determine the composition and properties of a unit.

A map unit delineation on a soil map represents an area dominated by one or more major kinds of soil or miscellaneous areas. A map unit is identified and named according to the taxonomic classification of the dominant soils. Within a taxonomic class there are precisely defined limits for the properties of the soils. On the landscape, however, the soils are natural phenomena, and they have the characteristic variability of all natural phenomena. Thus, the range of some observed properties may extend beyond the limits defined for a taxonomic class. Areas of soils of a single taxonomic class rarely, if ever, can be mapped without including areas of other taxonomic classes. Consequently, every map unit is made up of the soils or miscellaneous areas for which it is named and some minor components that belong to taxonomic classes other than those of the major soils.

Most minor soils have properties similar to those of the dominant soil or soils in the map unit, and thus they do not affect use and management. These are called noncontrasting, or similar, components. They may or may not be mentioned in a particular map unit description. Other minor components, however, have properties and behavioral characteristics divergent enough to affect use or to require different management. These are called contrasting, or dissimilar, components. They generally are in small areas and could not be mapped separately because of the scale used. Some small areas of strongly contrasting soils or miscellaneous areas are identified by a special symbol on the maps. If included in the database for a given area, the contrasting minor components are identified in the map unit descriptions along with some characteristics of each. A few areas of minor components may not have been observed, and consequently they are not mentioned in the descriptions, especially where the pattern was so complex that it was impractical to make enough observations to identify all the soils and miscellaneous areas on the landscape.

The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The objective of mapping is not to delineate pure taxonomic classes but rather to separate the landscape into landforms or landform segments that have similar use and management requirements. The delineation of such segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, however,

## Custom Soil Resource Report

onsite investigation is needed to define and locate the soils and miscellaneous areas.

An identifying symbol precedes the map unit name in the map unit descriptions. Each description includes general facts about the unit and gives important soil properties and qualities.

Soils that have profiles that are almost alike make up a *soil series*. Except for differences in texture of the surface layer, all the soils of a series have major horizons that are similar in composition, thickness, and arrangement.

Soils of one series can differ in texture of the surface layer, slope, stoniness, salinity, degree of erosion, and other characteristics that affect their use. On the basis of such differences, a soil series is divided into *soil phases*. Most of the areas shown on the detailed soil maps are phases of soil series. The name of a soil phase commonly indicates a feature that affects use or management. For example, Alpha silt loam, 0 to 2 percent slopes, is a phase of the Alpha series.

Some map units are made up of two or more major soils or miscellaneous areas. These map units are complexes, associations, or undifferentiated groups.

A *complex* consists of two or more soils or miscellaneous areas in such an intricate pattern or in such small areas that they cannot be shown separately on the maps. The pattern and proportion of the soils or miscellaneous areas are somewhat similar in all areas. Alpha-Beta complex, 0 to 6 percent slopes, is an example.

An *association* is made up of two or more geographically associated soils or miscellaneous areas that are shown as one unit on the maps. Because of present or anticipated uses of the map units in the survey area, it was not considered practical or necessary to map the soils or miscellaneous areas separately. The pattern and relative proportion of the soils or miscellaneous areas are somewhat similar. Alpha-Beta association, 0 to 2 percent slopes, is an example.

An *undifferentiated group* is made up of two or more soils or miscellaneous areas that could be mapped individually but are mapped as one unit because similar interpretations can be made for use and management. The pattern and proportion of the soils or miscellaneous areas in a mapped area are not uniform. An area can be made up of only one of the major soils or miscellaneous areas, or it can be made up of all of them. Alpha and Beta soils, 0 to 2 percent slopes, is an example.

Some surveys include *miscellaneous areas*. Such areas have little or no soil material and support little or no vegetation. Rock outcrop is an example.

## Lagrange County, Indiana

### OsA—Oshtemo loamy sand, 0 to 2 percent slopes

#### Map Unit Setting

*National map unit symbol:* 94p2  
*Elevation:* 600 to 940 feet  
*Mean annual precipitation:* 34 to 40 inches  
*Mean annual air temperature:* 47 to 51 degrees F  
*Frost-free period:* 140 to 185 days  
*Farmland classification:* Not prime farmland

#### Map Unit Composition

*Oshtemo and similar soils:* 90 percent  
*Minor components:* 10 percent  
*Estimates are based on observations, descriptions, and transects of the mapunit.*

#### Description of Oshtemo

##### Setting

*Landform:* Outwash plains  
*Landform position (two-dimensional):* Summit  
*Landform position (three-dimensional):* Interfluve  
*Down-slope shape:* Linear  
*Across-slope shape:* Linear  
*Parent material:* Loamy outwash over sandy and gravelly outwash

##### Typical profile

*Ap - 0 to 9 inches:* loamy sand  
*BE,Bt1,Bt2 - 9 to 36 inches:* gravelly sandy loam  
*Bt3,BC - 36 to 53 inches:* gravelly sandy loam  
*2C - 53 to 60 inches:* stratified very gravelly coarse sand to sand

##### Properties and qualities

*Slope:* 0 to 2 percent  
*Depth to restrictive feature:* 24 to 60 inches to strongly contrasting textural stratification  
*Drainage class:* Well drained  
*Runoff class:* Very low  
*Capacity of the most limiting layer to transmit water (Ksat):* High (2.00 to 6.00 in/hr)  
*Depth to water table:* More than 80 inches  
*Frequency of flooding:* None  
*Frequency of ponding:* None  
*Calcium carbonate, maximum content:* 40 percent  
*Available water supply, 0 to 60 inches:* Moderate (about 6.6 inches)

##### Interpretive groups

*Land capability classification (irrigated):* None specified  
*Land capability classification (nonirrigated):* 3s  
*Hydrologic Soil Group:* A  
*Ecological site:* F098XA015MI - Dry Loamy Drift Plains  
*Hydric soil rating:* No

### Minor Components

#### Boyer

*Percent of map unit:* 7 percent  
*Hydric soil rating:* No

#### Gilford

*Percent of map unit:* 3 percent  
*Landform:* Depressions  
*Hydric soil rating:* Yes

## OsB—Oshtemo loamy sand, 2 to 6 percent slopes

### Map Unit Setting

*National map unit symbol:* 94p3  
*Elevation:* 600 to 940 feet  
*Mean annual precipitation:* 34 to 40 inches  
*Mean annual air temperature:* 47 to 51 degrees F  
*Frost-free period:* 140 to 185 days  
*Farmland classification:* Not prime farmland

### Map Unit Composition

*Oshtemo and similar soils:* 100 percent  
*Estimates are based on observations, descriptions, and transects of the mapunit.*

### Description of Oshtemo

#### Setting

*Landform:* Outwash plains  
*Landform position (two-dimensional):* Backslope, shoulder  
*Landform position (three-dimensional):* Side slope  
*Down-slope shape:* Linear  
*Across-slope shape:* Linear  
*Parent material:* Loamy outwash over sandy and gravelly outwash

#### Typical profile

*Ap - 0 to 9 inches:* loamy sand  
*BE,Bt1,Bt2 - 9 to 36 inches:* gravelly sandy loam  
*Bt3,BC - 36 to 53 inches:* gravelly sandy loam  
*2C - 53 to 60 inches:* stratified very gravelly coarse sand to sand

#### Properties and qualities

*Slope:* 2 to 6 percent  
*Depth to restrictive feature:* 24 to 60 inches to strongly contrasting textural stratification  
*Drainage class:* Well drained  
*Runoff class:* Very low  
*Capacity of the most limiting layer to transmit water (Ksat):* High (2.00 to 6.00 in/hr)  
*Depth to water table:* More than 80 inches

## Custom Soil Resource Report

*Frequency of flooding:* None

*Frequency of ponding:* None

*Calcium carbonate, maximum content:* 40 percent

*Available water supply, 0 to 60 inches:* Moderate (about 6.6 inches)

### **Interpretive groups**

*Land capability classification (irrigated):* None specified

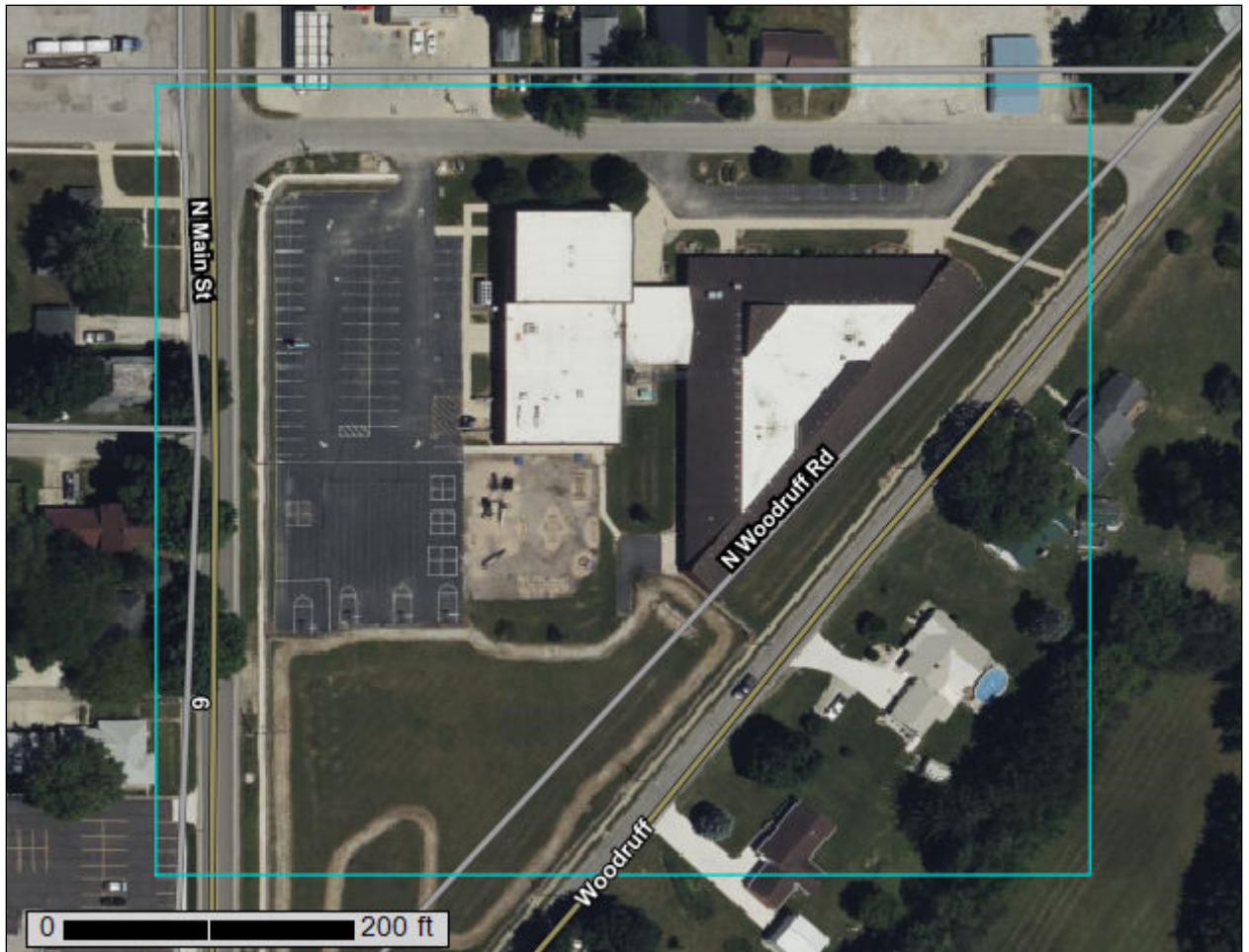
*Land capability classification (nonirrigated):* 3e

*Hydrologic Soil Group:* A

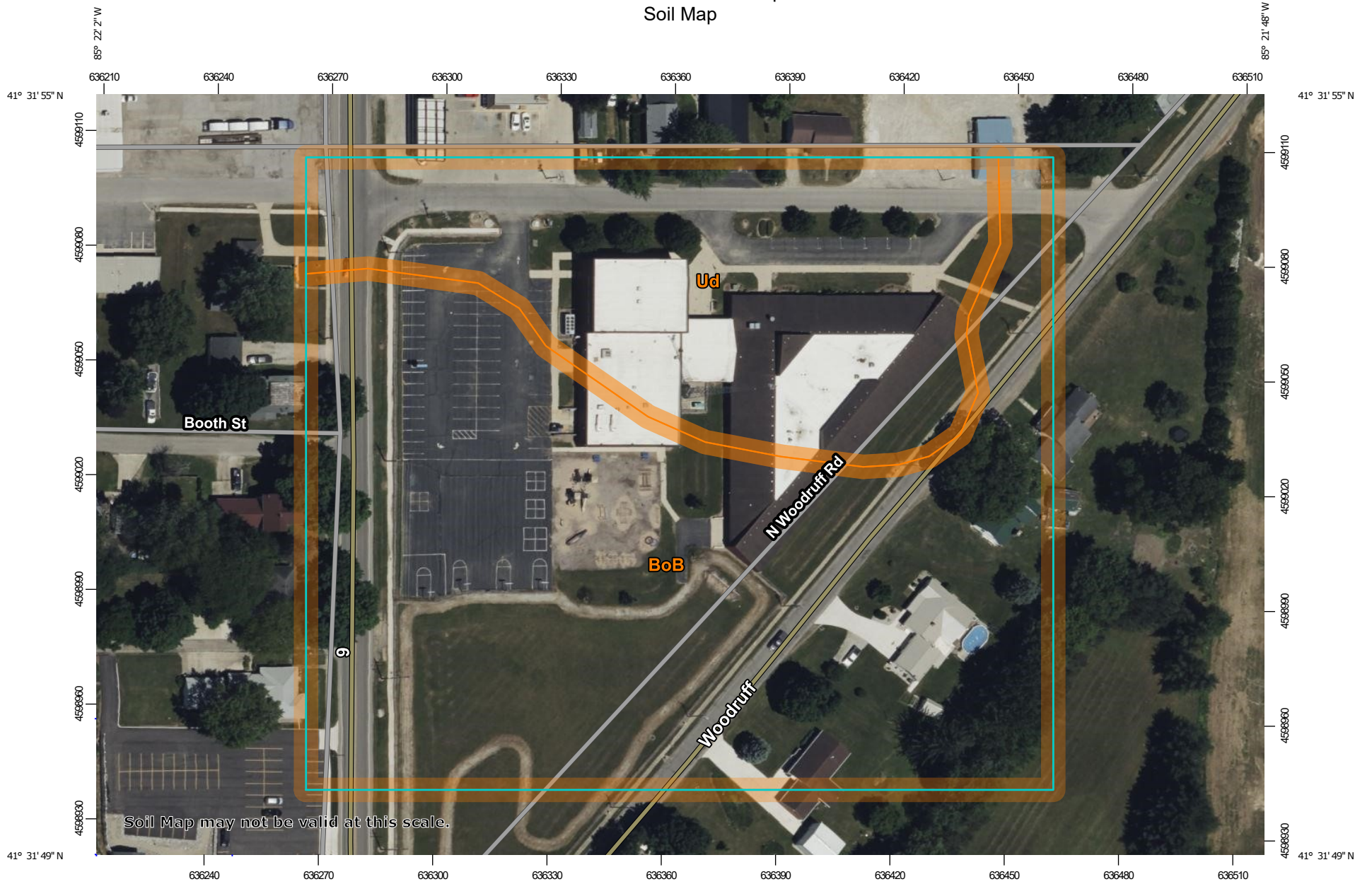
*Ecological site:* F098XA015MI - Dry Loamy Drift Plains

*Hydric soil rating:* No

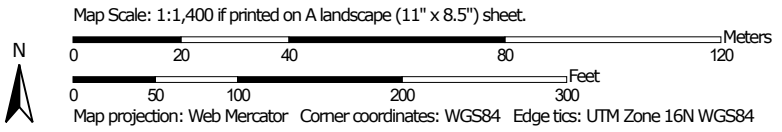
# Custom Soil Resource Report for Lagrange County, Indiana



# Custom Soil Resource Report Soil Map




Soil Map may not be valid at this scale.




### MAP LEGEND

**Area of Interest (AOI)**

 Area of Interest (AOI)




















**Soils**







 Soil Map Unit Polygons

 Soil Map Unit Lines


 Soil Map Unit Points

**Special Point Features**






-  Blowout
-  Borrow Pit
-  Clay Spot
-  Closed Depression
-  Gravel Pit
-  Gravelly Spot
-  Landfill
-  Lava Flow
-  Marsh or swamp
-  Mine or Quarry
-  Miscellaneous Water
-  Perennial Water
-  Rock Outcrop
-  Saline Spot
-  Sandy Spot
-  Severely Eroded Spot
-  Sinkhole
-  Slide or Slip
-  Sodic Spot

-  Spoil Area
-  Stony Spot
-  Very Stony Spot
-  Wet Spot
-  Other
-  Special Line Features


**Water Features**

 Streams and Canals

**Transportation**

-  Rails
-  Interstate Highways
-  US Routes
-  Major Roads
-  Local Roads

**Background**

 Aerial Photography

### MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:20,000.

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Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service  
 Web Soil Survey URL:  
 Coordinate System: Web Mercator (EPSG:3857)

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Soil Survey Area: Lagrange County, Indiana  
 Survey Area Data: Version 29, Sep 3, 2025

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Jun 18, 2022—Jun 21, 2022

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

## Map Unit Legend

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
BoB	Boyer loamy sand, 2 to 6 percent slopes	5.5	68.0%
Ud	Udorthents, loamy	2.6	32.0%
<b>Totals for Area of Interest</b>		<b>8.0</b>	<b>100.0%</b>

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## Custom Soil Resource Report

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## Lagrange County, Indiana

### BoB—Boyer loamy sand, 2 to 6 percent slopes

#### Map Unit Setting

*National map unit symbol:* 94n6  
*Elevation:* 600 to 940 feet  
*Mean annual precipitation:* 34 to 40 inches  
*Mean annual air temperature:* 47 to 51 degrees F  
*Frost-free period:* 140 to 185 days  
*Farmland classification:* Not prime farmland

#### Map Unit Composition

*Boyer and similar soils:* 100 percent  
*Estimates are based on observations, descriptions, and transects of the mapunit.*

#### Description of Boyer

##### Setting

*Landform:* Outwash plains  
*Landform position (two-dimensional):* Backslope, shoulder  
*Landform position (three-dimensional):* Side slope  
*Down-slope shape:* Linear  
*Across-slope shape:* Linear  
*Parent material:* Sandy over loamy outwash over sandy and gravelly outwash

##### Typical profile

*Ap - 0 to 10 inches:* loamy sand  
*BE - 10 to 18 inches:* loamy sand  
*Bt1-3 - 18 to 36 inches:* sandy loam  
*2C - 36 to 60 inches:* stratified gravelly coarse sand to sand

##### Properties and qualities

*Slope:* 2 to 6 percent  
*Depth to restrictive feature:* 20 to 40 inches to strongly contrasting textural stratification  
*Drainage class:* Well drained  
*Runoff class:* Low  
*Capacity of the most limiting layer to transmit water (Ksat):* High (2.00 to 6.00 in/hr)  
*Depth to water table:* More than 80 inches  
*Frequency of flooding:* None  
*Frequency of ponding:* None  
*Calcium carbonate, maximum content:* 40 percent  
*Available water supply, 0 to 60 inches:* Low (about 5.4 inches)

##### Interpretive groups

*Land capability classification (irrigated):* None specified  
*Land capability classification (nonirrigated):* 3e  
*Hydrologic Soil Group:* A  
*Ecological site:* F098XA014MI - Dry Sandy Drift Plains  
*Hydric soil rating:* No

## **Ud—Udorthents, loamy**

### **Map Unit Setting**

*National map unit symbol:* 1jg6k  
*Elevation:* 340 to 1,000 feet  
*Mean annual precipitation:* 34 to 40 inches  
*Mean annual air temperature:* 47 to 50 degrees F  
*Frost-free period:* 140 to 170 days  
*Farmland classification:* Not prime farmland

### **Map Unit Composition**

*Udorthents, loamy:* 100 percent  
*Estimates are based on observations, descriptions, and transects of the mapunit.*

### **Description of Udorthents, Loamy**

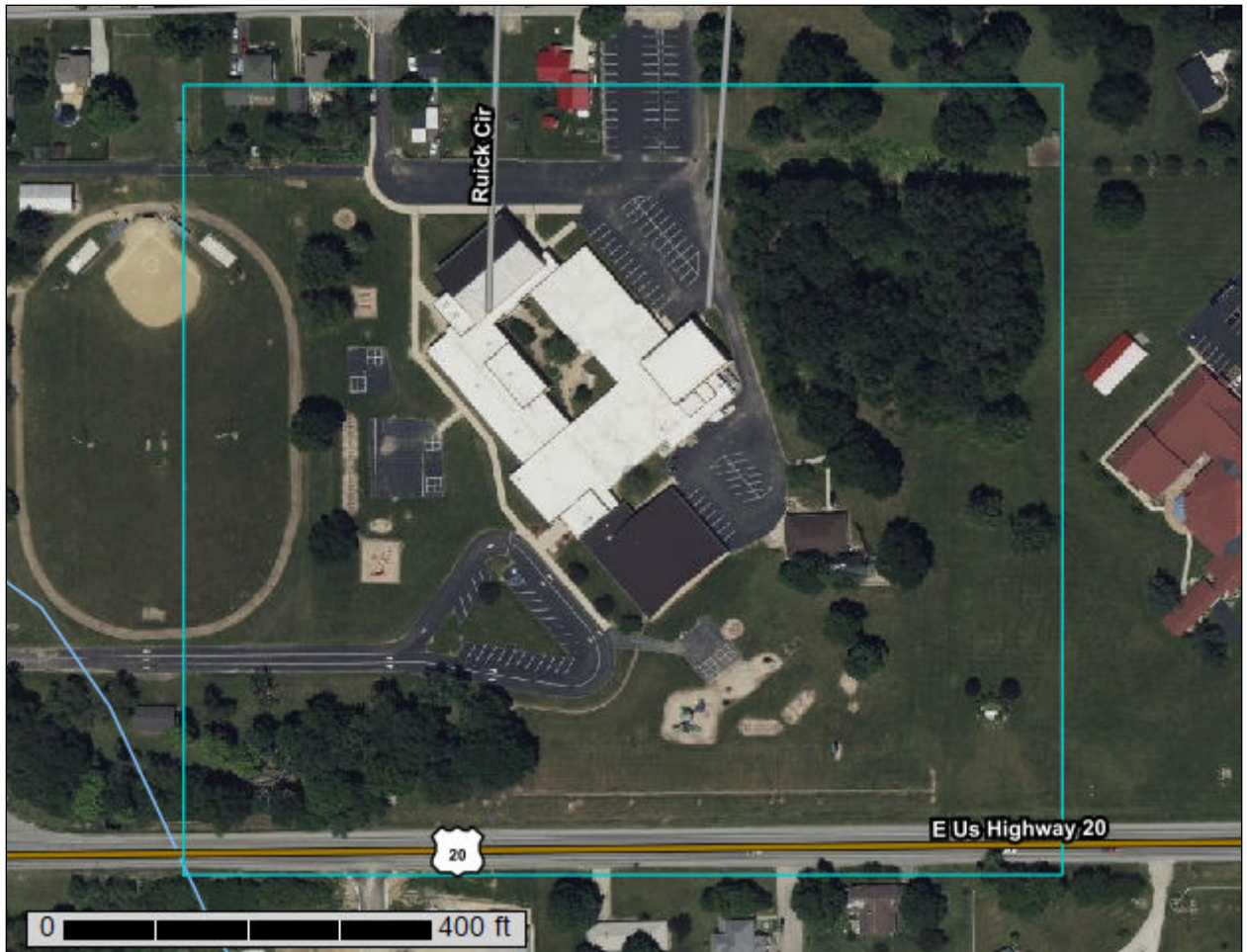
#### **Setting**

*Landform:* Moraines, Till plains  
*Landform position (two-dimensional):* Backslope, shoulder, summit  
*Landform position (three-dimensional):* Side slope  
*Down-slope shape:* Convex  
*Across-slope shape:* Convex  
*Parent material:* Loamy till

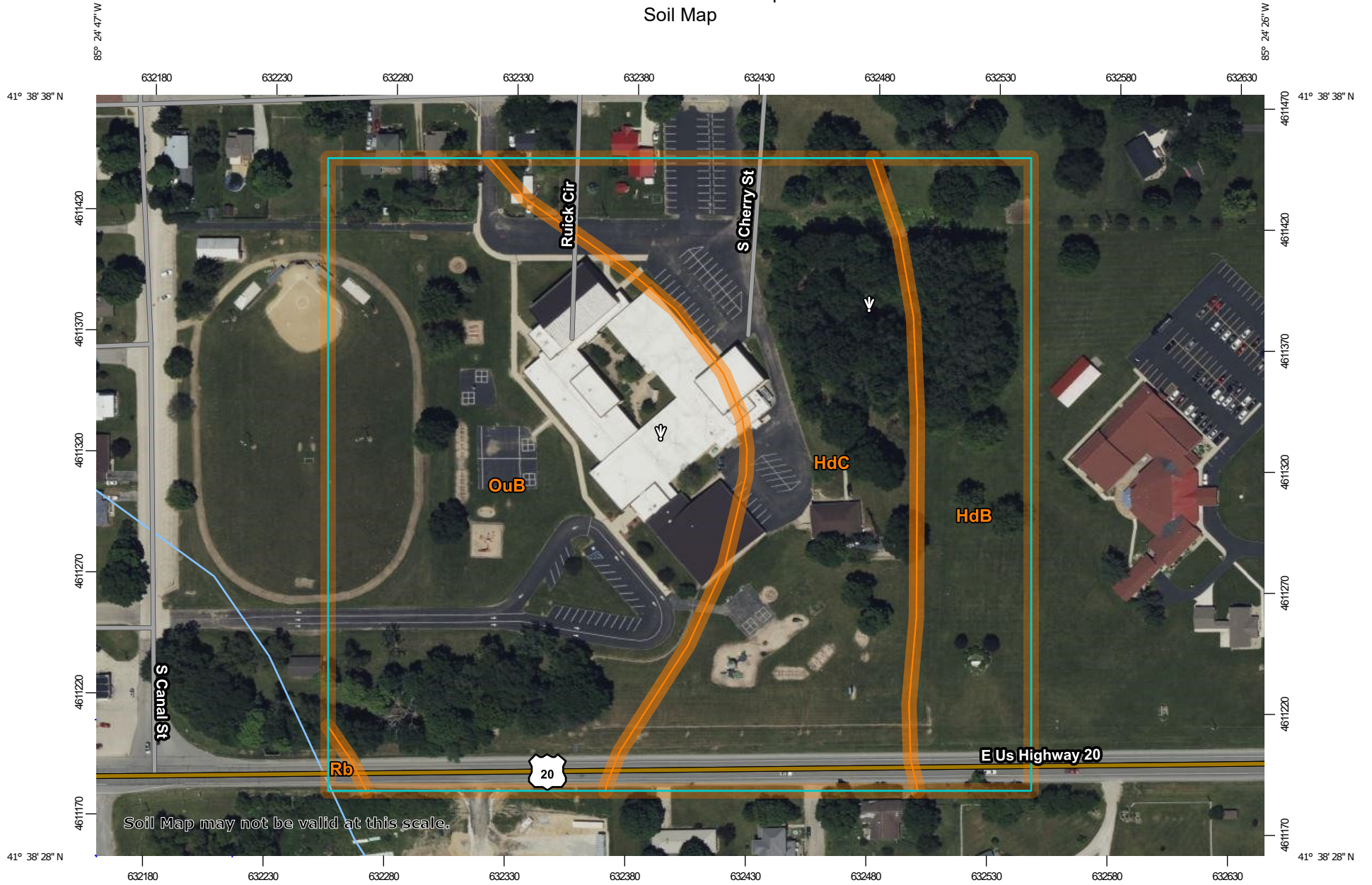
#### **Interpretive groups**

*Land capability classification (irrigated):* None specified  
*Land capability classification (nonirrigated):* 8  
*Hydric soil rating:* Unranked

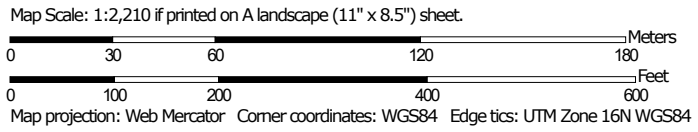
# Custom Soil Resource Report for Lagrange County, Indiana



# Custom Soil Resource Report Soil Map




Soil Map may not be valid at this scale.



### MAP LEGEND

**Area of Interest (AOI)**

 Area of Interest (AOI)




















**Soils**







 Soil Map Unit Polygons

 Soil Map Unit Lines


 Soil Map Unit Points

**Special Point Features**






-  Blowout
-  Borrow Pit
-  Clay Spot
-  Closed Depression
-  Gravel Pit
-  Gravelly Spot
-  Landfill
-  Lava Flow
-  Marsh or swamp
-  Mine or Quarry
-  Miscellaneous Water
-  Perennial Water
-  Rock Outcrop
-  Saline Spot
-  Sandy Spot
-  Severely Eroded Spot
-  Sinkhole
-  Slide or Slip
-  Sodic Spot

-  Spoil Area
-  Stony Spot
-  Very Stony Spot
-  Wet Spot
-  Other
-  Special Line Features


**Water Features**

 Streams and Canals

**Transportation**

-  Rails
-  Interstate Highways
-  US Routes
-  Major Roads
-  Local Roads

**Background**

 Aerial Photography

### MAP INFORMATION

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 Web Soil Survey URL:  
 Coordinate System: Web Mercator (EPSG:3857)

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Date(s) aerial images were photographed: Jun 18, 2022—Jun 21, 2022

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## Map Unit Legend

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
HdB	Hillsdale sandy loam, 2 to 6 percent slopes	3.3	17.2%
HdC	Hillsdale sandy loam, 6 to 12 percent slopes	6.5	34.2%
OuB	Oshtemo-Hillsdale-Chelsea complex, 3 to 6 percent slopes	9.1	48.3%
Rb	Rensselaer loam, 0 to 1 percent slopes	0.1	0.3%
<b>Totals for Area of Interest</b>		<b>18.9</b>	<b>100.0%</b>

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## Custom Soil Resource Report

The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The objective of mapping is not to delineate pure taxonomic classes but rather to separate the landscape into landforms or landform segments that have similar use and management requirements. The delineation of such segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, however, onsite investigation is needed to define and locate the soils and miscellaneous areas.

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Soils of one series can differ in texture of the surface layer, slope, stoniness, salinity, degree of erosion, and other characteristics that affect their use. On the basis of such differences, a soil series is divided into *soil phases*. Most of the areas shown on the detailed soil maps are phases of soil series. The name of a soil phase commonly indicates a feature that affects use or management. For example, Alpha silt loam, 0 to 2 percent slopes, is a phase of the Alpha series.

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Some surveys include *miscellaneous areas*. Such areas have little or no soil material and support little or no vegetation. Rock outcrop is an example.

## Lagrange County, Indiana

### HdB—Hillsdale sandy loam, 2 to 6 percent slopes

#### Map Unit Setting

*National map unit symbol:* 94nn  
*Elevation:* 600 to 940 feet  
*Mean annual precipitation:* 34 to 40 inches  
*Mean annual air temperature:* 47 to 51 degrees F  
*Frost-free period:* 140 to 185 days  
*Farmland classification:* All areas are prime farmland

#### Map Unit Composition

*Hillsdale and similar soils:* 100 percent  
*Estimates are based on observations, descriptions, and transects of the mapunit.*

#### Description of Hillsdale

##### Setting

*Landform:* Moraines, Till plains  
*Landform position (two-dimensional):* Backslope, shoulder  
*Landform position (three-dimensional):* Side slope  
*Down-slope shape:* Linear  
*Across-slope shape:* Linear  
*Parent material:* Loamy till

##### Typical profile

*Ap - 0 to 10 inches:* sandy loam  
*E - 10 to 13 inches:* sandy loam  
*Bw - 13 to 17 inches:* sandy loam  
*Bt1 - 17 to 35 inches:* sandy loam  
*Bt2 - 35 to 41 inches:* sandy loam  
*Bt3 - 41 to 50 inches:* sandy loam  
*BC - 50 to 70 inches:* sandy loam  
*C - 70 to 80 inches:* sandy loam

##### Properties and qualities

*Slope:* 2 to 6 percent  
*Depth to restrictive feature:* More than 80 inches  
*Drainage class:* Well drained  
*Runoff class:* Medium  
*Capacity of the most limiting layer to transmit water (Ksat):* Moderately high to high (0.60 to 2.00 in/hr)  
*Depth to water table:* More than 80 inches  
*Frequency of flooding:* None  
*Frequency of ponding:* None  
*Available water supply, 0 to 60 inches:* Moderate (about 8.3 inches)

##### Interpretive groups

*Land capability classification (irrigated):* None specified  
*Land capability classification (nonirrigated):* 2e  
*Hydrologic Soil Group:* A  
*Ecological site:* F111XC007IN - Glacial Ridge  
*Hydric soil rating:* No

## **HdC—Hillsdale sandy loam, 6 to 12 percent slopes**

### **Map Unit Setting**

*National map unit symbol:* 94np  
*Elevation:* 600 to 940 feet  
*Mean annual precipitation:* 34 to 40 inches  
*Mean annual air temperature:* 47 to 51 degrees F  
*Frost-free period:* 140 to 185 days  
*Farmland classification:* Not prime farmland

### **Map Unit Composition**

*Hillsdale and similar soils:* 100 percent  
*Estimates are based on observations, descriptions, and transects of the mapunit.*

### **Description of Hillsdale**

#### **Setting**

*Landform:* Moraines, Till plains  
*Landform position (two-dimensional):* Backslope, shoulder  
*Landform position (three-dimensional):* Side slope  
*Down-slope shape:* Linear  
*Across-slope shape:* Linear  
*Parent material:* Loamy till

#### **Typical profile**

*Ap - 0 to 10 inches:* sandy loam  
*E - 10 to 13 inches:* sandy loam  
*Bw - 13 to 17 inches:* sandy loam  
*Bt1 - 17 to 35 inches:* sandy loam  
*Bt2 - 35 to 41 inches:* sandy loam  
*Bt3 - 41 to 50 inches:* sandy loam  
*BC - 50 to 70 inches:* sandy loam  
*C - 70 to 80 inches:* sandy loam

#### **Properties and qualities**

*Slope:* 6 to 12 percent  
*Depth to restrictive feature:* More than 80 inches  
*Drainage class:* Well drained  
*Runoff class:* Medium  
*Capacity of the most limiting layer to transmit water (Ksat):* Moderately high to high (0.60 to 2.00 in/hr)  
*Depth to water table:* More than 80 inches  
*Frequency of flooding:* None  
*Frequency of ponding:* None  
*Available water supply, 0 to 60 inches:* Moderate (about 8.3 inches)

#### **Interpretive groups**

*Land capability classification (irrigated):* None specified  
*Land capability classification (nonirrigated):* 3e  
*Hydrologic Soil Group:* A  
*Ecological site:* F111XC007IN - Glacial Ridge

*Hydric soil rating:* No

## **OuB—Oshtemo-Hillsdale-Chelsea complex, 3 to 6 percent slopes**

### **Map Unit Setting**

*National map unit symbol:* 94p7  
*Elevation:* 600 to 940 feet  
*Mean annual precipitation:* 34 to 40 inches  
*Mean annual air temperature:* 47 to 51 degrees F  
*Frost-free period:* 140 to 185 days  
*Farmland classification:* Not prime farmland

### **Map Unit Composition**

*Oshtemo and similar soils:* 45 percent  
*Hillsdale and similar soils:* 35 percent  
*Chelsea and similar soils:* 20 percent  
*Estimates are based on observations, descriptions, and transects of the mapunit.*

### **Description of Oshtemo**

#### **Setting**

*Landform:* Moraines, Outwash plains  
*Landform position (two-dimensional):* Backslope, shoulder  
*Landform position (three-dimensional):* Side slope  
*Down-slope shape:* Linear  
*Across-slope shape:* Linear  
*Parent material:* Loamy outwash over sandy and gravelly outwash

#### **Typical profile**

*Ap - 0 to 9 inches:* loamy sand  
*Bw,Bt1,Bt2 - 9 to 36 inches:* gravelly sandy loam  
*Bt3,BC - 36 to 53 inches:* gravelly sandy loam  
*2C - 53 to 60 inches:* stratified very gravelly coarse sand to sand

#### **Properties and qualities**

*Slope:* 3 to 6 percent  
*Depth to restrictive feature:* 24 to 60 inches to strongly contrasting textural stratification  
*Drainage class:* Well drained  
*Runoff class:* Very low  
*Capacity of the most limiting layer to transmit water (Ksat):* High (2.00 to 6.00 in/hr)  
*Depth to water table:* More than 80 inches  
*Frequency of flooding:* None  
*Frequency of ponding:* None  
*Calcium carbonate, maximum content:* 40 percent  
*Available water supply, 0 to 60 inches:* Moderate (about 6.6 inches)

#### **Interpretive groups**

*Land capability classification (irrigated):* None specified  
*Land capability classification (nonirrigated):* 3e  
*Hydrologic Soil Group:* A

## Custom Soil Resource Report

*Ecological site:* F098XA015MI - Dry Loamy Drift Plains  
*Hydric soil rating:* No

### Description of Hillsdale

#### Setting

*Landform:* Outwash plains, Moraines  
*Landform position (two-dimensional):* Backslope, shoulder  
*Landform position (three-dimensional):* Side slope  
*Down-slope shape:* Linear  
*Across-slope shape:* Linear  
*Parent material:* Loamy till

#### Typical profile

*Ap - 0 to 10 inches:* sandy loam  
*E - 10 to 13 inches:* sandy loam  
*Bw - 13 to 17 inches:* sandy loam  
*Bt1 - 17 to 35 inches:* sandy loam  
*Bt2 - 35 to 41 inches:* sandy loam  
*Bt3 - 41 to 50 inches:* sandy loam  
*BC - 50 to 70 inches:* sandy loam  
*C - 70 to 80 inches:* sandy loam

#### Properties and qualities

*Slope:* 3 to 6 percent  
*Depth to restrictive feature:* More than 80 inches  
*Drainage class:* Well drained  
*Runoff class:* Low  
*Capacity of the most limiting layer to transmit water (Ksat):* Moderately high to high (0.60 to 2.00 in/hr)  
*Depth to water table:* More than 80 inches  
*Frequency of flooding:* None  
*Frequency of ponding:* None  
*Available water supply, 0 to 60 inches:* Moderate (about 8.3 inches)

#### Interpretive groups

*Land capability classification (irrigated):* None specified  
*Land capability classification (nonirrigated):* 2s  
*Hydrologic Soil Group:* A  
*Ecological site:* F098XA015MI - Dry Loamy Drift Plains  
*Hydric soil rating:* No

### Description of Chelsea

#### Setting

*Landform:* Outwash plains, Moraines  
*Landform position (two-dimensional):* Backslope, shoulder  
*Landform position (three-dimensional):* Side slope  
*Down-slope shape:* Linear  
*Across-slope shape:* Linear  
*Parent material:* Eolian sands

#### Typical profile

*Ap - 0 to 10 inches:* fine sand  
*E1,E2,A&B,C - 10 to 80 inches:* sand

#### Properties and qualities

*Slope:* 3 to 6 percent

## Custom Soil Resource Report

*Depth to restrictive feature:* More than 80 inches  
*Drainage class:* Excessively drained  
*Runoff class:* Negligible  
*Capacity of the most limiting layer to transmit water (Ksat):* High to very high (6.00 to 20.00 in/hr)  
*Depth to water table:* More than 80 inches  
*Frequency of flooding:* None  
*Frequency of ponding:* None  
*Available water supply, 0 to 60 inches:* Low (about 3.3 inches)

### **Interpretive groups**

*Land capability classification (irrigated):* None specified  
*Land capability classification (nonirrigated):* 4s  
*Hydrologic Soil Group:* A  
*Ecological site:* F098XA014MI - Dry Sandy Drift Plains  
*Hydric soil rating:* No

## **Rb—Rensselaer loam, 0 to 1 percent slopes**

### **Map Unit Setting**

*National map unit symbol:* 2wp2b  
*Landscape:* Till plains  
*Elevation:* 600 to 1,010 feet  
*Mean annual precipitation:* 34 to 40 inches  
*Mean annual air temperature:* 46 to 50 degrees F  
*Frost-free period:* 150 to 185 days  
*Farmland classification:* Prime farmland if drained

### **Map Unit Composition**

*Rensselaer and similar soils:* 85 percent  
*Minor components:* 15 percent  
*Estimates are based on observations, descriptions, and transects of the mapunit.*

### **Description of Rensselaer**

#### **Setting**

*Landscape:* Till plains  
*Landform:* Depressions  
*Landform position (three-dimensional):* Talf  
*Down-slope shape:* Linear  
*Across-slope shape:* Concave  
*Parent material:* Loamy outwash

#### **Typical profile**

*Ap - 0 to 15 inches:* loam  
*Btg1 - 15 to 38 inches:* clay loam  
*Btg2 - 38 to 42 inches:* loam  
*Cg1 - 42 to 76 inches:* stratified fine sand to silt loam  
*Cg2 - 76 to 79 inches:* loam

## Custom Soil Resource Report

### Properties and qualities

*Slope:* 0 to 1 percent  
*Depth to restrictive feature:* More than 80 inches  
*Drainage class:* Poorly drained  
*Runoff class:* Negligible  
*Capacity of the most limiting layer to transmit water (Ksat):* Moderately low to moderately high (0.06 to 0.20 in/hr)  
*Depth to water table:* About 0 to 6 inches  
*Frequency of flooding:* None  
*Frequency of ponding:* Frequent  
*Calcium carbonate, maximum content:* 25 percent  
*Maximum salinity:* Nonsaline to very slightly saline (0.0 to 2.0 mmhos/cm)  
*Available water supply, 0 to 60 inches:* High (about 10.7 inches)

### Interpretive groups

*Land capability classification (irrigated):* None specified  
*Land capability classification (nonirrigated):* 2w  
*Hydrologic Soil Group:* B/D  
*Ecological site:* R111XB401IN - Wet Outwash Mollisol, R111XC008IN - Wet Overflow  
*Hydric soil rating:* Yes

### Minor Components

#### Whitaker

*Percent of map unit:* 7 percent  
*Landscape:* Till plains  
*Landform:* Outwash plains  
*Landform position (three-dimensional):* Rise  
*Down-slope shape:* Linear  
*Across-slope shape:* Linear  
*Ecological site:* F111XB403IN - Outwash Upland  
*Hydric soil rating:* No

#### Crosier

*Percent of map unit:* 5 percent  
*Landscape:* Till plains  
*Landform:* Moraines  
*Landform position (three-dimensional):* Rise  
*Down-slope shape:* Linear  
*Across-slope shape:* Linear  
*Ecological site:* F111XB502IN - Wet Till Ridge  
*Hydric soil rating:* No

#### Houghton, undrained

*Percent of map unit:* 3 percent  
*Landscape:* Till plains  
*Landform:* Depressions  
*Landform position (three-dimensional):* Dip  
*Down-slope shape:* Concave  
*Across-slope shape:* Concave  
*Ecological site:* R111XB003IN - Deep Muck  
*Hydric soil rating:* Yes

SECTION 00 41 00  
BID FORM 96



**CONTRACTOR'S BID FOR PUBLIC WORK - FORM 96**

State Form 52414 (R2 / 2-13) / Form 96 (Revised 2013)  
Prescribed by State Board of Accounts

**PART 1**

*(To be completed for all bids. Please type or print)*

Date (month, day, year): \_\_\_\_\_

1. Governmental Unit (Owner): \_\_\_\_\_
2. County: \_\_\_\_\_
3. Bidder (Firm): \_\_\_\_\_  
     Address: \_\_\_\_\_  
     City/State/ZIPcode: \_\_\_\_\_
4. Telephone Number: \_\_\_\_\_
5. Agent of Bidder (if applicable): \_\_\_\_\_

Pursuant to notices given, the undersigned offers to furnish labor and/or material necessary to complete the public works project of LAKELAND MAINTENANCE FACILITY FOR LAKELAND SCHOOL CORPORATION in accordance with the plans and specifications prepared by krM Architecture, {GT#10000029} and dated 04.10.26 for the sum of

\_\_\_\_\_ \$ \_\_\_\_\_

The undersigned further agrees to furnish a bond or certified check with this bid for an amount specified in the notice of the letting. If alternate bids apply, the undersigned submits a proposal for each in accordance with the notice. Any addendums attached will be specifically referenced at the applicable page.

If additional units of material included in the contract are needed, the cost of units must be the same as that shown in the original contract if accepted by the governmental unit. If the bid is to be awarded on a unit basis, the itemization of the units shall be shown on a separate attachment.

The contractor and his subcontractors, if any, shall not discriminate against or intimidate any employee, or applicant for employment, to be employed in the performance of this contract, with respect to any matter directly or indirectly related to employment because of race, religion, color, sex, national origin or ancestry. Breach of this covenant may be regarded as a material breach of the contract.

**CERTIFICATION OF USE OF UNITED STATES STEEL PRODUCTS**

*(if applicable)*

I, the undersigned bidder or agent as a contractor on a public works project, understand my statutory obligation to use steel products made in the United States. (I.C. 5-16-8-2). I hereby certify that I and all subcontractors employed by me for this project will use U.S. steel products on this project if awarded. I understand that violations hereunder may result in forfeiture of contractual payments.

The undersigned acknowledges receipt of the following Addenda No.(s):

\_\_\_\_\_

Bidder agrees that this Bid shall remain in force for a period of {ch#1} consecutive calendar days from the due date, and Bids may be accepted or rejected during this period. Bids not accepted within this period shall be deemed rejected.

Attended pre-bid conference YES \_\_\_\_\_ NO \_\_\_\_\_

Has visited jobsite YES \_\_\_\_\_ NO \_\_\_\_\_

Insert number of calendar days for the duration of the construction project: \_\_\_\_\_ Days

**UNIT PRICING**

**A. PARTICULARS**

1. THE FOLLOWING ARE UNIT PRICES FOR SPECIFIC PORTIONS OF THE WORK AS LISTED, AND ARE APPLICABLE TO AUTHORIZED VARIATIONS FROM THE CONTRACT DOCUMENTS.

**B. UNIT PRICE LIST**

ITEM DESCRIPTION	UNIT	QUANTITY	UNIT VALUE
1. _____			
2. _____			

**ALTERNATE BIDS**

A blank entry or an entry of "No Bid", "N/A", or similar entry on any Alternate will cause the bid to be rejected as non-responsive only if that Alternate is selected and affects the submitted bid category.

**MARK "ADD" OR "DEDUCT" FOR EACH ALTERNATIVE**

Alternate Bid No. 1 - ADD - WASH BAY

Change the Base Bid the sum of

\_\_\_\_\_

(sum in words)

\_\_\_\_\_ DOLLARS (\$) \_\_\_\_\_ ) ADD / DEDUCT

Alternate Bid No. 2 - DEDUCT - (ONE) PASS THROUGH BAY

Change the Base Bid the sum of

\_\_\_\_\_

(sum in words)

\_\_\_\_\_ DOLLARS (\$) \_\_\_\_\_ ) ADD / DEDUCT

Alternate Bid No. 3 - ADD - ROOF INSULATION PANEL

Change the Base Bid the sum of

\_\_\_\_\_

(sum in words)

\_\_\_\_\_ DOLLARS (\$) \_\_\_\_\_ ) ADD / DEDUCT

Alternate Bid No. 4 - ADD - SITE DRIVE MODIFICATION

Change the Base Bid the sum of

\_\_\_\_\_

(sum in words)

\_\_\_\_\_ DOLLARS (\$\_\_\_\_\_ ) ADD / DEDUCT

Alternate Bid No. 5 - [ADD - REMOVE REMAINDER OF BERM - ONLY TO BE ACCEPTED  
ALONG WITH ALTERNATE NO. 4]

Change the Base Bid the sum of

\_\_\_\_\_

(sum in words)

\_\_\_\_\_ DOLLARS (\$\_\_\_\_\_ ) ADD / DEDUCT

**ACCEPTANCE**

The above bid is accepted this \_\_\_\_\_ day of \_\_\_\_\_, \_\_\_\_\_, subject to the following conditions: \_\_\_\_\_

Contracting Authority Members:


**PART II**

*(For projects of \$150,000 or more - IC 36-1-12-4)*

Governmental Unit: \_\_\_\_\_

Bidder (Firm): \_\_\_\_\_

Date (month, day, year): \_\_\_\_\_

These statements to be submitted under oath by each bidder with and as a part of his bid. Attach additional pages for each section as needed.

**SECTION I EXPERIENCE QUESTIONNAIRE**

1. What public works projects has your organization completed for the period of one (1) year prior to the date of the current bid?

Contract Amount	Class of Work	Completion Date	Name and Address of Owner

2. What public works projects are now in process of construction by your organization?

Contract Amount	Class of Work	Expected Completion Date	Name and Address of Owner

3. Have you ever failed to complete any work awarded to you? \_\_\_\_\_ If so, where and why?

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A.

4. List references from private firms for which you have performed work.

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**SECTION II PLAN AND EQUIPMENT QUESTIONNAIRE**

1. Explain your plan or layout for performing proposed work. (Examples could include a narrative of when you could begin work, completed the project, number of workers, etc. and any other information which you believe would enable the governmental unit to consider your bid.)

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B.

2. Please list the names and addresses of all subcontractors (i.e. persons or firms outside your own firm who have performed part of the work) that you have used on public works projects during the past five (5) years along with a brief description of the work done by each subcontractor.

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3. If you intend to sublet any portion of the work, state the name and address of each subcontractor, equipment to be used by the subcontractor, and whether you expect to require a bond. However, if you are unable to currently provide a listing, please understand a listing must be provided prior to contract approval. Until the completion of the proposed project, you are under a continuing obligation to immediately notify the governmental unit in the event that you subsequently determine that you will use a subcontractor on the proposed project.

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4. What equipment do you have available to use for the proposed project? Any equipment to be used by subcontractors may also be required to be listed by the governmental unit.

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5. Have you entered into contracts or received offers for all materials which substantiate the prices used in preparing your proposal? If not, please explain the rationale used which would corroborate the prices listed.

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**SECTION III CONTRACTOR'S FINANCIAL STATEMENT**

Attachment of bidder's financial statement is mandatory. Any bid submitted without said financial statement as required by statute shall thereby be rendered invalid. The financial statement provided hereunder to the governing body awarding the contract must be specific enough in detail so that said governing body can make a proper determination of the bidder's capability for completing the project if awarded.

**SECTION IV CONTRACTOR'S NON - COLLUSION AFFIDAVIT**

The undersigned bidder or agent, being duly sworn on oath, says that he has not, nor has any other member, representative, or agent of the firm, company, corporation or partnership represented by him, entered into any combination, collusion or agreement with any person relative to the price to be bid by anyone at such letting nor to prevent any person from bidding nor to include anyone to refrain from bidding, and that this bid is made without reference to any other bid and without any agreement, understanding or combination with any other person in reference to such bidding.

He further says that no person or persons, firms, or corporation has, have or will receive directly or indirectly, any rebate, fee, gift, commission or thing of value on account of such sale.

**SECTION V OATH AND AFFIRMATION**

I HEREBY AFFIRM UNDER THE PENALTIES FOR PERJURY THAT THE FACTS AND INFORMATION CONTAINED IN THE FOREGOING BID FOR PUBLIC WORKS ARE TRUE AND CORRECT.

Dated at \_\_\_\_\_ this \_\_\_\_\_ day of \_\_\_\_\_, \_\_\_\_\_.

\_\_\_\_\_  
*(Name of Organization)*

By \_\_\_\_\_

\_\_\_\_\_  
*(Title of Person Signing)*

**ACKNOWLEDGEMENT**

STATE OF \_\_\_\_\_)

) SS

COUNTY OF \_\_\_\_\_)

Before me, a Notary Public, personally appeared the above-named \_\_\_\_\_ and

swore that the statements contained in the foregoing document are true and correct.

Subscribed and sworn to before me this \_\_\_\_\_ day of \_\_\_\_\_, \_\_\_\_\_.

\_\_\_\_\_  
*Notary Public*

My Commission Expires: \_\_\_\_\_

County of Residence: \_\_\_\_\_

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BID OF

---

(Contractor)

---

(Address)

---

FOR  
PUBLIC WORKS PROJECTS  
OF

---

Filed \_\_\_\_\_

---

Action taken \_\_\_\_\_

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END OF SECTION

**SECTION 01 23 00  
ALTERNATES**

**PART 1 GENERAL**

**1.01 SECTION INCLUDES**

- A. Description of Alternates.

**1.02 RELATED REQUIREMENTS**

- A. Document 00 21 13 - Instructions to Bidders: Instructions for preparation of pricing for Alternates.

**1.03 ACCEPTANCE OF ALTERNATES**

- A. Alternates quoted on Bid Forms will be reviewed and accepted or rejected at Owner's option. Accepted Alternates will be identified in the Owner-Contractor Agreement.

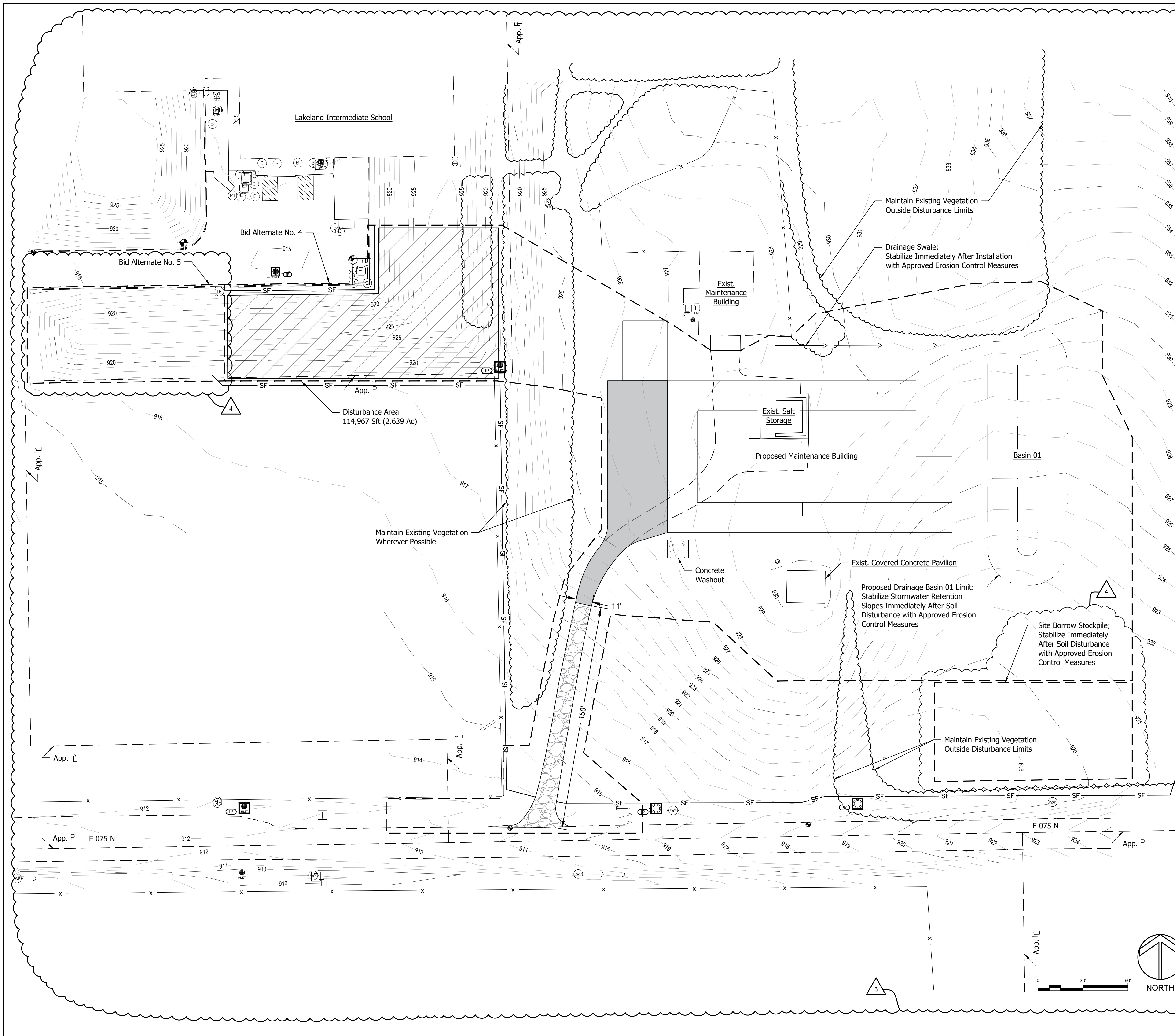
**1.04 SCHEDULE OF ALTERNATES**

- A. Alternate No. 1 - ADD - WASH BAY :
1. Provide the change in price to ADD, in it's entirety the Wash Bay between Columns 7 and 8 as shown on the Floor plans A1-1, Roofing and Elevation Sheets A4-1. Work includes but not limited to the PEMB, Garage door, Slab and foundation work, and inteior finish and MEP build out. Price should include addition of door along column line 7 into the wash bay.
  2. Civil portion - add additional excavation, aggregate material, and concrete apron necessary to support the half bay structure at the southeast corner of the maintenance barn site, refer to Civil Drawings for additional scope for this alternate.
- B. Alternate No. 2 - DEDUCT - ONE PASS THROUGH BAY:
1. Provide the change in price to DEDUCT in it's entirety the a PASS THROUGH BAY, between Columns 5 and 6 as shown on the Floor plans A1-1, Roofing and Elevation Sheets A4-1. Work includes but not limited to the PEMB, (2) Garage door, Slab and foundation work, and inteior finish and MEP build out removal.
- C. Alternate No. 3 - ADD - ROOF INSULATION PANEL:
1. Provide the change in price to ADD a higher grade Insulation Panel.
  2. Base Bid: Vapor barrier faced fiberglass insulation. Min R-19.
  3. Alternate: Energy Saver Roof insulation System.
  4. The alternate is noted on all of the walls sections on A6-1 & A6-2
- D. Alternate No. 4 - ADD - SITE DRIVE MODIFICATION, ADD - SITE DRIVE MODIFICATION  
ADD - SITE DRIVE MODIFICATION:
1. Provide a change in price to ADD the additional excavation and aggregate material necessary to construct the drive connecting the maintenance barn site with the LIS drive / parking lot as shown on the Civil Drawings
- E. Alternate No. 5 - REMOVE REMAINING BERM AND PLACE FILL AT TEMPORARY  
LOCATION ON SITE:
1. Provide a change in price to REMOVE the remaining berm in as an additional alternate to be accepted with Alternate No. 4. The remaining berm is to be relocated per Civil Drawings on the site as a temporary location that would allow the moved material to be easily accessed and could stay in its location as long as necessary.

**PART 2 PRODUCTS - NOT USED**

**PART 3 EXECUTION - NOT USED**

**END OF SECTION**



**GENERAL NOTES**

- Contractor shall field verify all existing conditions. If discrepancies are discovered by Contractor, Contractor shall notify Owner and Engineer immediately for coordination to remedy discrepancies.
- Contractor shall install additional inlet protection as required for proposed storm water structures.
- Refer to Erosion Control Notes and Details Sheet (C1-011) for Erosion Control and Construction Sequence Notes in compliance with the Indiana Stormwater Quality Manual.
- All excavation and site grading shall comply with local, state, and federal regulations, including required IDEM CSGP, prior to the start of construction.
- All erosion control measures must comply with the required IDEM CSGP, Indiana Stormwater Quality Manual, and local ordinances.
- Contractor shall install temporary silt fence barriers to contain runoff around stockpiled material.
- Contractor shall stockpile topsoil separately for use as directed by Owner.
- Dust control shall comply with the required IDEM CSGP, Indiana Stormwater Quality Manual, and local air quality standards.
- All disturbed areas shall be stabilized with seed, mulch, or approved methods within 7 days of achieving final grade.
- Vegetation must achieve 70% density per 2 ft by 2 ft area coverage for permanent stabilization per IDEM CSGP requirements.
- Access points shall be maintained on an IDEM CSGP approved calendar cycle. A stabilized construction entrance free of mud and debris shall be used to minimize off-site tracking per plan.
- Any discovery of hazardous materials during grading activities must be immediately reported to the Owner, Engineer, and all appropriate authorities regardless of time of day or day of week.
- Work in areas affected by hazardous materials must cease until further instructions are provided by Owner.

**LEGEND**

- 000— Existing Major Contour
- 000--- Existing Minor Contour
- Ditch Flowline
- Top of Bank / Toe of Slope; Refer to Sheet C2-100 for Details
- Tree Line
- x Fence Line
- - - Disturbance Limits
- SF Silt Fence; Refer to Detail 1/C1-011
- Inlet Protection; Refer to Detail 3/C1-011
- Construction Entrance; Refer to Detail 2/C1-011
- Construction Staging
- Concrete Washout; Refer to Detail 4/C1-011
- Electric Box
- Round Inlet
- Electric Transformer
- Gas Meter
- Fire Hydrant
- Iron Pin Found
- Manhole
- Storm Riser
- Power Pole
- Sign
- Telephone Pedestal
- Water Meter
- Water Spigot
- Water Valve
- Clean out
- Guy Anchor
- Bench Mark
- Horizontal Control



**REVISIONS**

- 3 Addendum #3, 4/30/2026
- 4 Addendum #4, 5/05/2026

12/17/25  
 LAKELAND SCHOOL CORPORATION  
**25082 - PROJECT B.1 - MAINTENANCE BUILDING**  
 E 075 N, LAGRANGE, IN 46761  
 BID SET

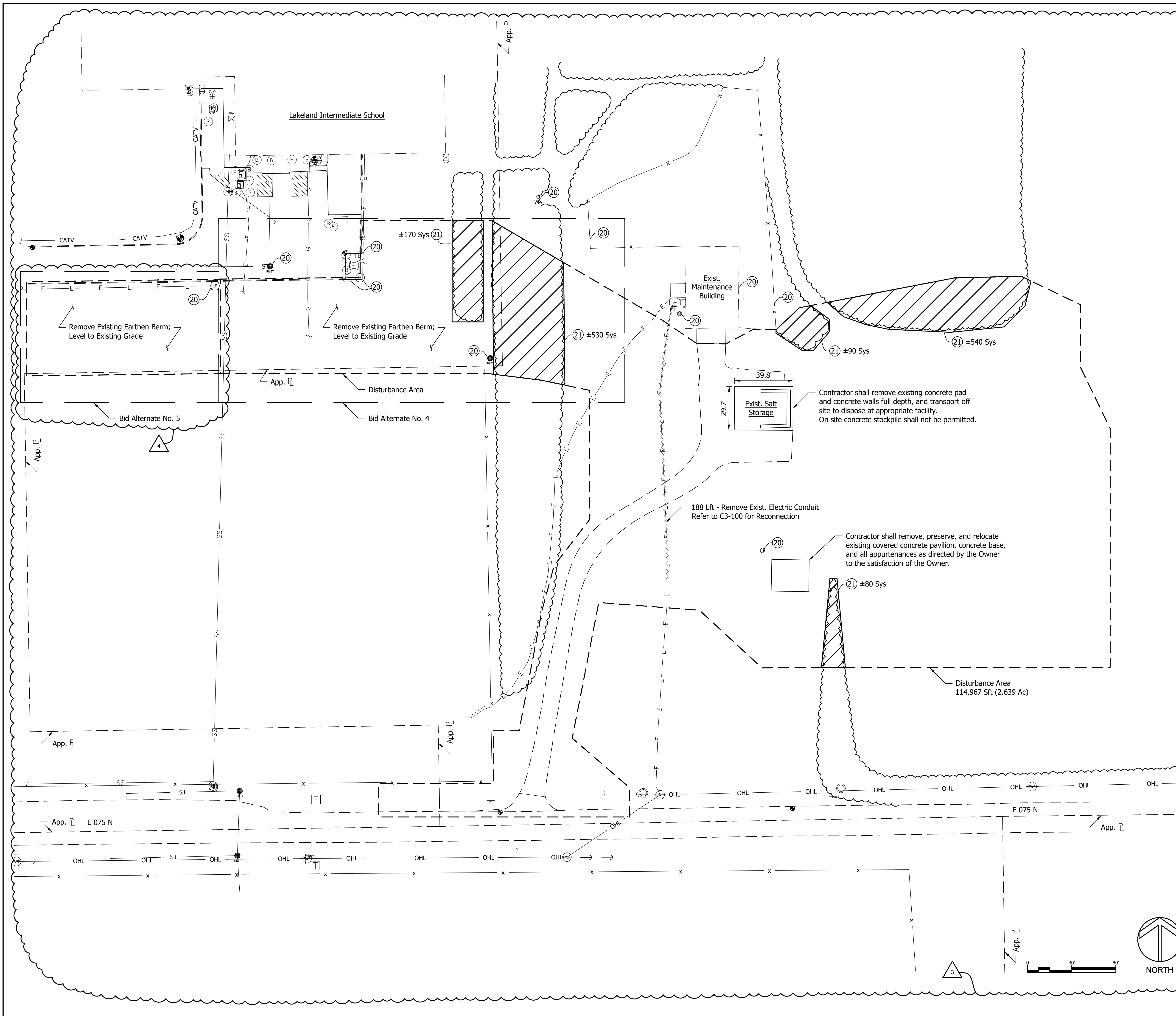


04/30/2026  
*Ryan J. Anderson*

BID SET  
 4/30/26  
 kM JOB NO.  
 25082  
 DRAWN BY  
 BWC

DRAWING NAME  
**EROSION CONTROL PLAN**

DRAWING NO.  
**C1-010**



**DEMOLITION NOTES**

- Contractor shall call Indiana 811 or submit an Online Ticket with Exactix to begin the process for locating utilities and wait the required number of working days for utility locates prior to any excavation work. Protecting marked and unmarked utility locations are the sole responsibility of the Contractor. Damages to utilities shall be remedied by Contractor at the discretion of the Owner to the satisfaction of the Owner at no additional cost to the Owner.
- All edges of disturbed pavement to be saw cut, full depth, to achieve clean, straight lines between existing and new pavement.
- Contractor shall protect existing conditions, including pavement and subgrade facilities, not marked for demolition. Damages to existing conditions not marked for demolition shall be remedied by Contractor at the discretion of the Owner to the satisfaction of the Owner at no additional cost to the Owner.
- Contractor shall field verify all existing conditions. If discrepancies are discovered by Contractor, Contractor shall notify Owner and Engineer immediately for coordination to remedy discrepancies.
- Contractor shall dispose of removed materials on a continuous and daily basis as needed. An accumulation of trash debris will not be permitted.
- The Contractor shall coordinate with Owner regarding the sequencing and timing of all utility disconnections and relocations prior to commencing work. The Contractor shall provide the Owner with adequate advance notice of anticipated utility work to allow for proper scheduling and notification to utility providers and affected parties. The Contractor shall schedule utility interruptions to minimize disruption to existing operations.

**KEYNOTES**

- (20) Protect Existing Structure
- (21) Remove Existing Vegetation; Contractor shall coordinate with Owner prior to tree removal.

**LEGEND**

- Disturbance Limits
- Bid Alternate Limits
- Utility Removal Limits
- ST --- Storm Sewer
- SS --- Sanitary Sewer
- E --- Underground Electric
- CATV --- Underground Cable TV
- G --- Underground Gas
- W --- Underground Water
- OHL --- Overhead Line
- Tree Line
- X --- Fence Line
- ⊠ Electric Box
- Round Inlet
- ⊠ Electric Transformer
- ⊠ Gas Meter
- ⊠ Fire Hydrant
- ⊠ Manhole
- ⊠ Storm Riser
- ⊠ Power Pole
- ⊠ Sign
- ⊠ Telephone Pedestal
- ⊠ Water Meter
- ⊠ Water Spigot
- ⊠ Water Valve
- ⊠ Clean out
- ⊠ Guy Anchor
- ⊠ Bench Mark



REVISIONS

- 3 Addendum #3, 4/30/2026
- 4 Addendum #4, 5/05/2026

12/17/25  
 LAKELAND SCHOOL CORPORATION  
**25082 - PROJECT B.1 - MAINTENANCE BUILDING**  
 E 075 N, LAGRANGE, IN 46761  
 BID SET

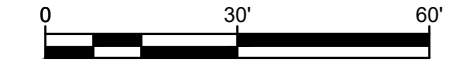
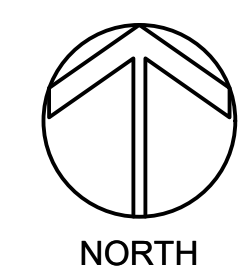


*Ryan J. Anderson*

BID SET  
 4/30/26  
 kM JOB NO.  
 25082  
 DRAWN BY  
 BWC

DRAWING NAME  
**SITE DEMOLITION  
 OVERALL PLAN**

DRAWING NO.  
**C1-020**





### BASIN 01

Elev. (ft)	Elev. Change (ft)	Area (sft)	Avg. Area (sft)	Volume (cft)
927.0		8663.0		
926.0	1.0	7074.0	7868.5	7868.5
925.0	1.0	5586.0	6330.0	6330.0
924.0	1.0	4197.0	4891.5	4891.5
923.0	1.0	2910	3553.5	3553.5
922.0	1.0	1723	2316.5	2316.5
		<b>Total Volume (cft) =</b>	<b>17091.5</b>	
		<b>6% Siltation Factor (cft) =</b>	<b>1025.5</b>	
		<b>Total Available Storage (cft) =</b>	<b>16066.0</b>	
		<b>Surplus Storage Volume (cft) =</b>	<b>3593.9</b>	

### CATCHMENT 1 STORAGE REQUIRED

**Rational Method:**  
Overall Area = 48689 sft

**Storm Duration:**  
24 Hour, 100 Year Storm

**Runoff Coefficient:**

Building	C <sub>1</sub> =	1.00
Pavement	C <sub>2</sub> =	0.98
Gravel	C <sub>3</sub> =	0.80
Lawn	C <sub>4</sub> =	0.20

**Surface Area:**

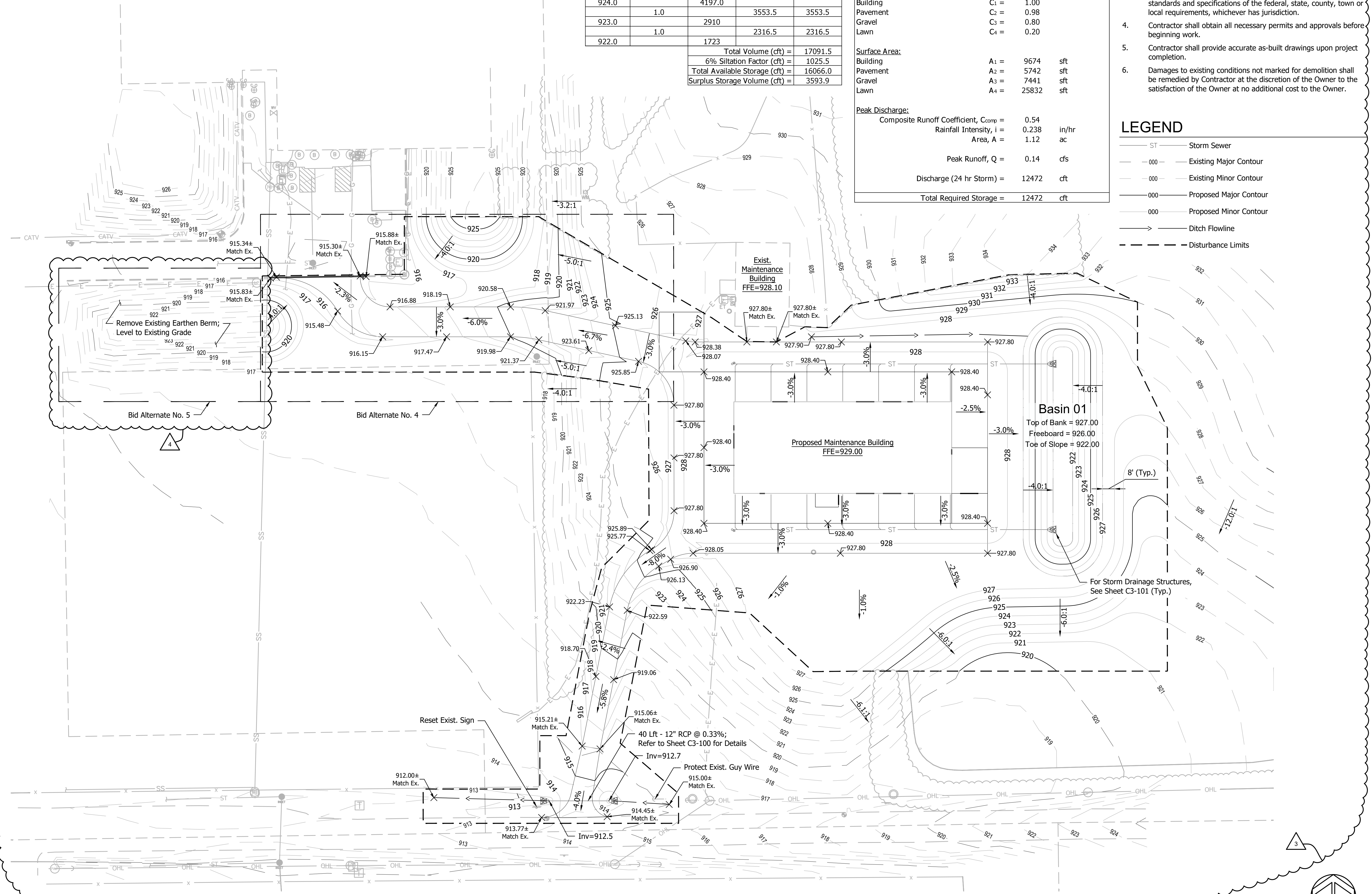
Building	A <sub>1</sub> =	9674	sft
Pavement	A <sub>2</sub> =	5742	sft
Gravel	A <sub>3</sub> =	7441	sft
Lawn	A <sub>4</sub> =	25832	sft

**Peak Discharge:**

Composite Runoff Coefficient, C <sub>comp</sub>	=	0.54	
Rainfall Intensity, i	=	0.238	in/hr
Area, A	=	1.12	ac
Peak Runoff, Q	=	0.14	cfs
Discharge (24 hr Storm)	=	12472	cft
<b>Total Required Storage</b>	=	<b>12472</b>	<b>cft</b>

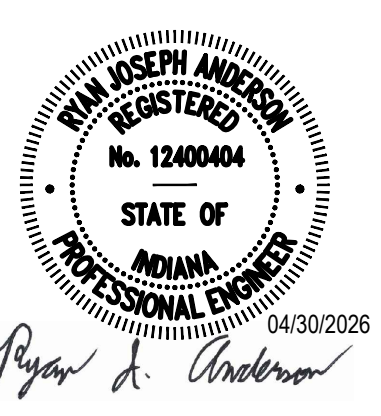
- ### GENERAL NOTES
- Refer to Architectural drawings for building dimensions and additional information.
  - Contractor shall field verify all existing conditions. If discrepancies are discovered by Contractor, Contractor shall notify Owner and Engineer immediately for coordination to remedy discrepancies.
  - All construction methods and materials must conform to current standards and specifications of the federal, state, county, town or local requirements, whichever has jurisdiction.
  - Contractor shall obtain all necessary permits and approvals before beginning work.
  - Contractor shall provide accurate as-built drawings upon project completion.
  - Damages to existing conditions not marked for demolition shall be remedied by Contractor at the discretion of the Owner to the satisfaction of the Owner at no additional cost to the Owner.

- ### LEGEND
- ST — Storm Sewer
  - 000 — Existing Major Contour
  - 000 — Existing Minor Contour
  - 000 — Proposed Major Contour
  - 000 — Proposed Minor Contour
  - Ditch Flowline
  - - - Disturbance Limits



- ### REVISIONS
- 3 Addendum #3, 4/30/2026
  - 4 Addendum #4, 5/05/2026

12/17/25  
LAKELAND SCHOOL CORPORATION  
**25082 - PROJECT B.1 - MAINTENANCE BUILDING**  
E 075 N. LAGRANGE, IN 46761  
BID SET



BID SET  
4/30/26  
kM JOB NO.  
25082  
DRAWN BY  
BWC

DRAWING NAME  
**SITE GRADING  
AND DRAINAGE  
PLAN**

DRAWING NO.  
**C2-100**



**SPECIALTY STRUCTURAL ENGINEERING**

- A SPECIALTY STRUCTURAL ENGINEER (SSE) IS DEFINED AS A PROFESSIONAL ENGINEER LICENSED IN THE STATE OF THE PROJECT LOCATION, NOT THE STRUCTURAL ENGINEER OF RECORD (SER), WHO PERFORMS STRUCTURAL ENGINEERING FUNCTIONS NECESSARY FOR THE STRUCTURE TO BE COMPLETED AND HAS SHOWN EXPERIENCE AND/OR TRAINING IN THE SPECIFIC SPECIALTY.
- IT IS THE SSE'S RESPONSIBILITY TO REVIEW THE CONSTRUCTION DRAWINGS AND SPECIFICATIONS TO DETERMINE THE APPROPRIATE SCOPE OF ENGINEERING.
- IT IS THE INTENT OF THE DRAWINGS AND SPECIFICATIONS TO PROVIDE SUFFICIENT INFORMATION FOR THE SSE TO PERFORM THEIR DESIGN AND ANALYSIS. IF THE SSE DETERMINES THERE ARE DETAILS, FEATURES, OR UNANTICIPATED PROJECT LIMITS WHICH CONFLICT WITH THE ENGINEERING REQUIREMENTS AS DESCRIBED IN THE PROJECT DOCUMENTS, THE SSE SHALL CONTACT THE SER IN A TIMELY MANNER FOR RESOLUTION OF CONFLICTS.
- THE SSE SHALL FORWARD DOCUMENTS TO THE SER FOR REVIEW. SUCH DOCUMENTS SHALL BEAR THE STAMP OF THE SSE AND INCLUDE:
  - DRAWINGS INTRODUCING ENGINEERING INPUT, SUCH AS DEFINING THE CONFIGURATION OR STRUCTURAL CAPACITY OF STRUCTURAL COMPONENTS AND/OR THEIR ASSEMBLY INTO STRUCTURAL SYSTEMS.
  - CALCULATIONS.
  - COMPUTER PRINTOUTS, WHICH ARE AN ACCEPTABLE SUBSTITUTE FOR MANUAL CALCULATIONS PROVIDED THEY ARE ACCOMPANIED BY SUFFICIENT DESIGN PARAMETERS AND IDENTIFIED INPUT AND OUTPUT INFORMATION TO PERMIT THEIR PROPER EVALUATION. SUCH INFORMATION SHALL BEAR THE STAMP OF THE SSE AS AN INDICATION THAT SSE HAS ACCEPTED RESPONSIBILITY FOR THE RESULTS.
- CONTRACTORS ARE REFERRED TO THE SPECIFIC TECHNICAL SPECIFICATION SECTIONS AND THE STRUCTURAL DRAWINGS FOR THOSE ELEMENTS REQUIRING SPECIALTY STRUCTURAL ENGINEERING. EXAMPLES OF COMPONENTS REQUIRING SPECIALTY STRUCTURAL ENGINEERING INCLUDE, BUT ARE NOT LIMITED TO THE FOLLOWING:
  - SPECIALTY FOUNDATION SYSTEMS.
  - TEMPORARY AND PERMANENT RETENTION SYSTEMS.
  - TEMPORARY AND PERMANENT DEWATERING SYSTEMS.
  - COLD-FORMED STEEL FRAMING.
  - PREFABRICATED WOOD TRUSSES.
  - PREFABRICATED WOOD JOISTS.
  - PRE-ENGINEERED WOOD POLE BARN STRUCTURES.
  - FALL RESTRAINT SYSTEMS.
  - HANDRAILS AND GUARDS.
  - LADDERS.
  - ANCHORAGE FOR MECHANICAL/ELECTRICAL/PLUMBING SYSTEMS.
  - EQUIPMENT SUPPORTS.
- WHEN MODIFICATIONS ARE PROPOSED TO ELEMENTS UNDER THE DESIGN AND CERTIFICATION OF THE SSE, WRITTEN AUTHORIZATION BY THE SSE MUST BE OBTAINED AND SUBMITTED TO THE SER FOR REVIEW, PRIOR TO PERFORMING THE PROPOSED MODIFICATION.

**WOOD FRAMING NOTES**

- FOR WOOD CONNECTIONS NOT SPECIFICALLY NOTED OR DETAILED, FOLLOW THE REQUIREMENTS OF IBC 2012 TABLE 2304.9.1 OR ESR 1539.
- WHERE A SPECIALTY STRUCTURAL ENGINEER DESIGNS WALL OR ROOF FRAMING (SUCH AS TRUSSES OR PRE-ENGINEERED POLE BARN STRUCTURES), THE DESIGNER SHALL SUBMIT STAMPED DRAWINGS FOR REVIEW AS INDICATED IN THE SPECIALTY STRUCTURAL ENGINEER NOTES.
- ALL NAILS ARE COMMON NAILS UNLESS NOTED OTHERWISE. ALL NAILS SHALL BE CAREFULLY DRIVEN AND NOT OVERDRIVEN. SUBMIT ALL PROPOSED FASTENERS FOR APPROVAL PRIOR TO CONSTRUCTION. INSTALLATION OF ALL FASTENERS SHALL MEET THE REQUIREMENTS OF NDS AND ISANTA GUIDELINES, INCLUDING THOSE IN ESR 1539, AND SECTION 2303.5 OF THE IBC.
- REFER TO FRAMING PLANS FOR SIZE, SPACING, AND SPECIES OF WALL STUDS AND PLATES NOT ASSOCIATED WITH THE PRE-ENGINEERED STRUCTURE BY OTHERS. IF NOT SHOWN OTHERWISE, STUDS AND PLATES ARE TO BE #1 OR #2 SPRUCE-PINE-FIR (SPF) WITH STUD SPACING 16" O.C. MAXIMUM. IF NOT SHOWN OTHERWISE, BEARING WALL HEADERS ARE TO BE #2 SOUTHERN PINE (SYP).
- AT THE CONTRACTOR'S DISCRETION, STUDS IN NON-LOAD BEARING INTERIOR WALLS MAY BE PREMIUM STUD GRADE SPACED AT 16" O.C. ON ALL LEVELS.
- FASTEN DOUBLE (DBL) STUDS TOGETHER WITH 0.131" X 3" NAILS AT 8" O.C. UNLESS NOTED OTHERWISE. FOR MORE THAN TWO STUDS, FASTEN IN THE SAME WAY, NAILING AS EACH STUD IS ADDED.
- SEE THE HEADER SCHEDULE FOR ALL HEADER SIZES AND MATERIALS. ALL HEADERS IN NON-LOAD-BEARING INTERIOR WALLS ARE TO BE (2) 2X4 #2 SPF FOR OPENINGS UP TO 4'-0" AND (2) 2X6 #2 SPF FOR OPENINGS OVER 4'-0". ALL HEADERS IN NON-LOAD-BEARING WALLS TO HAVE (1) JACK STUD AT EACH END.
- REFER TO THE SHEAR WALL SCHEDULE FOR SHEATHING, NAILING, STRAP TIES, HOLD DOWNS, ETC. REQUIRED FOR WOOD-PANEL-SHEATHED AND GYPSUM-WALLBOARD-SHEATHED SHEAR WALLS.
- USE DOUBLE TOP PLATES ON ALL WALLS, INCLUDING NON-LOAD-BEARING WALLS, WITH ALL SPLICES AND CORNERS LAPPED AT 1" INTERSECTIONS DO NOT LAP TOP PLATE OF INTERSECTING WALL CUTTING THE TOP PLATE OF THE CONTINUOUS WALL, RATHER USE A METAL TIE PLATE AS DESCRIBED IN THE EXCEPTION TO SECTION 2308.9.2.1 OF THE IBC.
- UNLESS OTHERWISE NOTED ON PLAN OR DETAIL, ANCHOR WALL PLATES TO FOUNDATIONS AND/OR SUPPORTING STRUCTURE USING SIMPSON STRONG-TIE TITEN HD HEAVY DUTY SCREW ANCHORS, 5/8" DIAMETER WITH MINIMUM 5" EMBEDMENT. SPACE ANCHORS AT 48" O.C. FOR LOAD-BEARING AND NON-LOAD-BEARING WALLS. REDUCE SPACING TO 24" O.C. FOR ALL SHEAR WALLS.
- COORDINATE FINAL FLOOR AND ROOF FRAMING INCLUDING JOIST OR TRUSS LAYOUT & TRUSS MEMBER CONFIGURATION WITH MECHANICAL, ELECTRICAL, & PLUMBING (MEP) DRAWINGS. OBTAIN ADDITIONAL MEP INFORMATION AS NEEDED FOR COMPLETE COORDINATION. KEEP ALL MECHANICAL CHASES FREE OF FRAMING. DO NOT LOCATE JOISTS OR TRUSSES AT PARALLEL PLUMBING WALLS.
- ALL EXPOSED FRAMING TO BE PRESURE PRESERVATIVE TREATED WOOD (PPT) AS DESCRIBED IN THE SPECIFICATIONS. ALL PPT WOOD TO BE KLN DRIED AFTER TREATMENT (KDAT). HARDWARE USED WITH PPT WOOD TO BE HOT-DIP GALVANIZED OR STAINLESS STEEL.
- ALL HARDWARE TO BE BY SIMPSON STRONG-TIE OR APPROVED EQUAL. WHERE HARDWARE IS NOT SPECIFICALLY DESIGNATED, SUBMIT PROPOSED HARDWARE FOR APPROVAL. WHERE MORE THAN ONE TYPE OF FASTENER OR FASTENER PATTERN IS ALLOWED BY THE HARDWARE MANUFACTURER, HARDWARE FASTENERS ARE TO BE OF THE TYPE, SIZE, AND QUANTITY TO MAXIMIZE THE LOAD CAPACITY OF THE HARDWARE IN THE SPECIFIC APPLICATION SHOWN ON THESE PLANS, UNLESS NOTED OTHERWISE.
- REFERENCE THE ARCHITECTURAL PLANS FOR LAYOUT OF ALL WALLS, OPENINGS, WALL TYPES, ETC. VERIFY ALL DIMENSIONS PRIOR TO DESIGN OF WALL PANELS & IMMEDIATELY NOTIFY THE ARCHITECT AND ENGINEER OF ANY DISCREPANCIES.

**GENERAL**

THE STRUCTURE HAS BEEN DESIGNED FOR THE IN-SERVICE LOADS ONLY. THE METHODS, MEANS, PROCEDURES, AND SEQUENCES OF CONSTRUCTION ARE THE RESPONSIBILITY OF THE CONTRACTOR. THE STRUCTURE IS DESIGNED TO BE SELF-SUPPORTING AND STABLE AFTER THE BUILDING IS COMPLETE. IT IS THE CONTRACTOR'S RESPONSIBILITY TO DETERMINE ERECTION PROCEDURES AND SEQUENCE TO INSURE SAFETY OF THE BUILDING AND ITS COMPONENTS DURING ERECTION. THIS INCLUDES THE ADDITION OF NECESSARY SHORING, SHEETING, TEMPORARY BRACING, GUYS, TIEDOWNS, ETC.

THE STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH CIVIL, ARCHITECTURAL, MECHANICAL, ELECTRICAL AND PLUMBING DRAWINGS AND SPECIFICATIONS.

THE GENERAL NOTES ON THE DRAWINGS ARE TO BE USED IN CONJUNCTION WITH THE FULL WRITTEN MATERIAL SPECIFICATIONS (IF ANY) FOR THE PROJECT. IF A DISCREPANCY OCCURS BETWEEN THE NOTES AND THE FULL SPECIFICATIONS, THE MORE STRINGENT REQUIREMENT SHALL APPLY.

NO PENETRATIONS THROUGH STRUCTURAL ELEMENTS, OTHER THAN THOSE SHOWN ON THE DRAWINGS, SHALL BE MADE WITHOUT THE APPROVAL OF THE ARCHITECT.

**DESIGN DATA/PRE-ENGINEERED METAL BUILDING**

THE BUILDING STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH THE 2014 INDIANA BUILDING CODE (2012 IBC W AMENDMENTS).

SEE SPECIFICATION SECTION 13 34 19 FOR THE FOLLOWING DESIGN DATA:

- SNOW LOAD DESIGN DATA
- WIND LOAD DESIGN DATA
- EARTHQUAKE DESIGN DATA

- COMBINING OF NORMAL AND AUXILIARY LOADS FOR DESIGN PURPOSES SHALL BE AS PRESCRIBED BY LATEST INTERNATIONAL BUILDING CODE AND INDIANA CONSTRUCTION RULES
- MEMBERS NOT SIZED OR SHOWN ON PLANS AND SECTIONS SHALL BE AS DESIGNED BY BUILDING MANUFACTURER
- BUILDING MANUFACTURER SHALL PROVIDE ANCHOR BOLT LAYOUT PLAN
- BUILDING MANUFACTURER SHALL SUBMIT COMPLETE STRUCTURAL ANALYSIS AND SHOP DRAWINGS OF BUILDING SYSTEM TO ENGINEER PRIOR TO FABRICATION.

**CONCRETE MIX DESIGN SUBMITTAL**

THE CONTRACTOR SHALL SUBMIT FOR THE REVIEW OF THE STRUCTURAL ENGINEER A MIX DESIGN FOR EACH PROPOSED CLASS OF CONCRETE. EACH MIX DESIGN SHALL BE IDENTIFIED BY A MIX NUMBER OR OTHER UNIQUE IDENTIFICATION. THE CONTRACTOR SHALL NOT VARY FROM THE MIX DESIGNS NOR USE ANY CONCRETE OTHER THAN THE APPROVED MIX DESIGNS WITHOUT THE APPROVAL OF THE STRUCTURAL ENGINEER. MIX DESIGN SUBMITTALS SHALL INCLUDE THE FOLLOWING INFORMATION:

- MIX DESIGN NUMBER OR UNIQUE IDENTIFICATION AND INTENDED LOCATION OF PLACEMENT.
- CEMENT TYPE, PROPORTION AND NAME OF MANUFACTURER.
- FLY ASH PROPORTION (WHEN USED), LABORATORY ANALYSIS CERTIFICATION, AND NAME AND LOCATION OF SUPPLIER.
- COURSE AGGREGATE PROPORTION, GRADATION REPORT, NAME AND LOCATION OF SUPPLIER.
- FINE AGGREGATE PROPORTION, GRADATION REPORT, NAME AND LOCATION OF SUPPLIER.
- MIXING WATER PROPORTION AND SOURCE.
- ADMIXTURE DOSAGES, PRODUCT NAME(S) AND MANUFACTURER NAME(S).
- FIBER REINFORCEMENT DOSAGE (WHEN USED), PRODUCT NAME AND MANUFACTURER NAME.
- DESIGN 28-DAY COMPRESSIVE STRENGTH (FC).
- DESIGN SLUMP RANGE.
- DESIGN AIR-ENTRAINMENT (FOR CONCRETE REQUIRING ENTRAINMENT AIR).
- STATISTICAL ANALYSIS OF LABORATORY STRENGTH TEST DATA IN ACCORDANCE WITH "STANDARD DEVIATION" DETERMINATION OUTLINED IN ACI 318.

**PRODUCT DATA SUBMITTALS**

THE CONTRACTOR SHALL SUBMIT FOR APPROVAL PRODUCT DATA FOR THE SPECIFIC ITEMS LISTED BELOW. CONTRACTOR SHALL NOT USE PRODUCTS OTHER THAN THOSE SUBMITTED WITHOUT THE APPROVAL OF THE STRUCTURAL ENGINEER.

MANUFACTURER'S TRADE NAMES AND NUMBERS USED HEREIN ARE ONLY TO IDENTIFY COLORS, FINISHES, TEXTURES, AND PATTERNS AS A BASIS OF DESIGN. PRODUCTS OF AUTHORIZED EQUAL MANUFACTURERS EQUIVALENT TO COLORS, FINISHES, TEXTURES, AND PATTERNS OF MANUFACTURERS LISTED THAT MEET REQUIREMENTS OF TECHNICAL SPECIFICATIONS IN EVERY RESPECT MAY BE ACCEPTABLE WITH SUBMITTAL OF A COMPLETED SUBSTITUTION REQUEST CONTAINING ALL PRODUCT DATA, TESTING AND ACTUAL SAMPLES, AND UPON APPROVAL IN WRITING BY CONTRACTING OFFICER.

SUBMIT ELECTRONIC VERSIONS OF PRODUCT DATA IN PDF FORMAT TO THE STRUCTURAL ENGINEER AND COR FOR REVIEW.

SUBMIT SHOP DRAWINGS FOR EACH OF THE FOLLOWING ITEMS:

- FIBER REINFORCEMENT FOR CONCRETE
- CONCRETE REINFORCEMENT
- DIAMOND PLATE DOWELS FOR SLABS ON GRADE
- CONCRETE CURING COMPOUND
- CONCRETE JOINT SEALANT
- WATER STOPS
- EXPANSION ANCHORS
- ADHESIVE ANCHORS
- NON-SHRINK GROUT
- STRUCTURAL STEEL
- STEEL METAL DECK

**FOUNDATIONS**

FOUNDATION EXCAVATIONS AND SOIL RELATED WORK SHALL BE PERFORMED IN ACCORDANCE WITH THE GEOTECHNICAL REPORT.

CONTRACTOR IS TO REPLACE FILL AS NEEDED WHEN ENCOUNTERED, REF. GEOTECHNICAL REPORT

FOUNDATIONS HAVE BEEN DESIGNED BASED ON ASSUMED ALLOWABLE CAPACITIES INDICATED BELOW. CONTRACTOR COORDINATE WITH THE OWNER'S GEOTECHNICAL ENGINEER TO REVIEW THE SOIL CONDITIONS AT THE SITE AS NECESSARY TO CONFIRM THE ALLOWABLE CAPACITIES PRIOR TO PLACEMENT OF THE FOUNDATIONS. CONFIRMATION OF THE ACCEPTABILITY OF THE SOILS SHALL BE REPORTED IN WRITING TO THE ARCHITECT.

SOFT SOILS FOUND BELOW SPREAD FOOTING LOCATIONS MUST BE REMOVED AND REPLACED WITH WELL-COMPACTED ENGINEERED FILL PER DIRECTION OF GEOTECHNICAL ENGINEER.

IF ACTUAL ALLOWABLE CAPACITIES ARE FOUND TO BE LESS THAN THE ASSUMED VALUES, THE CONTRACTOR SHALL NOTIFY THE ARCHITECT IMMEDIATELY AND SHALL NOT PROCEED WITH FOUNDATION CONSTRUCTION WITHOUT REVIEW BY THE ARCHITECT.

DESIGN ALLOWABLE BEARING PRESSURE: 1500 PSF

EXCAVATIONS FOR SPREAD FOOTINGS AND CONTINUOUS FOOTINGS SHALL BE CLEANED AND HAND TAMPED TO UNIFORM SURFACE AND SHALL BE PROTECTED AND MAINTAINED UNIFORM UNTIL CONCRETE IS PLACED. IF EXCAVATIONS ARE EXPECTED TO BE OPEN FOR MORE THAN 24 HOURS, A 3" CONCRETE "MUD" MAT SHALL BE PLACED.

**CONCRETE**

REINFORCED CONCRETE HAS BEEN DESIGNED IN ACCORDANCE WITH THE BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE (ACI 318) AND COMMENTARY (ACI 318R).

MIXING, TRANSPORTING, AND PLACING OF CONCRETE SHALL CONFORM TO THE LATEST EDITION OF THE SPECIFICATIONS FOR STRUCTURAL CONCRETE (ACI 318). READY-MIXED CONCRETE SHALL CONFORM TO THE REQUIREMENTS OF ASTM C94. IN CASE OF A DISCREPANCY, THE PLANS AND SPECIFICATIONS SHALL GOVERN.

CEMENT SHALL CONFORM TO ASTM C150, TYPE III/ OR TYPE II, UNO.

FLY ASH SHALL CONFORM TO ASTM C618, CLASS C OR F. THE RATIO OF THE AMOUNT (BY WEIGHT) OF FLY ASH TO TOTAL CEMENTITIOUS MATERIALS IN THE MIX SHALL NOT EXCEED 25 PERCENT.

NORMAL WEIGHT AGGREGATES SHALL CONFORM TO ASTM C33.

WATER-REDUCING ADMIXTURES SHALL CONFORM TO ASTM C494.

AIR-ENTRAINING ADMIXTURES SHALL CONFORM TO ASTM C260 AND SHALL BE CERTIFIED BY THE MANUFACTURER TO BE COMPATIBLE WITH OTHER ADMIXTURES.

CALCIUM CHLORIDE ADMIXTURES OR ADMIXTURES CONTAINING MORE THAN 0.1 PERCENT CHLORIDE IONS SHALL NOT BE USED.

IN COLD WEATHER CONDITIONS, MIXING, PLACING, FINISHING, CURING AND PROTECTION OF CONCRETE SHALL BE PERFORMED IN ACCORDANCE WITH THE LATEST EDITION OF ACI 308R, COLD WEATHER CONCRETING.

IN HOT WEATHER CONDITIONS, MIXING, PLACING, FINISHING, CURING AND PROTECTION OF CONCRETE SHALL BE PERFORMED IN ACCORDANCE WITH THE LATEST EDITION OF ACI 305R, HOT WEATHER CONCRETING.

USE OF CONSTRUCTION JOINTS AT LOCATIONS OTHER THAN THOSE INDICATED ON THE DRAWINGS SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER FOR REVIEW. IF NONE SHOW CONTRACTOR SHALL SUBMIT PLANS FOR REVIEW AND APPROVAL.

SLUMP FOR PUMPED CONCRETE SHALL BE MEASURED AT POINT OF DISCHARGE.

NORMAL WEIGHT CONCRETE SHALL BE USED IN THE FOLLOWING AREAS AND SHALL HAVE THE PROPERTIES AS SHOWN BELOW:

**CONCRETE SLABS ON GRADE**

SLABS ON GRADE SHALL BE CONSTRUCTED IN ACCORDANCE WITH THE LATEST EDITION OF THE GUIDE FOR CONCRETE FLOOR AND SLAB CONSTRUCTION (ACI 302.1R).

PLACE CONCRETE IN A MANNER SO AS TO PREVENT SEGREGATION OF THE MIX. DELAY FLOATING AND TROWELING OPERATIONS UNTIL THE CONCRETE HAS LOST SURFACE WATER SHEEN OR ALL FREE WATER. DO NOT SPRINKLE FREE CEMENT ON THE SLAB SURFACE.

PROVIDE CURING OF CONCRETE SLABS IMMEDIATELY AFTER FINISHING USING A SPRAYED ON DISSIPATING-RESIN LIQUID CURING COMPOUND CONFORMING TO ASTM C309, UNLESS NOTED OTHERWISE. ALL SCUFFS OR ABRASIONS TO THE CURING MEMBRANE SHALL BE RECOATED DAILY. OTHER CURING METHODS MAY BE USED WITH APPROVAL BY THE STRUCTURAL ENGINEER.

SLABS ON GRADE SHALL BE PLACED TO ACHIEVE THE FOLLOWING MINIMUM TOLERANCES:

OVERALL VALUES:	ALL AREAS: F1 = 25, F1 = 20
LOCAL VALUES:	ALL AREAS: F1 = 17, F1 = 15

THE MINIMUM LOCAL AREA SHALL BE ANY BAY DEFINED BY COLUMN LINES.

UNLESS SHOWN OR NOTED OTHERWISE, PROVIDE CONTROL OR CONSTRUCTION JOINTS IN SLABS-ON-GRADE AT A MAXIMUM SPACING PROVIDED ON S1-0 JOINT DETAIL. PROVIDE JOINTS AT ALL COLUMN LOCATIONS. LOCATE JOINTS TO ELIMINATE RE-ENTRANT CORNERS AND TO CREATE SQUARE OR RECTANGULAR SECTIONS WITH MAXIMUM LONG SIDE TO SHORT SIDE RATIO OF 1.5 TO 1.

CONTROL JOINTS IN SLABS ON GRADE SHALL RECEIVE JOINT FILLER MATERIAL EXCEPT WHERE TILE IS CALLED FOR.

**REINFORCING STEEL**

REINFORCING STEEL SHALL BE DEFORMED BARS CONFORMING TO ASTM A615 AND SHALL HAVE A MINIMUM YIELD STRENGTH OF 80,000 PSI.

REINFORCING BAR DETAILING, FABRICATING, AND PLACING SHALL CONFORM TO THE LATEST EDITION OF THE FOLLOWING STANDARDS: ACI 301, ACI 315, ACI 318 AND ACI DETAILING MANUAL (SP66).

UNLESS A GREATER AMOUNT OF COVER IS INDICATED ON THE DRAWINGS, PROVIDE THE FOLLOWING MINIMUM CONCRETE COVER OVER REINFORCEMENT AS FOLLOWS:

- CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH: 3 INCHES
- #6 BARS AND LARGER EXPOSED TO EARTH OR WEATHER: 2 INCHES
- #5 BARS AND SMALLER EXPOSED TO EARTH OR WEATHER: 1-1/2 INCHES

REINFORCING STEEL SHALL NOT BE TACK WELDED, WELDED, HEATED OR CUT UNLESS INDICATED ON THE CONTRACT DOCUMENTS OR APPROVED BY THE STRUCTURAL ENGINEER.

WHERE LAP SPLICE LENGTHS ARE NOT SHOWN OR NOTED, PROVIDE A CLASS "B" LAP.

ALL 90 DEGREE AND 180 DEGREE BENDS SHOWN ON THE DRAWINGS SHALL BE STANDARD HOOKS, UNLESS NOTED OTHERWISE.

PROVIDE CORNER BARS OF SAME SIZE AND SPACING AS HORIZONTAL BARS AT CORNERS OF ALL WALLS. LAP SPLICE CORNER BARS WITH STRAIGHT BARS.

UNLESS OTHERWISE SHOWN OR NOTED, PROVIDE 2-#5 BARS (ONE EACH FACE) AROUND UNFRAMED OPENINGS IN CONCRETE WALLS. PLACE BARS PARALLEL TO THE SIDES OF THE OPENING AND EXTEND 24" BEYOND CORNERS.

**CONCRETE TESTING**

MAKE ONE SET OF TEST CYLINDERS IN ACCORDANCE WITH ASTM C31 FOR EACH DAY'S POUR AND FOR EACH 100 CUBIC YARDS. EACH SET SHALL INCLUDE 1 SPECIMEN TESTED AT 7 DAYS, 1 SPECIMEN TESTED AT 28 DAYS.

WHEN THE AMBIENT TEMPERATURE IS EXPECTED TO FALL BELOW 40 DEGREES DURING THE COURSE OF A CONCRETE POUR OR SUBSEQUENT CURING PROCESS, AN ADDITIONAL SET OF CONCRETE TEST CYLINDERS SHALL BE MADE AND TESTED. THESE CYLINDERS SHALL BE STORED IMMEDIATELY ADJACENT TO, AND CURED UNDER THE SAME CONDITIONS AS THE BUILDING CONCRETE. SPECIAL CURING BOXES ARE NOT PERMITTED FOR THESE TEST CYLINDERS.

FORWARD COPIES OF TEST RESULTS TO THE ARCHITECT WITHIN 24 HOURS AFTER TESTING.

**EXPANSION ANCHORS**

EXPANSION ANCHORS SHALL BE CARBON STEEL ANCHORS AS MANUFACTURED BY HILTI FASTENING SYSTEMS OR AN EQUIVALENT SUBSTITUTE APPROVED BY THE STRUCTURAL ENGINEER AND SHALL BE INSTALLED IN CONFORMANCE WITH THE MANUFACTURER'S RECOMMENDATIONS.

EXPANSION ANCHORS SHALL NOT BE INSTALLED IN CONCRETE UNTIL IT HAS ATTAINED ITS SPECIFIED MINIMUM 28 DAY COMPRESSIVE STRENGTH.

**NON-SHRINK GROUT**

GROUT SHALL BE A NON-METALLIC, SHRINKAGE RESISTANT (WHEN TESTED IN ACCORDANCE WITH THE LATEST EDITION OF ASTM C827 OR CRD-C821), PREMIXED, NON-CORROSIVE, NON-STAINING PRODUCT CONTAINING PORTLAND CEMENT, SILICA SANDS, SHRINKAGE COMPENSATING AGENTS AND FLUIDITY IMPROVING COMPOUNDS.

GROUT SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH (FC) OF 5000 PSI IN 28 DAYS.

CONCRETE MIX CLASSES	
<b>FOOTINGS AND FOUNDATION WALLS</b>	
COMPRESSIVE STRENGTH	4000 PSI
MAXIMUM WATER/CEMENT RATIO	0.45
AIR CONTENT	0 - 3 PERCENT
WATER-REDUCING ADMIXTURE	OPTIONAL
SLUMP	5 - 6.5"
<b>INTERIOR CONCRETE SLABS ON GRADE &amp; SUSPENDED SLABS</b>	
COMPRESSIVE STRENGTH	4000 PSI
MINIMUM CEMENTITIOUS MATERIAL CONTENT	517 LB/CU YD
AIR CONTENT	0 - 3 PERCENT
WATER-REDUCING ADMIXTURE	REQUIRED
SLUMP	4" +/- 1"
ES INTERNAL CURE ADMIXTURE	4 OZ/CWT
1. PROVIDE SEALERS PER FOUNDATION PLAN NOTES	
<b>EXTERIOR CONCRETE SUBJECT TO FREEZE-THAW</b>	
COMPRESSIVE STRENGTH	5000 PSI
MINIMUM CEMENTITIOUS MATERIAL CONTENT	564 LB/CU YD
AIR CONTENT	6 +/- 1 PERCENT
WATER-REDUCING ADMIXTURE	REQUIRED
SLUMP	5" +/- 1"
COARSE AGGREGATE	CRUSHED STONE
<b>LEAN CONCRETE FILL</b>	
COMPRESSIVE STRENGTH	2000 PSI
MAXIMUM WATER/CEMENT RATIO	0.65
AIR CONTENT	OPTIONAL
WATER-REDUCING ADMIXTURE	OPTIONAL
SLUMP	4" +/- 1"

- SLUMP:
  - MIXES CONTAINING TYPE A WRDA 5" MAXIMUM
  - MIXES CONTAINING MID-RANGE WRDA 5 - 6 1/2"
  - MIXES CONTAINING HIGH-RANGE WRDA 5 - 8"
- SPECIFIED MINIMUM CEMENTITIOUS MATERIAL CONTENTS ARE BASED ON THE USE OF WATER REDUCING ADMIXTURES.
- INCLUDE AN AIR-ENTRAINING ADMIXTURE FOR ALL CONCRETE EXPOSED TO FREEZING AND THAWING IN SERVICE AND FOR ALL CONCRETE EXPOSED TO COLD WEATHER DURING CONSTRUCTION, BEFORE ATTAINING ITS SPECIFIED DESIGN COMPRESSIVE STRENGTH. REF. ACI 306 FOR DEFINITION OF COLD WEATHER.
- CLASS C FLY ASH MAY BE USED AS A CEMENT SUBSTITUTE WITH A MAXIMUM 20% SUBSTITUTION RATE ON A POUND-PER-POUND BASIS.
- PROPORTION CONCRETE MIXES TO PROVIDE WORKABILITY AND CONSISTENCY TO PERMIT CONCRETE TO BE WORKED READILY INTO THE CORNERS AND ANGLES OF THE FORMS AND AROUND REINFORCEMENT BY THE METHODS OF PLACEMENT AND CONSOLIDATION TO BE EMPLOYED, WITHOUT SEGREGATION AND EXCESSIVE BLEEDING.
- ADJUSTMENTS TO THE APPROVED MIX DESIGNS MAY BE REQUESTED BY THE CONTRACTOR WHEN JOB CONDITIONS, WEATHER, TEST RESULTS, OR OTHER CIRCUMSTANCES WARRANT. THESE REVISED MIX DESIGNS SHALL BE SUBMITTED TO THE ARCHITECT/ENGINEER FOR APPROVAL PRIOR TO USE.



**REVISIONS**

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25082 - PROJECT B.1 - MAINTENANCE BUILDING  
 LAKELAND SCHOOL CORPORATION  
 04/10/26  
 E 075 N. LAGRANGE, IN 46761  
 PRICING SET



*Winifred Williams*

PRICING SET  
 04/10/26  
 kM JOB NO.  
 25082  
 DRAWN BY  
 KS

DRAWING NAME  
**STRUCTURAL  
 GENERAL NOTES**

DRAWING NO.  
**S0-1**



### FOUNDATION GENERAL NOTES

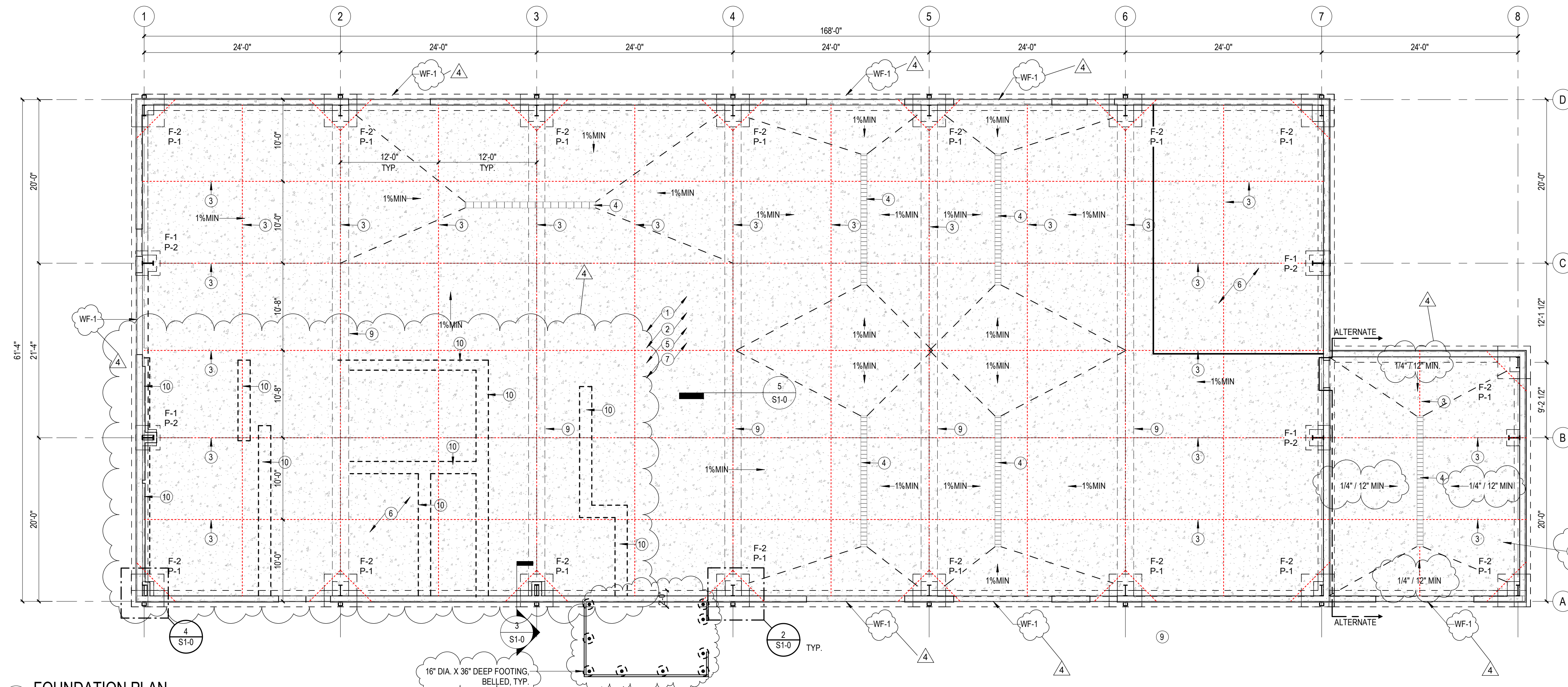
- SEE ADDITIONAL NOTES ON S0-1
- PROVIDE CONTROL JOINT ALIGNED WITH ALL COLUMNS IN EACH DIRECTION. ALSO PROVIDE CONTROL JOINT NOT LESS THAN 12 IN EACH DIRECTION.
- SOIL DESIGN ALLOWABLE BEARING PRESSURE IS 2,000 PSF. THIRD PARTY INSPECTOR TO VERIFY SOIL BEARING PRESSURE AT THE BOTTOM OF FOOTINGS PRIOR TO FOUNDATION INSTALLATION. IF SOIL PRESSURE IS BELOW DESIGN PRESSURE NOTIFY THE ARCHITECT IMMEDIATELY.
- SLAB REINFORCING TO BE SUPPORTED W/ METAL WIRE BAR CHAIRS TO ENSURE WIRE MESH IS CENTERED IN THE SLAB ON GRADE. SPACE CHAIRS @ 48" O.C. EACH WAY. PROVIDE CHAIRS W/ PLASTIC TIPS ON LEGS.
- FOOTING REINFORCING IS TO BE SUPPORTED BY METAL BAR CHAIRS TO PROVIDE 3 INCHES OF COVER.
- DEPTHS SHOWN ARE TO TOP OF FOOTING FROM TOP OF FLOORSLAB

### PLAN NOTES - FOUNDATION

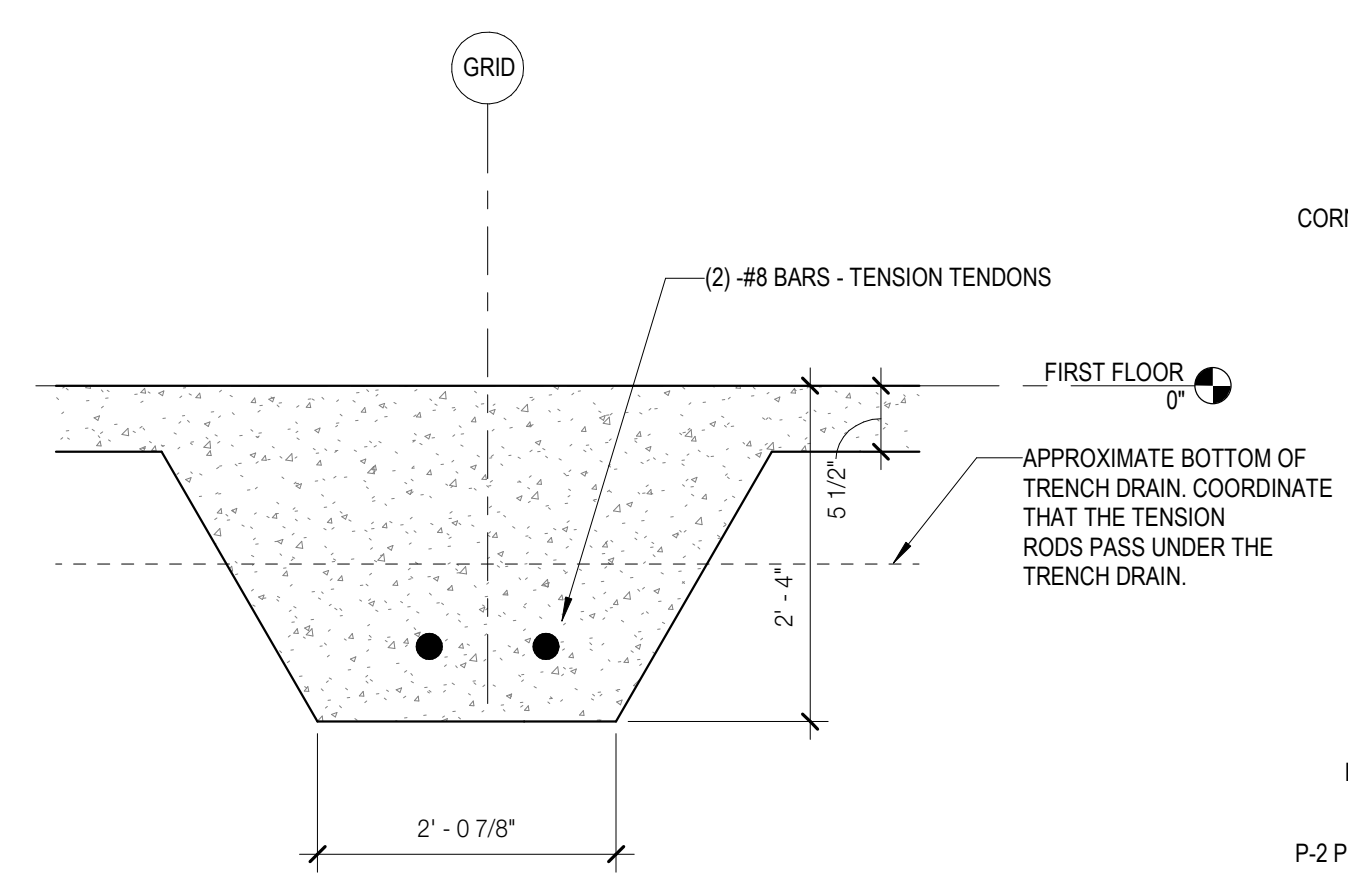
- TENSION TENDON RE-BARS ARE TO OVERLAP 30 DIAMETERS OF THE BAR THICKNESS. THE BARS ARE TO BE TIGHTLY TIED TOGETHER WITH WIRE.
- CONTRACTOR TO REQUEST INSPECTION BY ARCHITECT OF ALL FORMS AND REINFORCING PRIOR TO CONCRETE PLACEMENT.
- TYPICAL SAWED IN CONTROL JOINT, TYPICAL PER CONTROL JOINT DETAIL ON THIS SHEET. FILL WITH SEALANT.
- HEAVY DUTY FLOOR TRENCH DRAIN
- FLOOR SLAB TO BE 6" THICK CONCRETE WITH 6X6-66 HEAVY WELDED WIRE FABRIC.
- AREA FOR INTERIOR BUILD OUT, REMAIN FLAT. COORDINATE ON DEPTH.
- SEE FLOOR PLAN FOR DRAIN AND FLOORING FINISH SCOPE.
- LINE OF THICKENED SLAB FOR TENSION BARS. REFER TO 4/S1-0.
- THICKENED SLAB FOR LOAD BEAR WALLS - 18" W X 8" DEEP WITH (2) #4 REBAR
- SEAL SLAB W/ (2) COATS SILOXA TEK 8510 CONCRETE SEALER - WASHBAY ONLY

### REVISIONS

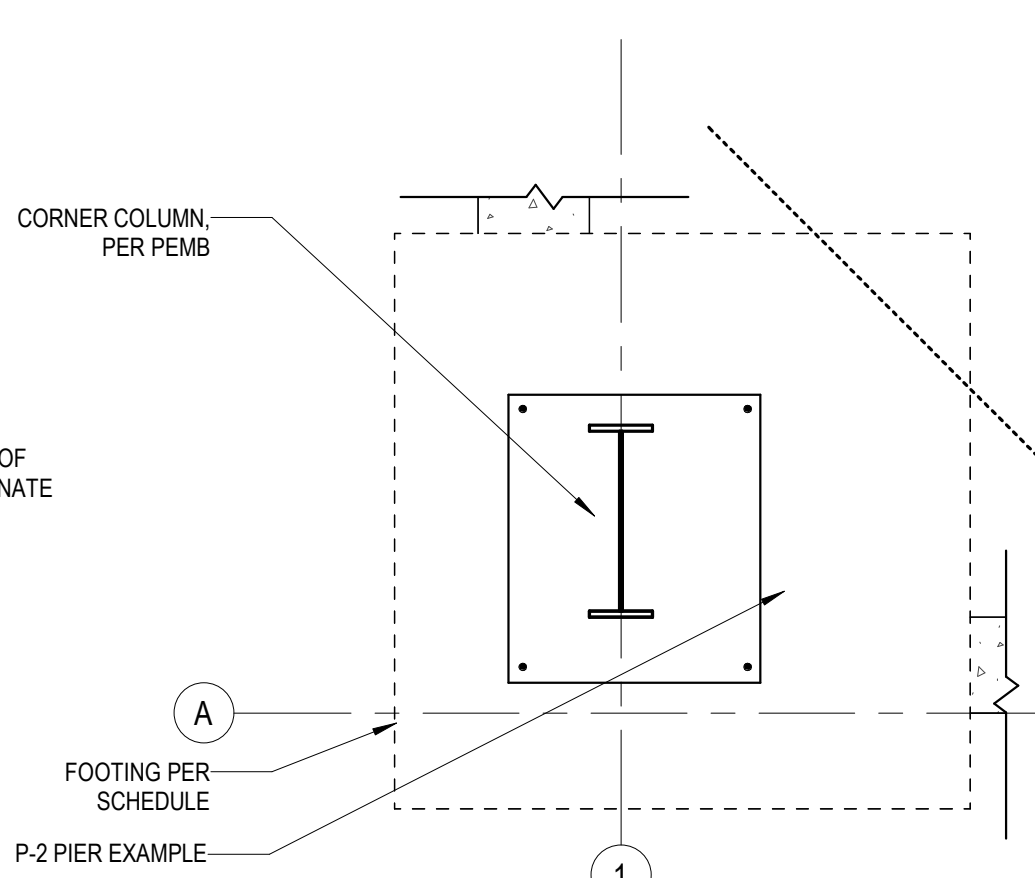
4	05/04/26	Addendum 4
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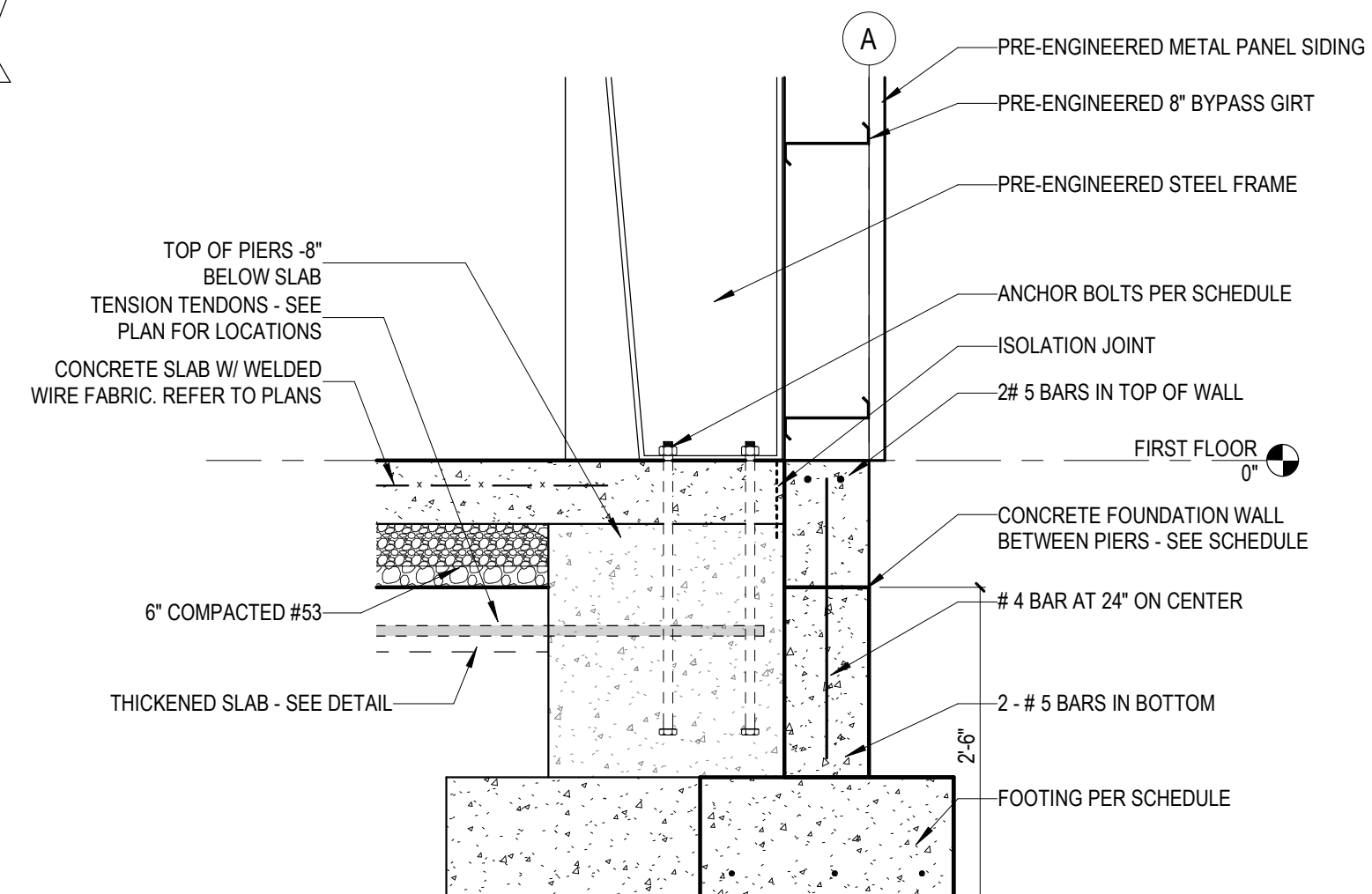
**1 FOUNDATION PLAN**  
SCALE: 1/8" = 1'-0"



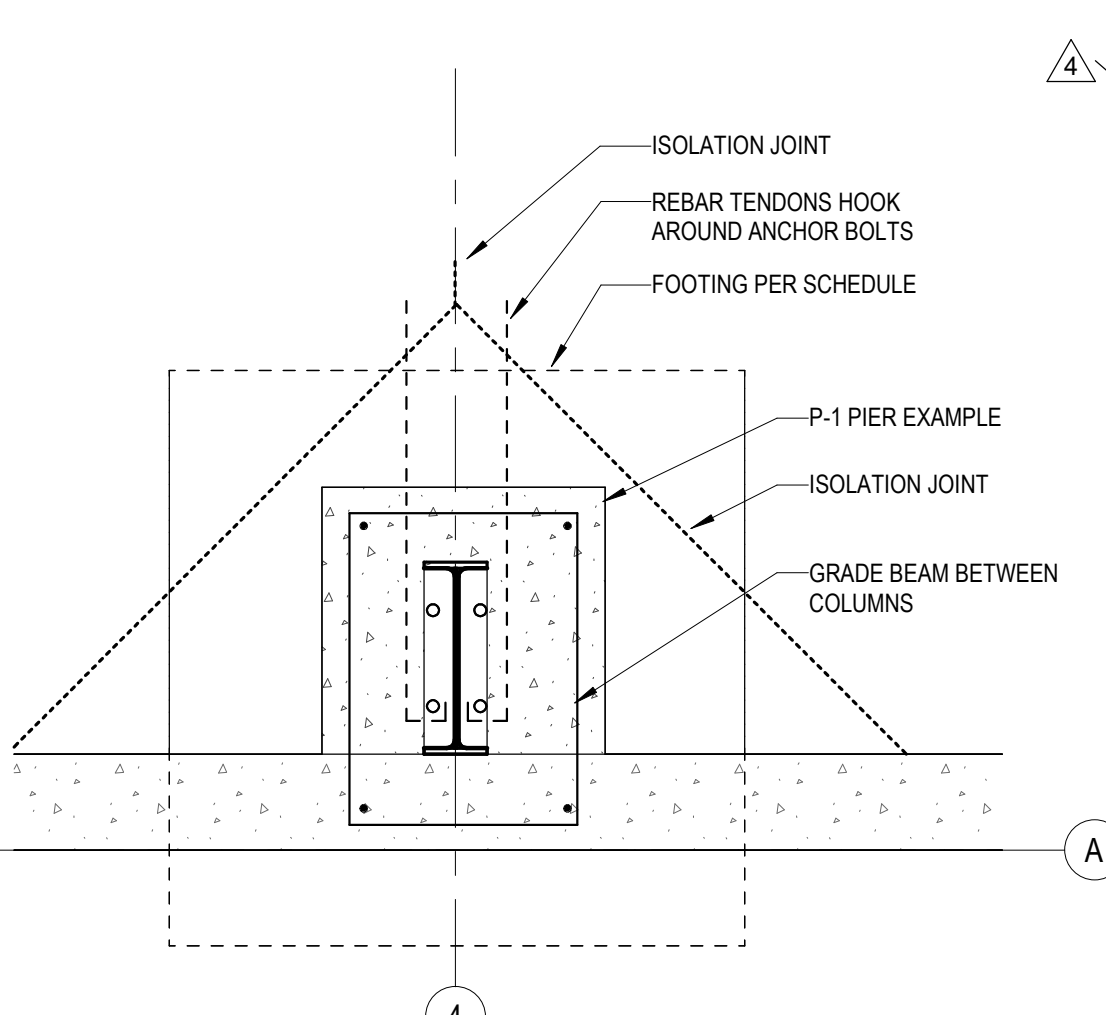
**5 TENSION TENDONS FOR PEMB**  
SCALE: 3/4" = 1'-0"



**4 TYP. CORNER COLUMN**  
SCALE: 3/4" = 1'-0"



**3 TYP. EXTERIOR STRUCTURE AND FOUNDATION DETAIL**  
SCALE: 3/4" = 1'-0"



**2 TYP. C OR F FOUNDATION**  
SCALE: 3/4" = 1'-0"

### WALL FOOTING SCHEDULE:

- WF-1: 12" TALL X 24" WIDE FOOTING WITH (3) #4 BARS 8" WIDE FOUNDATION WALL WITH REINFORCING AS SHOWN
- WF-2: 12" TALL X 24" WIDE FOOTING WITH (3) #4 BARS 18" WIDE FOUNDATION WALL WITH REINFORCING AS SHOWN

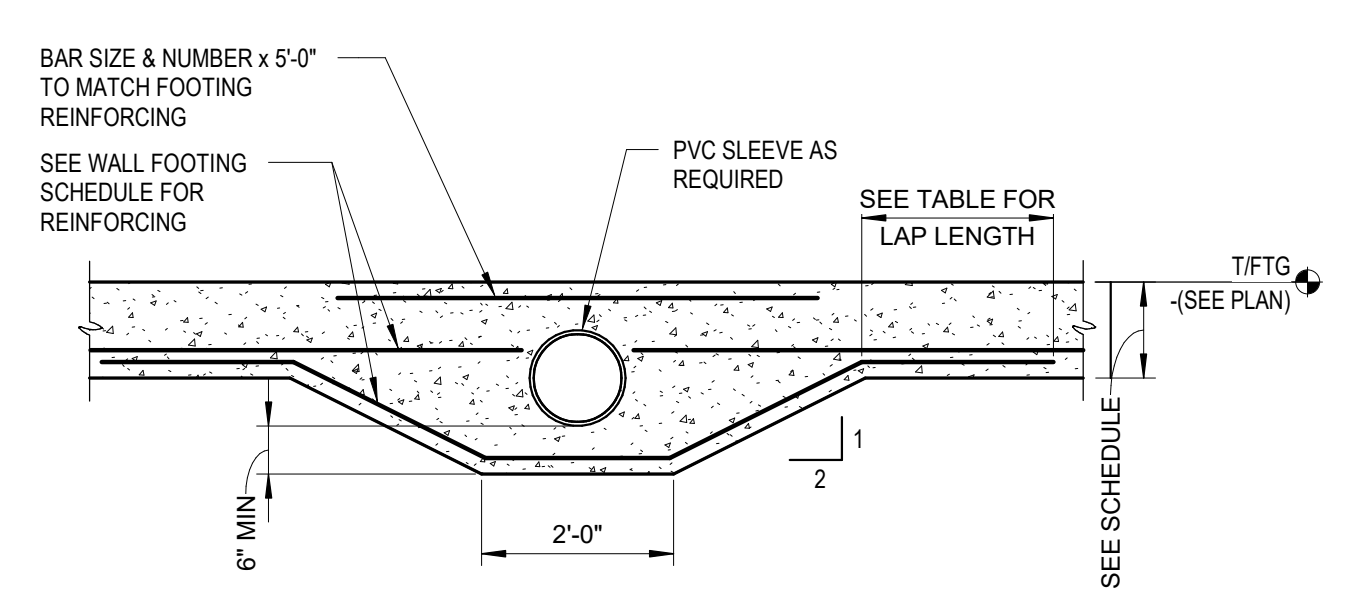
### FOOTING SCHEDULE:

- F-1: 3'x3'x1' #5 @ 10" O.C. EW (4 BARS)
- F-2: 4'x4'x1' #5 @ 8" O.C. EW (6 BARS)
- F-3: 5'x5'x1-4" #6 @ 12" O.C. EW (5 BARS)
- F-4: 3'-6"x3'-6"x1' #6 @ 9" O.C. EW (5 BARS)

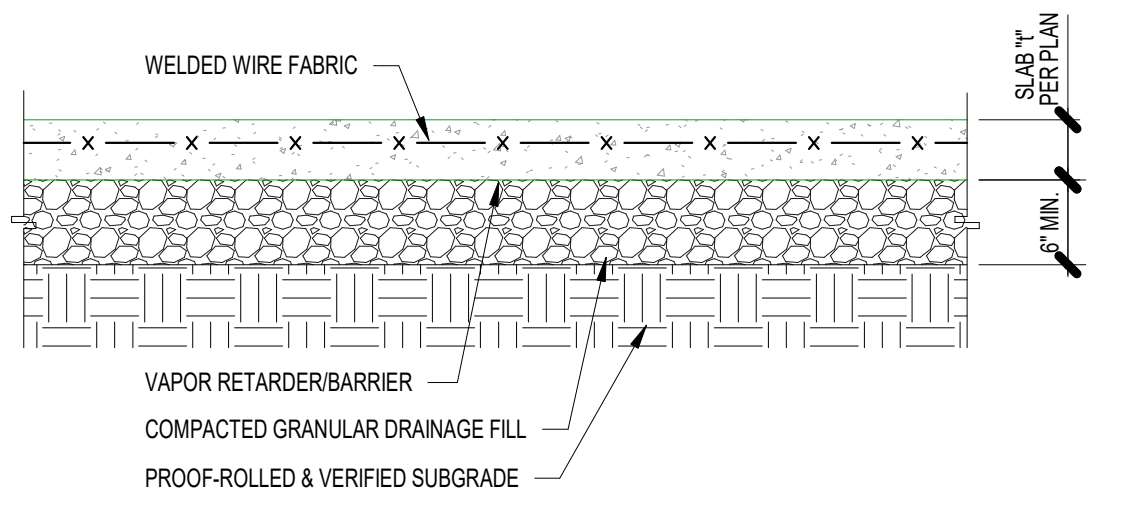
### COLUMN PIER SCHEDULE:

- P-1: 30" x 24" WITH (6) #5 BARS AND #3 TIE @ 16" O.C.
- P-2: 26" x 26" WITH (4) #5 BARS AND #3 TIE @ 16" O.C.

### SLAB ON GRADE CONSTRUCTION

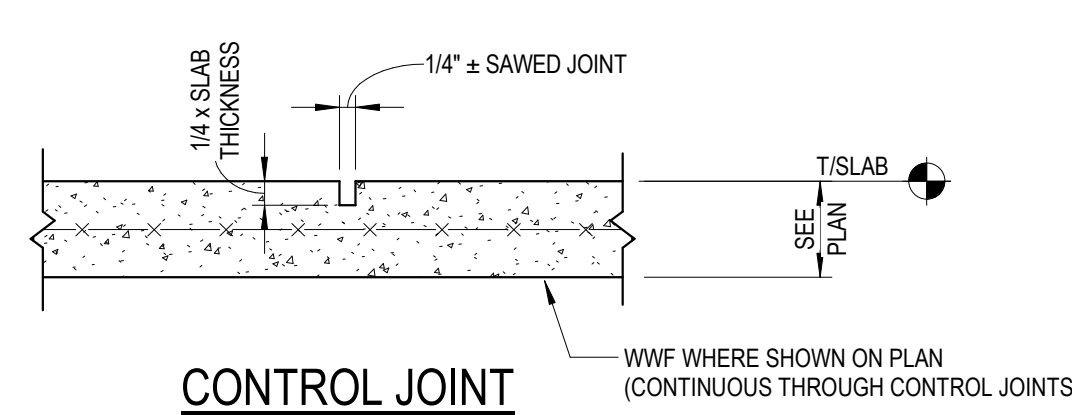


**6 TYP. SLEEVE DETAIL AT WALL FOOTING**  
SCALE: NTS



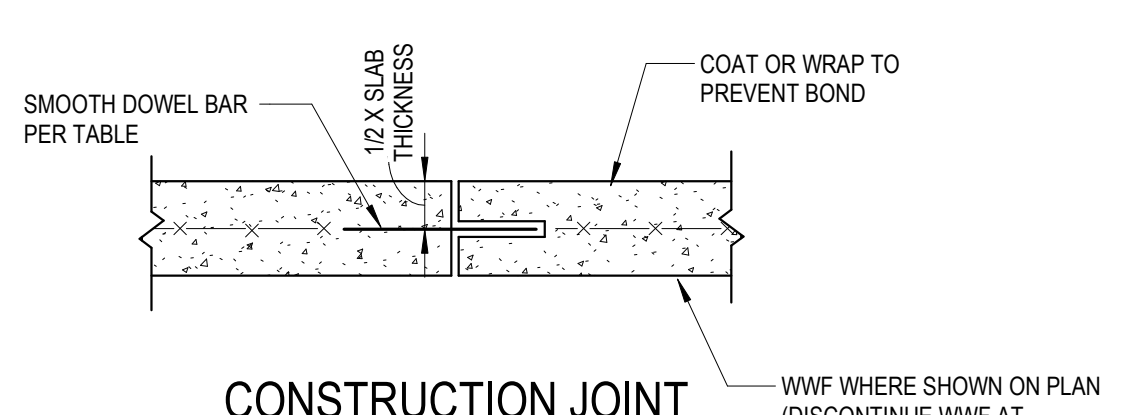
- LOCATE WELDED WIRE FABRIC IN UPPER THIRD OF SLAB. SUPPORT ON BOLSTERS, CHAIRS, OR CONCRETE BRICKS.
- LAP WELDED WIRE FABRIC A MIN. OF ONE FULL MESH SPACING.
- THE USE OF POLYPROPYLENE FIBER IN LIEU OF WELDED WIRE FABRIC AS A SECONDARY REINFORCEMENT IS PERMISSIBLE, UNLESS OTHERWISE NOTED.
- SEE PLAN, NOTES, AND SPECIFICATIONS FOR ADDITIONAL REQUIREMENTS SUCH AS FINISH, JOINTING, CURING, ETC.
- SLAB SLOPES TO FLOOR DRAINS. REF. PLUMBING DRAWINGS.

### TYPICAL SLAB JOINTS



**CONTROL JOINT**  
CONTROL JOINTS SHALL BE SAWED NOT LATER THAN THE FOLLOWING TABLE, OR 16 HOURS AFTER FINAL TROWELING (WHICHEVER IS LESS), WHERE TEMP. EQUALS THE AMBIENT TEMPERATURE IN DEGREES FAHRENHEIT AT THE TIME OF FINAL TROWELING. SAWCUTTING SHALL BE SUSPENDED ONLY IF THE LARGE AGGREGATE IS DISLODGED OR LOOSE. ALTERNATE: USE PRE-FORMED JOINT FORMER.

TEMP. °F	TIME, HOURS
<40°	16
50°	14
60°	8 1/2
70°	5 1/2
80°	4
90°	3



**CONSTRUCTION JOINT**  
WWF WHERE SHOWN ON PLAN (DISCONTINUE WWF AT CONSTRUCTION JOINTS)

DOWEL SIZE AND SPACING			
SLAB DEPTH	DOWEL DIAMETER	TOTAL DOWEL LENGTH	DOWEL SPACING CENTER TO CENTER
IN	IN	IN	IN
4	3/4	16	24
5-6	3/4	16	12

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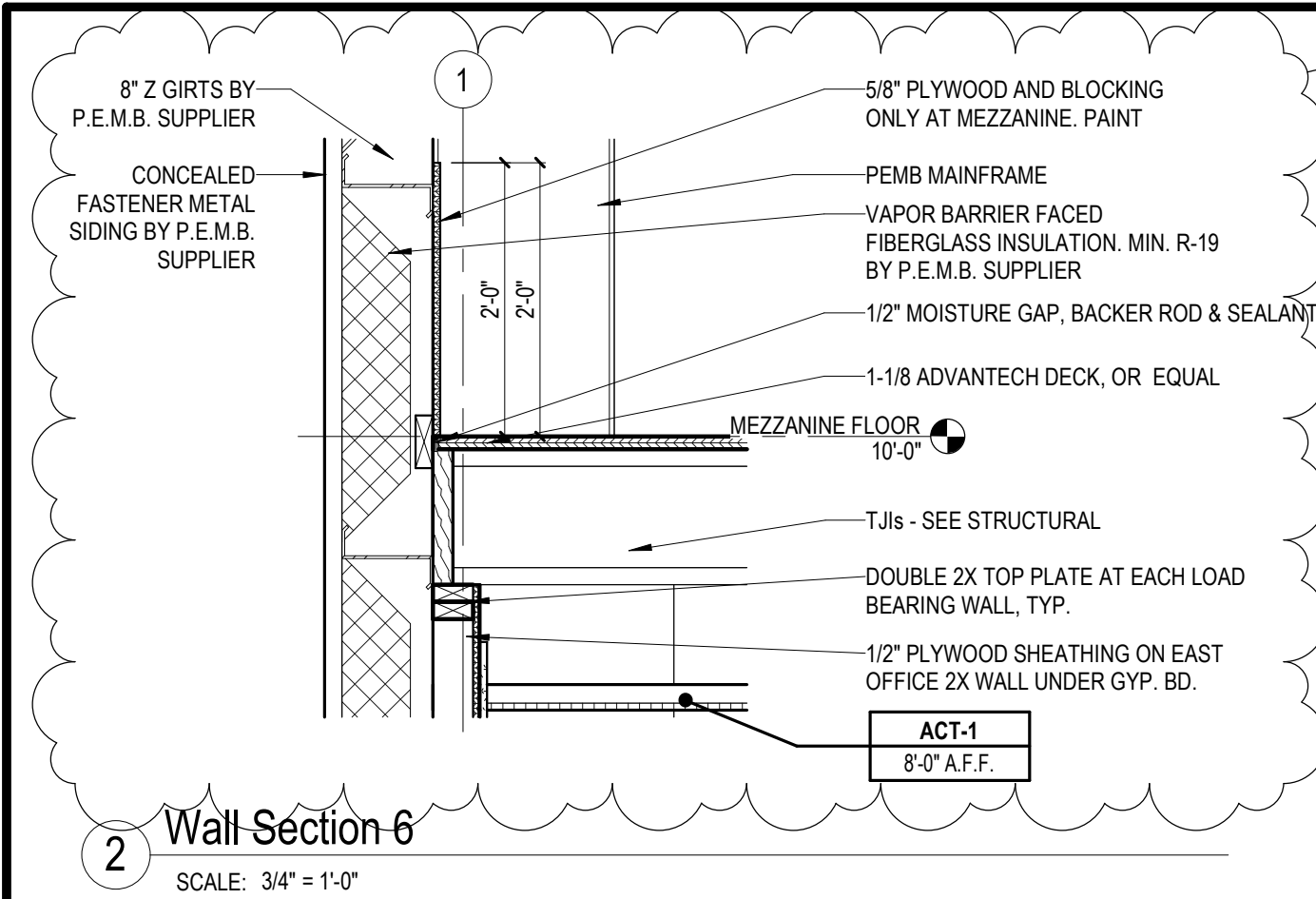


Winifred Williams

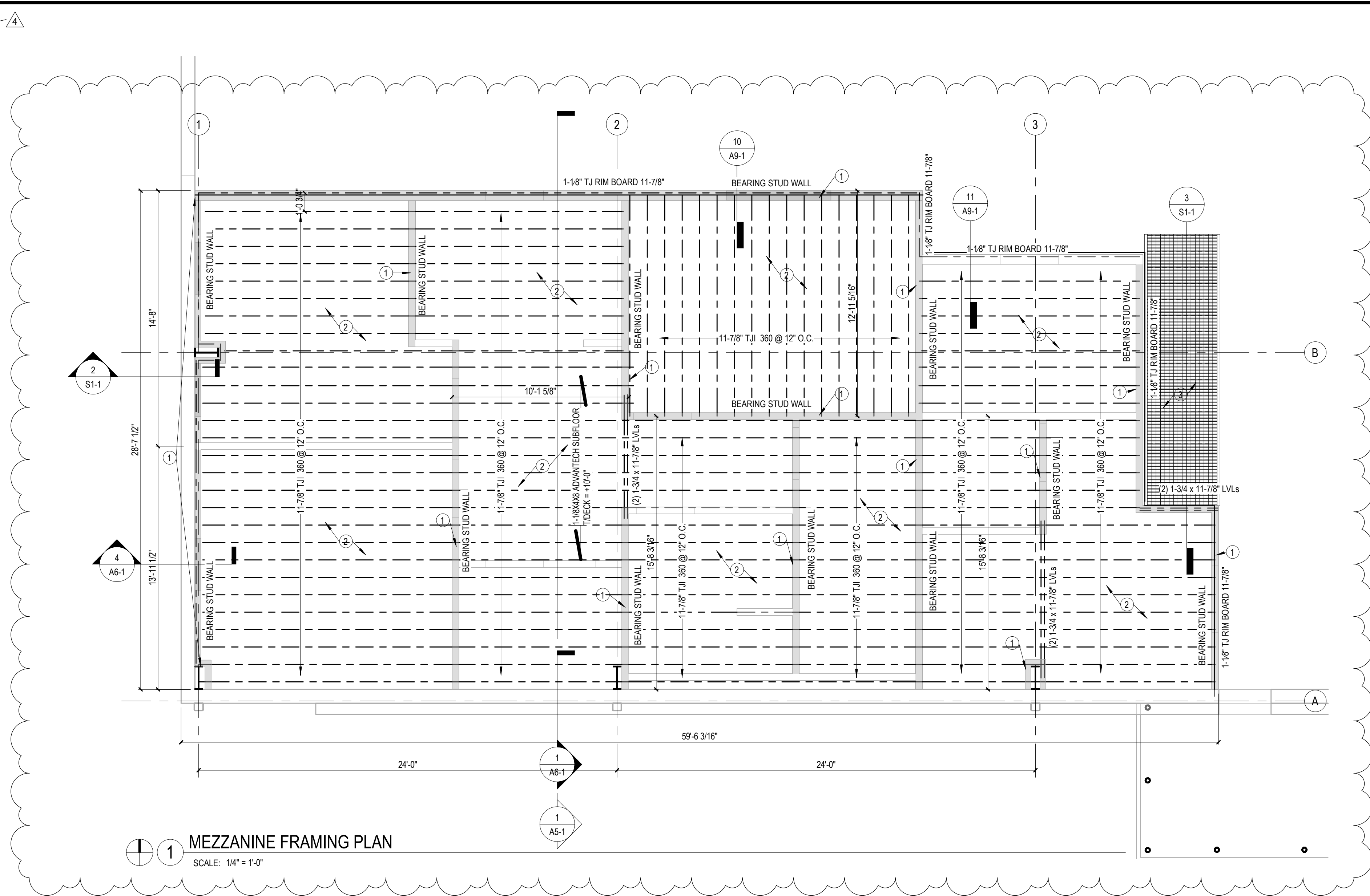
PRICING SET  
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km JOB NO.  
25082  
DRAWN BY  
Author

DRAWING NAME  
**FOUNDATION PLAN**

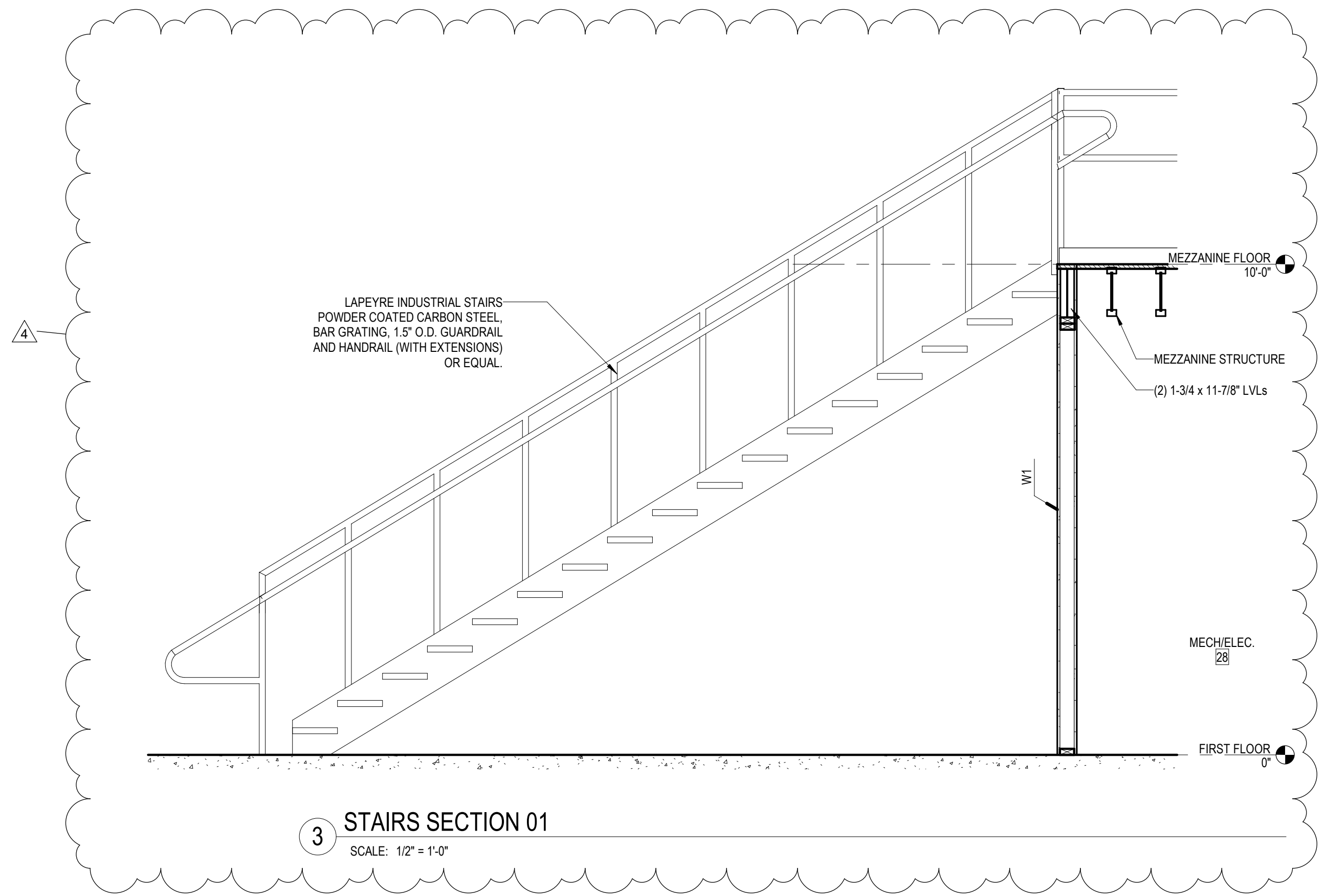
DRAWING NO.  
**S1-0**



**2 Wall Section 6**  
SCALE: 3/4\" = 1'-0"



**1 MEZZANINE FRAMING PLAN**  
SCALE: 1/4\" = 1'-0"



**3 STAIRS SECTION 01**  
SCALE: 1/2\" = 1'-0"

**PLAN NOTES - FRAMING PLAN**

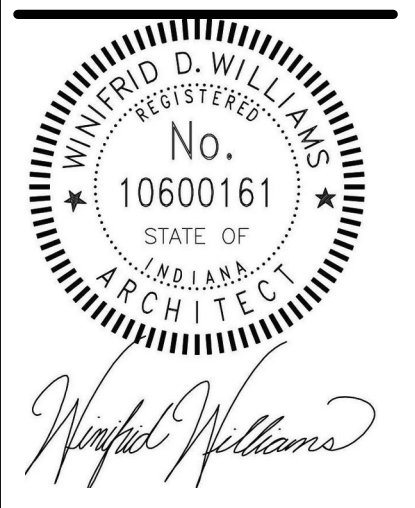
- 1 BEARING STUD WALL TO BE 2X4 @ 12" O.C. WITH DOUBLE TOP PLATE. BEARING WALL UP TIGHT TO BOTTOM OF TJI. BRACE AND STIFFEN TJI TO STUD WALL WHERE JOIST OCCURS AT DOORWAY. PROVIDE LVL HEADER TO CARRY JOIST.
- 2 PROVIDE WEB STIFFENERS AND BRACING AS REQUIRED FOR ALL TJI, TYP.
- 3 LAPEYRE INDUSTRIAL STAIRS POWDER COATED CARBON STEEL BAR GRATING, 1.5" O.D. GUARDRAIL AND HANDRAIL (WITH EXTENSIONS) OR EQUAL.



**REVISIONS**

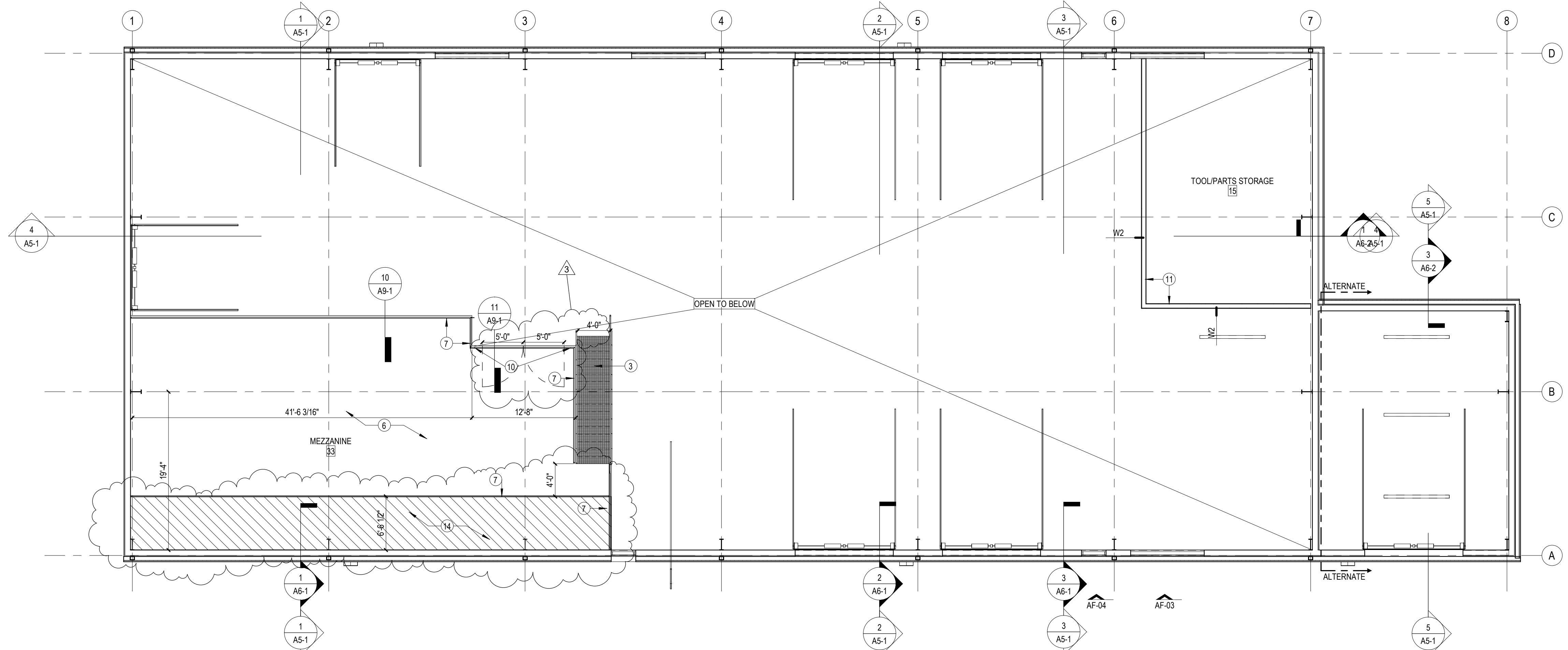
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LAKELAND SCHOOL CORPORATION  
**25082 - PROJECT B.1 - MAINTENANCE BUILDING**  
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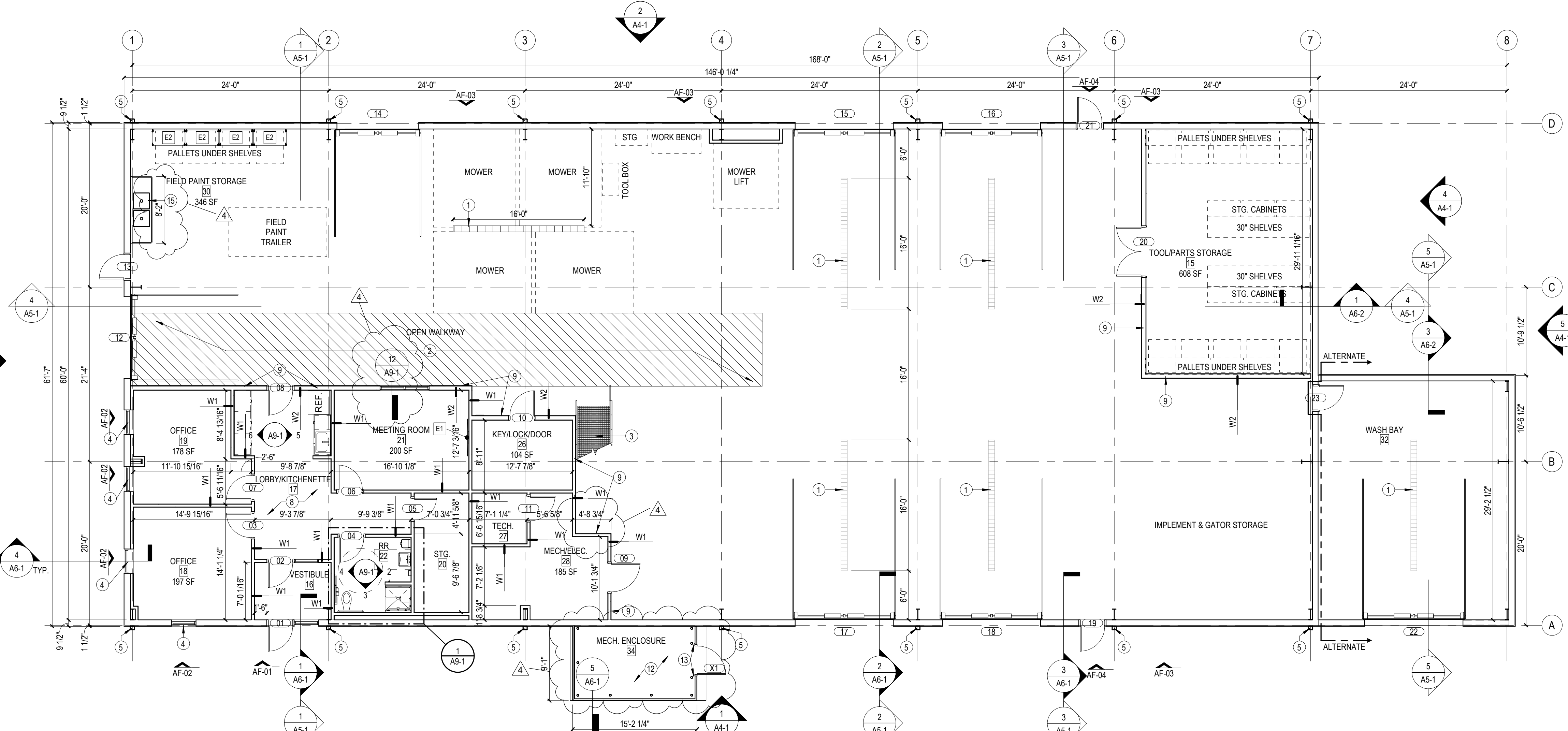


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04/10/26  
km JOB NO.  
25082  
DRAWN BY  
Author  
DRAWING NAME  
**FRAMING PLAN**

DRAWING NO.  
**S1-1**



2 MEZZANINE FLOOR PLAN  
SCALE: 1/8" = 1'-0"



1 FIRST FLOOR PLAN  
SCALE: 1/8" = 1'-0"

GENERAL NOTES - FLOOR PLAN

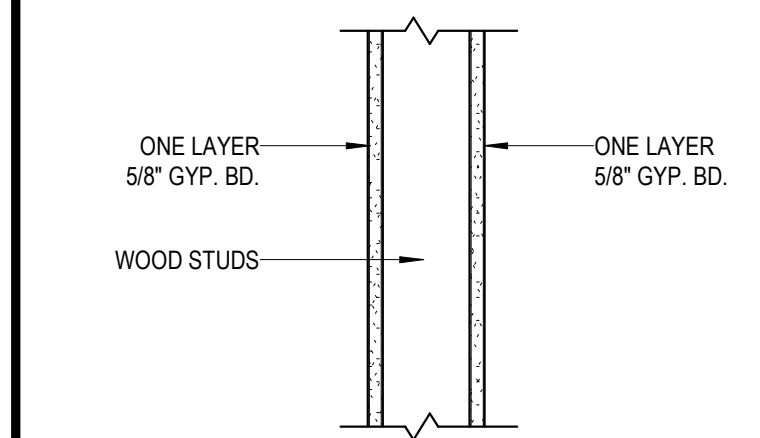
- A. CONTRACTOR TO VISIT SITE AND BECOME FAMILIAR WITH EXISTING CONDITIONS PRIOR TO START OF WORK. CONTRACTOR TO FIELD VERIFY ALL DIMENSIONS AND EXISTING CONDITIONS AND NOTIFY ARCHITECT, IN WRITING, OF ALL DISCREPANCIES. CONTRACTOR TO DOCUMENT EXISTING FIELD CONDITIONS, LIGHT FIXTURE AND MEP SUPPLY/RETURN LOCATIONS, SPRINKLER HEADS, AND ALL OTHER CEILING ITEM LOCATIONS PRIOR TO CONSTRUCTION. THIS INFORMATION SHALL BE PROVIDED TO ARCHITECT FOR INCORPORATION INTO A CONSTRUCTION SET.
- B. THE GENERAL CONTRACTOR AND EACH TRADE IS RESPONSIBLE FOR REVIEWING AND COORDINATING ALL NEW WORK WITH ALL EXISTING CONDITIONS AND WITH ALL OTHER TRADES.
- C. CONTRACTOR IS RESPONSIBLE TO PATCH/REPAIR/SEAL ALL NEW & EXISTING PENETRATIONS INTO RATED WALLS TO MAINTAIN RATED ASSEMBLY.
- D. ALL PENETRATIONS IN AND THROUGH FIRE AND SMOKE RATED WALLS SHALL BE SLEEVED AND FIRE STOPPED AS NECESSARY TO MAINTAIN RATINGS.
- E. UNLESS NOTED OTHERWISE, THE TERM "PROVIDE" INDICATES TO SUPPLY AND INSTALL COMPLETE, FOLLOWING MANUFACTURER'S INSTRUCTIONS AND RECOMMENDATIONS AND SUPPLYING AND INSTALLING ALL ASSOCIATED ITEMS AND ACCESSORIES AS REQUIRED FOR COMPLETE INSTALLATION.
- F. GENERAL CONTRACTOR SHALL COORDINATE ALL PHASES AND TIMING OF CONSTRUCTION WITH ARCHITECT, TENANT, AND BUILDING OWNER.
- G. PROVIDE WOOD BLOCKING IN WALL FOR ALL WALL-HUNG ITEMS (CASEWORK, RESTROOM ACCESSORIES, FURNITURE, ELECTRONICS, ETC.)
- H. IN NO CASE SHALL THE WALL INTERFERE WITH EXISTING WINDOWS. IF THIS OCCURS, MOVE WALL MINIMALLY TO CORRECT THE PROBLEM. NOTIFY ARCHITECT AND CONTRACTORS THAT WILL BE AFFECTED BY THIS CHANGE.
- I. DIMENSIONS ARE INDICATED FROM FINISH FACE TO FINISH FACE UNLESS NOTED OTHERWISE.
- J. UNLESS OTHERWISE NOTED, INTERIOR PARTITIONS SHALL BE TYPE M1.3.
- K. PROVIDE MOISTURE RESISTANT, TYPE "X" GYPSUM WALLBOARD FOR ALL WALLS THAT INCLUDE PLUMBING LINES.
- L. PROVIDE HAND SOAP AND PAPER TOWEL DISPENSER AT EACH SINK LOCATION.
- M. PROVIDE GRAB BARS, TOILET PAPER, AND TOILET SEAT COVER DISPENSERS AT EACH TOILET LOCATION.
- N. SEE A9 AND A10 SERIES FOR ENLARGED PLANS.
- O. SEE A9 SERIES FOR TYPICAL ADA MOUNTING HEIGHTS.

PLAN NOTES - FLOOR PLAN

- 1 HEAVY DUTY TRENCH DRAIN, CONCRETE TO SLOPE TO DRAIN.
- 2 PAINTED STRIPED WALKWAY.
- 3 LAPEYRE INDUSTRIAL STAIRS POWDER COATED CARBON STEEL BAR GRATING, 1.5" O.D. GUARDRAIL AND HANDRAIL (WITH EXTENSIONS) OR EQUAL.
- 4 NON-OPERABLE ALUMINUM FRAME STOREFRONT, 3'-0" W x 4'-0" H.
- 5 DOWNSPOUT, CONNECTED TO UNDERGROUND DRAIN PIPING.
- 6 1/2" ADVANTECH SHEATHING.
- 7 POWDER COATED 1.5" DIAMETER CARBON STEEL SAFETY RAILING WITH SURFACE MOUNTED BASE PLATE (4 HOLES), 4" KICK PLATE, MID RAIL, AND GUARDRAIL TUBE, SPACE COLUMNS 4'-0" O.C. PROVIDE SOLID WOOD BLOCKING FOR BASE PLATES, TYP.
- 8 ALL INTERIOR WALLS ARE 2X4 STUDS WITH 5/8" TYPE "X" GYP. BOARD U.N.O., PAINTED.
- 9 WALLS ARE WOOD STUDS WITH 5/8" PLYWOOD SHEATHING.
- 10 TOP LOCKING, OSHA COMPLIANT POWDER COATED MEZZANINE SAFETY SWING GATE, TOP LOCKING MECHANISM U CHANNEL THAT LAPS BOTH GATES 6".
- 11 WALLS UP TO GIRTS, PROVIDE BLOCKING FOR WALL PARALLEL TO GIRTS.
- 12 5" CONCRETE SLAB ON GRADE WITH DRAINAGE FILL BELOW.
- 13 BRACED LOCKING MECHANICAL ENCLOSURE GATE CLAD IN METAL PANELS BY PERM.
- 14 NON-OCCUPIED SPACE.
- 15 STAINLESS STEEL 2 BASIN STANDALONE SINK. SEE MEP SHEETS FOR PLUMBING COORDINATION.

EQUIPMENT SCHEDULE

E1	60 INCH TELEVISION	OWNER	OWNER
E2	21" O X 48" W INDUSTRIAL WIRE SHELVING	OWNER	OWNER



PARTITION TYPE W

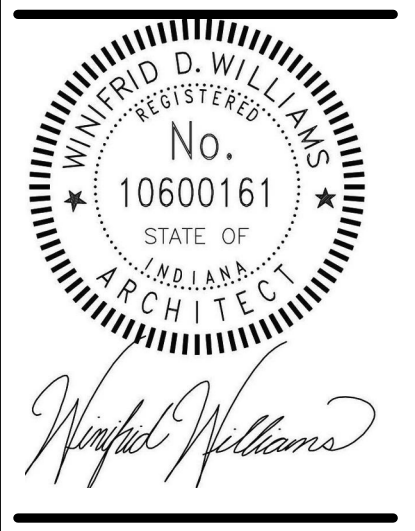
WALL TYPE	STUD WIDTH	PARTITION WIDTH
W1	3-1/2"	<varies>
W2	5-1/2"	6-3/4"



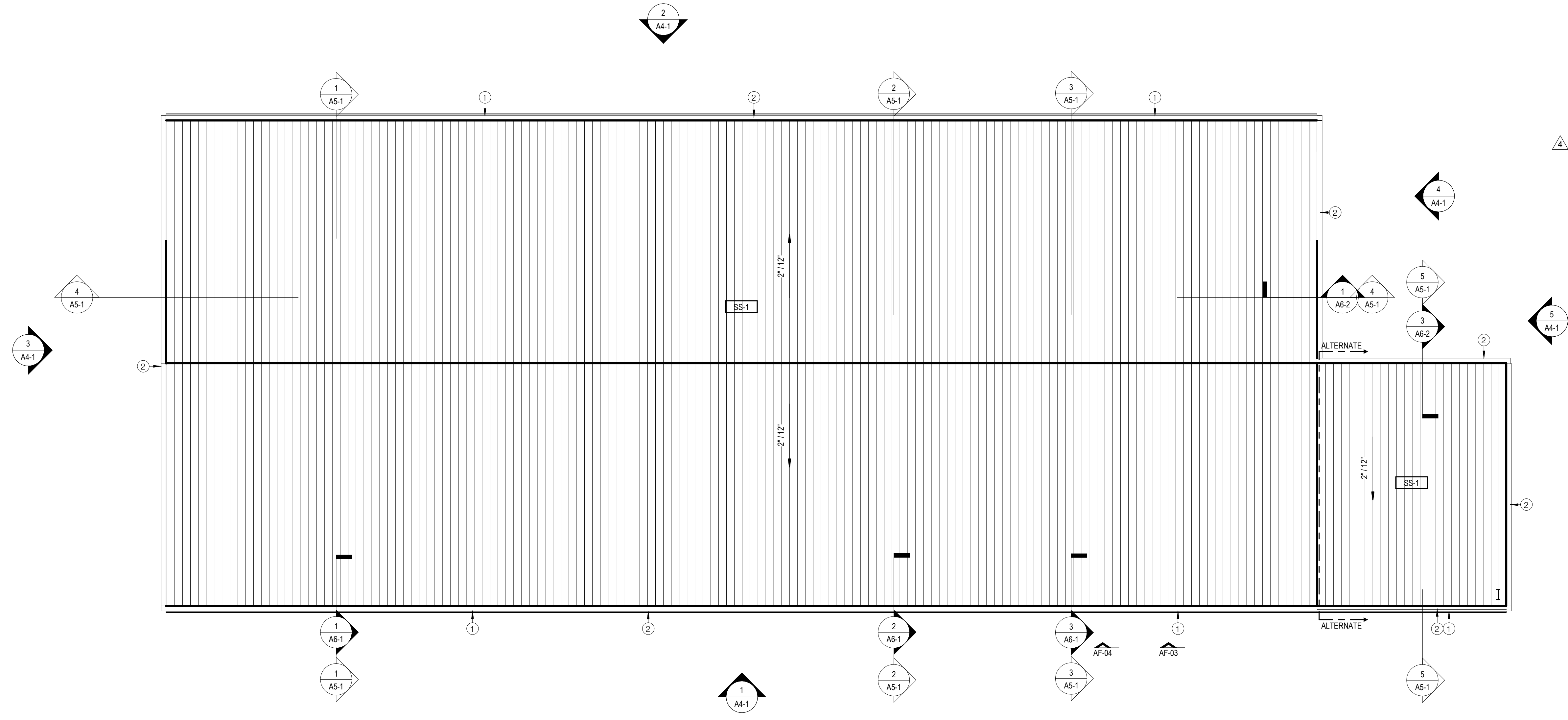
REVISIONS

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04/10/26  
 LAKELAND SCHOOL CORPORATION  
**25082 - PROJECT B.1 - MAINTENANCE BUILDING**  
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PRICING SET  
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 25082  
 DRAWN BY  
 BWD  
 DRAWING NAME  
**ARCHITECTURAL FLOOR PLAN**  
 DRAWING NO.  
**A1-1**



**1 ROOF PLAN**  
SCALE: 1/8" = 1'-0"

**GENERAL NOTES - ROOF PLAN**

- A. PROVIDE MANUFACTURER'S STANDARD DETAILS WHERE MECHANICAL EQUIPMENT OCCURS. COORDINATE W/ ARCHITECTURAL AND MEP.
- B. SEE MECHANICAL, ELECTRICAL, AND PLUMBING DRAWINGS FOR ADDITIONAL ROOF PENETRATION REQUIREMENTS.
- C. PROVIDE ICE AND WATER SHIELD AT ALL OVERHANGS - FROM EDGE OF ROOF TO 24" PAST INTERIOR LINE OF WALL. VALLEYS (3'-0" ON EACH SIDE), RIDGES (3'-0" ON EACH SIDE) AND HIPPS (3'-0" ON EACH SIDE).

**PLAN NOTES - ROOF PLAN**

- 1. PREFINISHED 5X5 BOX GUTTERS. COLOR TO BE SELECTED BY ARCHITECT.
- 2. FRAMED OUT METAL PANEL ACCENT. SEE SECTIONS FOR DETAILS.

**ROOF SCHEDULE**

SS-1	STANDING SEAM METAL ROOF PANEL SYSTEM OVER GIRTS BY PEMB. ARCHITECT TO SELECT FROM FULL COLOR RANGE.
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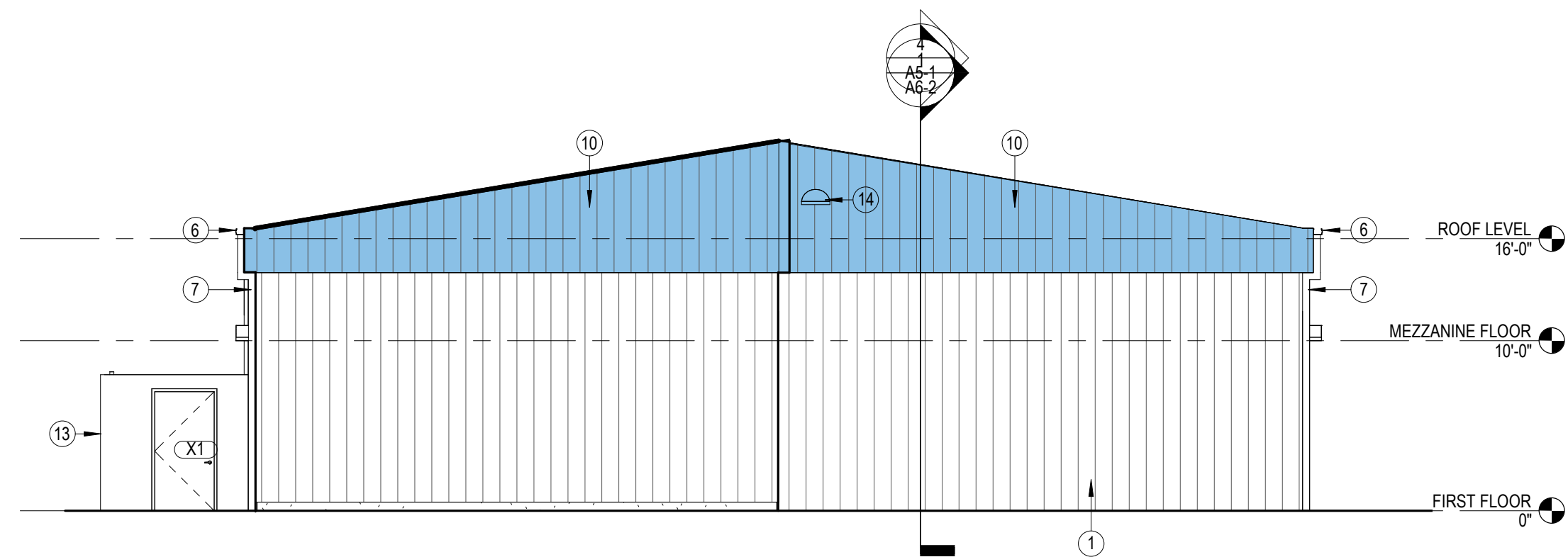
**ROOF SYMBOLS**

- XXX-1 — ROOF TYPE SYMBOL
- RD — 6" ROOF DRAIN
- ORD — 6" OVERFLOW ROOF DRAIN
- [Symbol] — ROOF DRAINAGE SADDLE
- [Symbol] — DIRECTIONS OF SLOPE
- [Symbol] — RELIEF HOOD

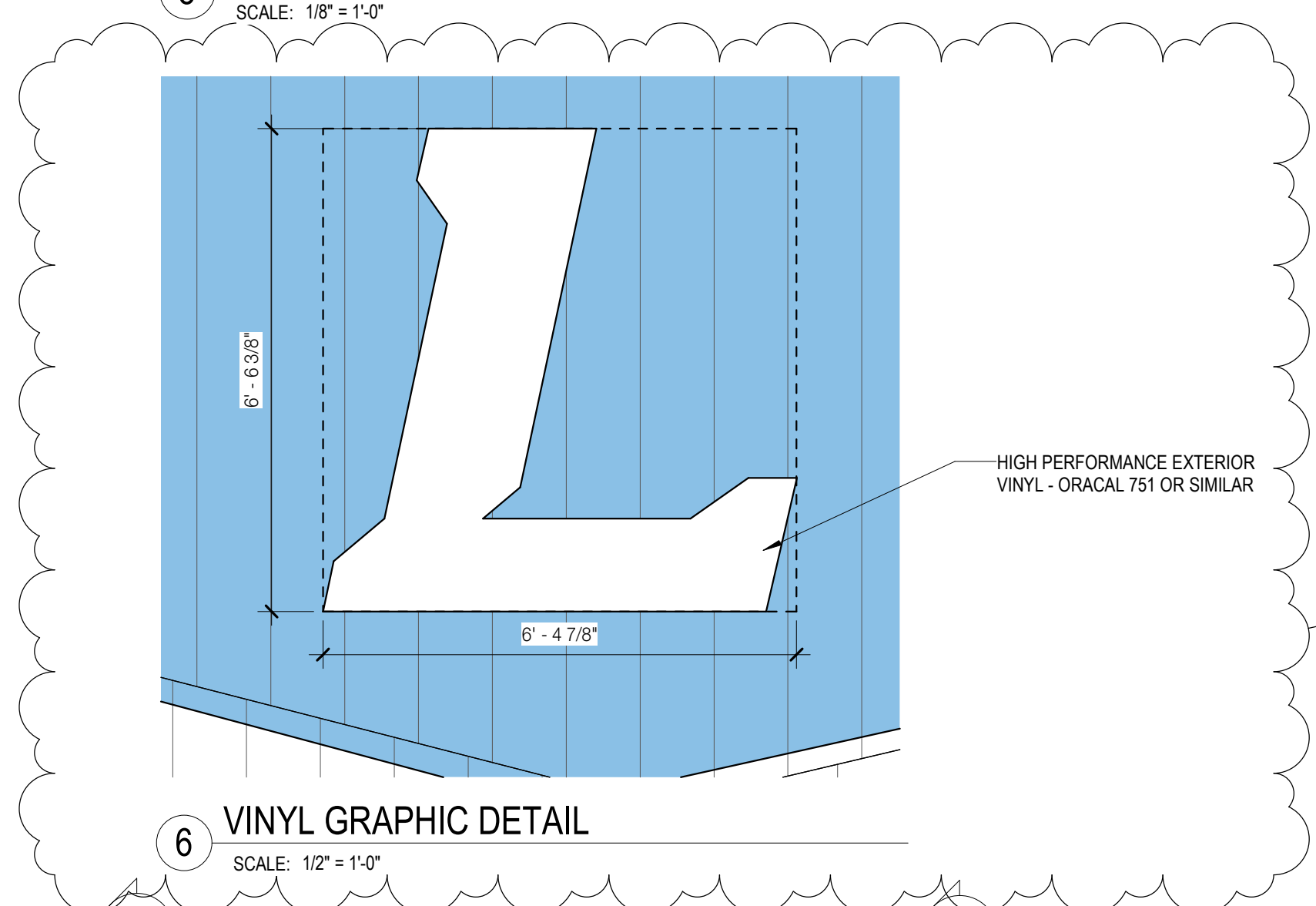
PRICING SET  
 04/10/26  
 kM JOB NO.  
 25082  
 DRAWN BY  
 Author

DRAWING NAME  
**ROOF PLAN**

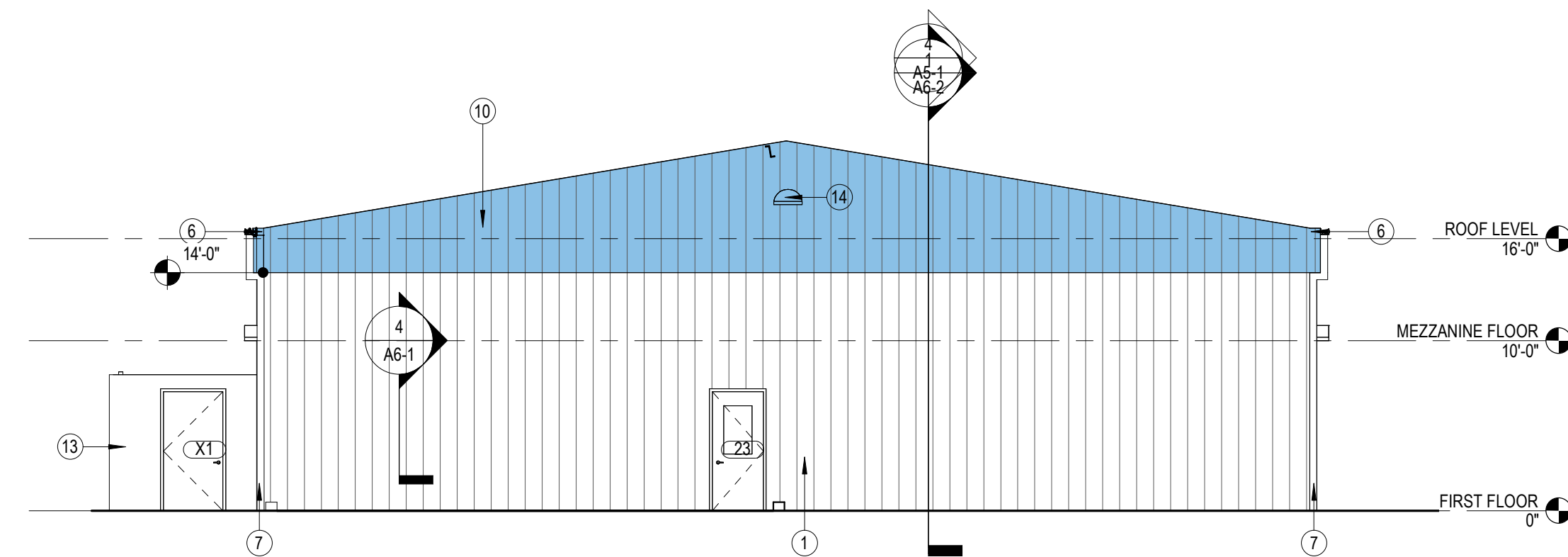
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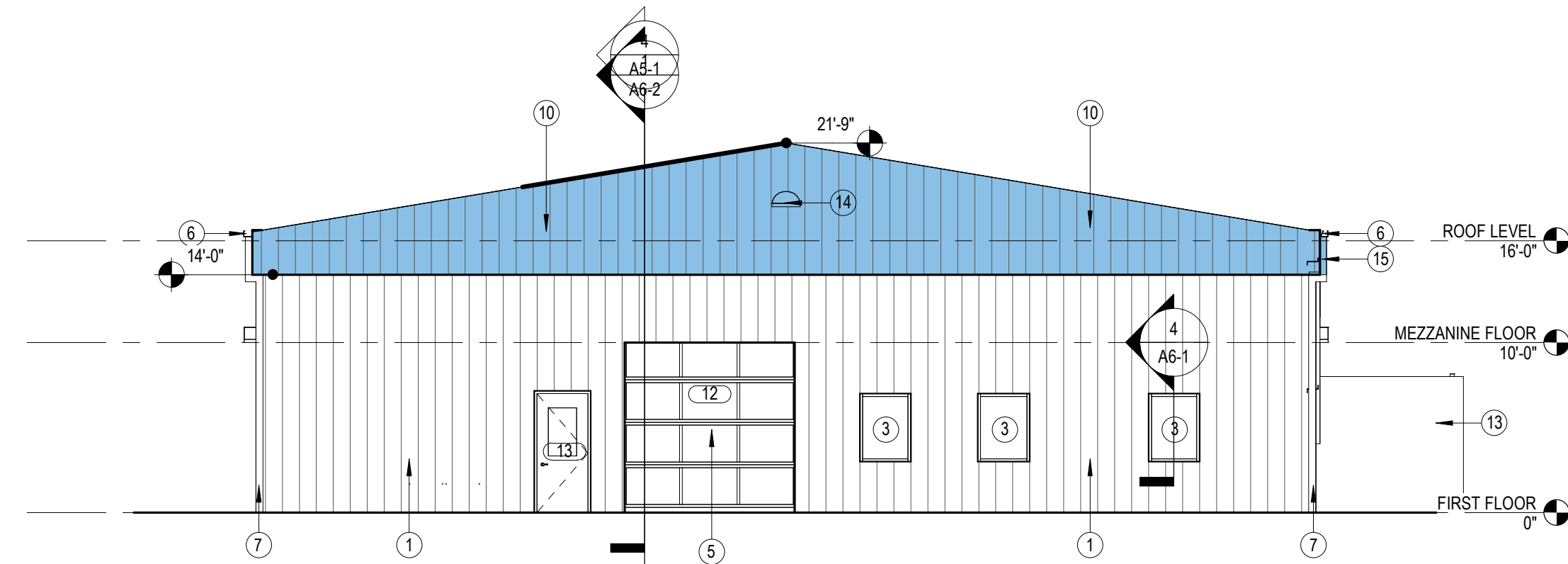
5 EAST ELEVATION - ALTERNATE  
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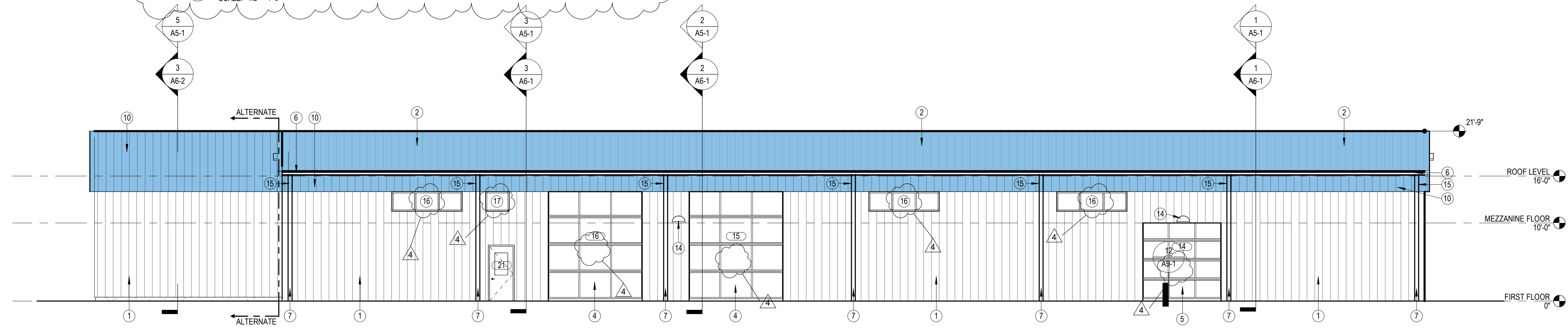
6 VINYL GRAPHIC DETAIL  
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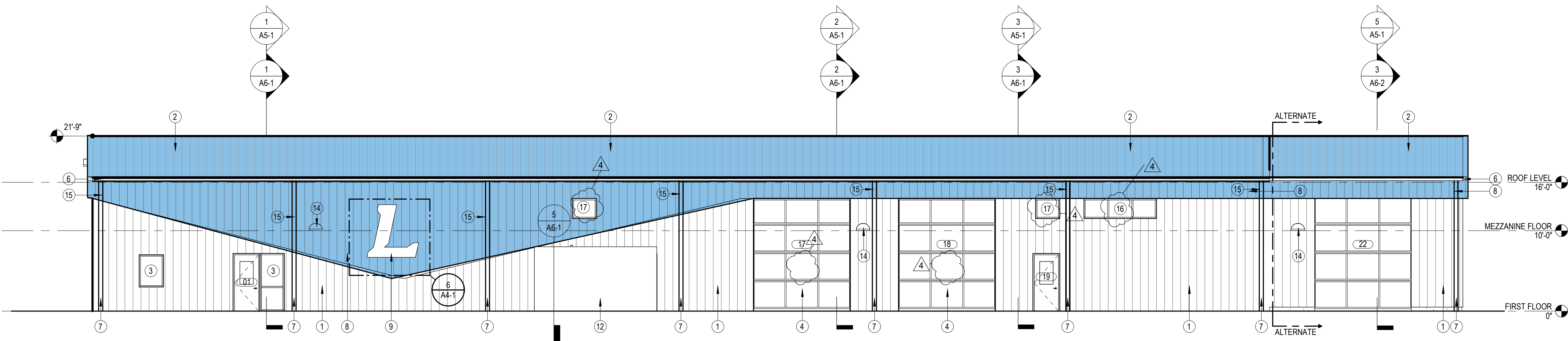
4 EAST ELEVATION  
SCALE: 1/8" = 1'-0"



3 WEST ELEVATION  
SCALE: 1/8" = 1'-0"



2 NORTH ELEVATION  
SCALE: 1/8" = 1'-0"



1 SOUTH ELEVATION  
SCALE: 1/8" = 1'-0"

ELEVATION NOTES - EXTERIOR

- 1 EXPOSED FASTENER METAL SIDING BY P.E.M.B. SUPPLIER (BASIS OF DESIGN: NUCOR A-PANEL OR SIMILAR)
- 2 PRE-FINISHED STANDING SEAM METAL ROOF BY P.E.M.B. SUPPLIER. COLOR: HAWAII BLUE
- 3 NON-OPERABLE ALUMINUM FRAME STOREFRONT W/ INSULATED GLAZING. 3'-0" W x 4'-0" H
- 4 14' H x 12' W INSULATED STEEL OVERHEAD DOOR BY P.E.M.B. SUPPLIER
- 5 10' H x 10' W INSULATED STEEL OVERHEAD DOOR BY P.E.M.B. SUPPLIER
- 6 METAL GUTTER BY P.E.M.B. SUPPLIER
- 7 DOWNSPOUT, CONNECTED TO UNDERGROUND DRAIN PIPING
- 8 CONCEALED FASTENER METAL SIDING BY P.E.M.B. SUPPLIER. THIS SECTION IS PROUD OF EXPOSED FASTENER PANEL BY 6" COLOR: BLUE TO MATCH LAKELAND STANDARDS
- 9 HIGH PERFORMANCE EXTERIOR VINYL GRAPHIC APPLIED TO METAL PANEL. COLOR TO BE DETERMINED BY ARCHITECT. FOLLOW METAL PANEL CORRUGATIONS WITH VINYL.
- 10 CONCEALED FASTENER METAL SIDING BY P.E.M.B. SUPPLIER. THIS SECTION IS FLUSH WITH EXPOSED FASTENER PANEL. COLOR: BLUE TO MATCH LAKELAND STANDARDS
- 11 MECHANICAL ENCLOSURE - MATCH METAL PANEL FINISH AND RIBBING LOCATION WITH MAIN BUILDING.
- 12 METAL PANEL MECHANICAL ENCLOSURE AND ACCESS GATE BY PEMB.
- 13 EXTERIOR WALL PACK - SEE ELECTRICAL.
- 14 WHERE DOWNSPOUTS JOG FOR METAL PANEL FASCIA DETAIL, MATCH TO METAL PANEL COLOR OF WALL BEHIND, TYP.
- 15 NON-OPERABLE ALUMINUM FRAME STOREFRONT W/ INSULATED GLAZING. 3'-0" W x 2'-6" H
- 16 NON-OPERABLE ALUMINUM FRAME STOREFRONT W/ INSULATED GLAZING. 3'-0" W x 2'-6" H



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*Winifred Williams*

PRICING SET  
04/10/26  
kM JOB NO.  
25082  
DRAWN BY  
BWD

DRAWING NAME  
**EXTERIOR ELEVATIONS**

DRAWING NO.  
**A4-1**



REVISIONS

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 E 075 N. LAGRANGE, IN 46761  
 PRICING SET

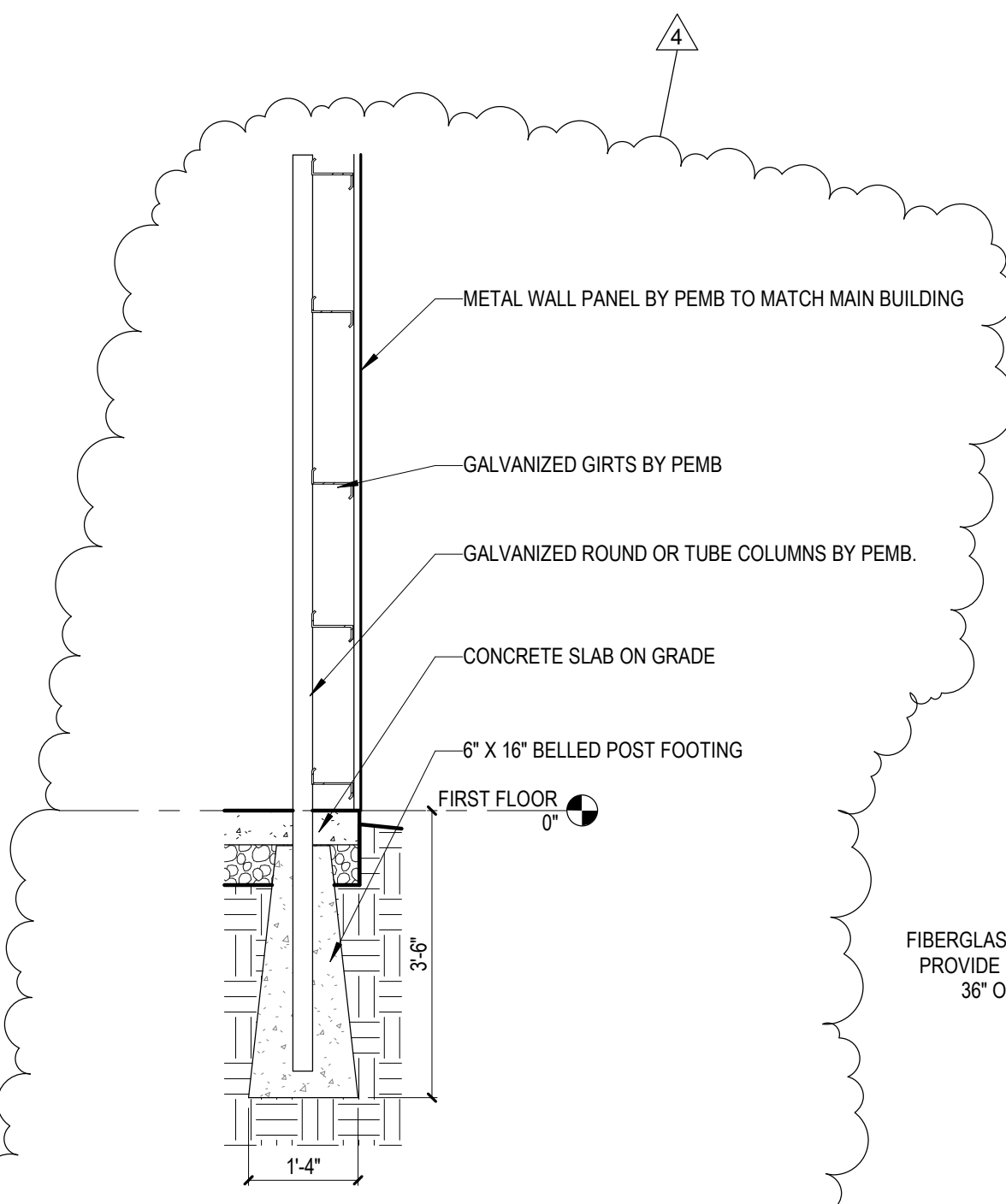


Winifred Williams

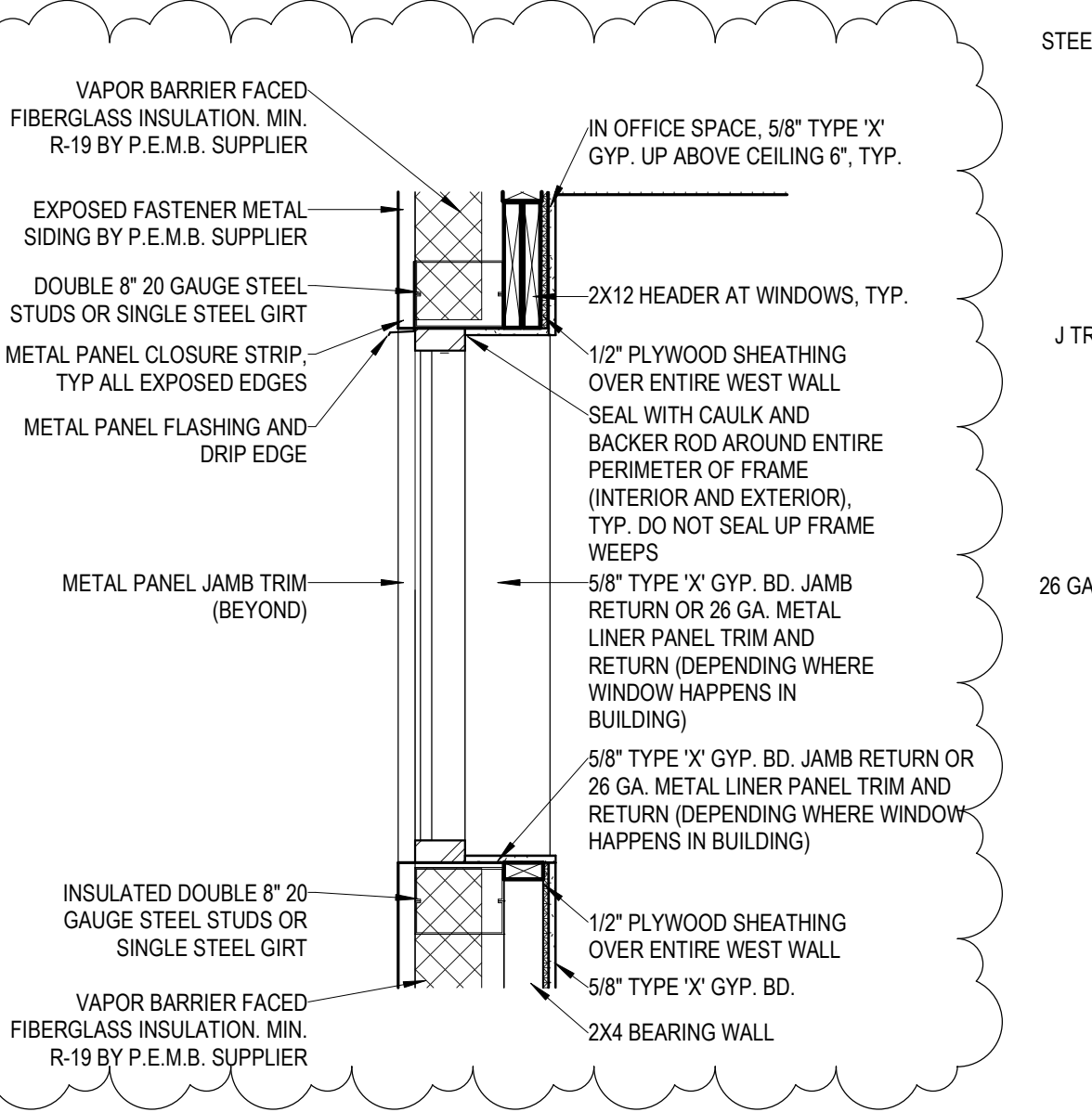
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 km JOB NO.  
 25082  
 DRAWN BY  
 BWD

DRAWING NAME  
WALL SECTIONS & DETAILS

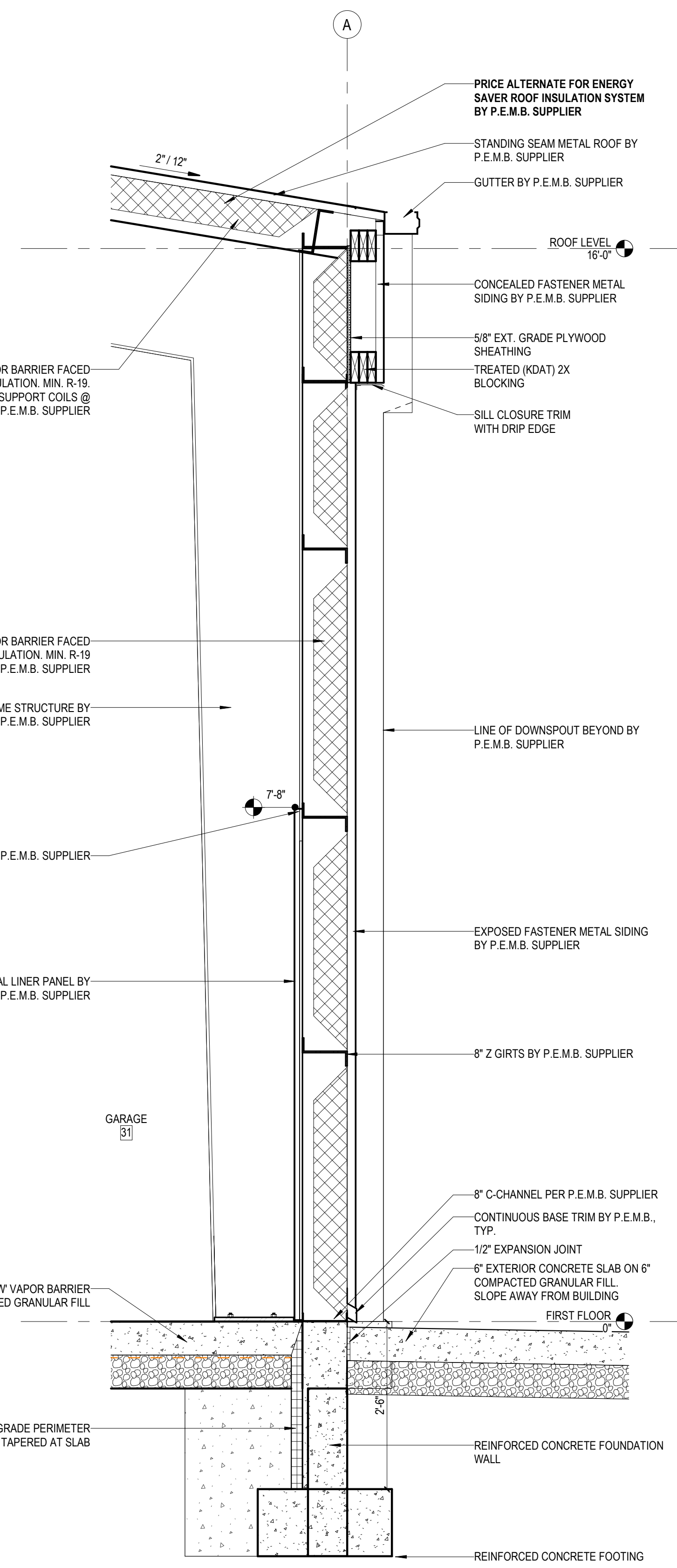
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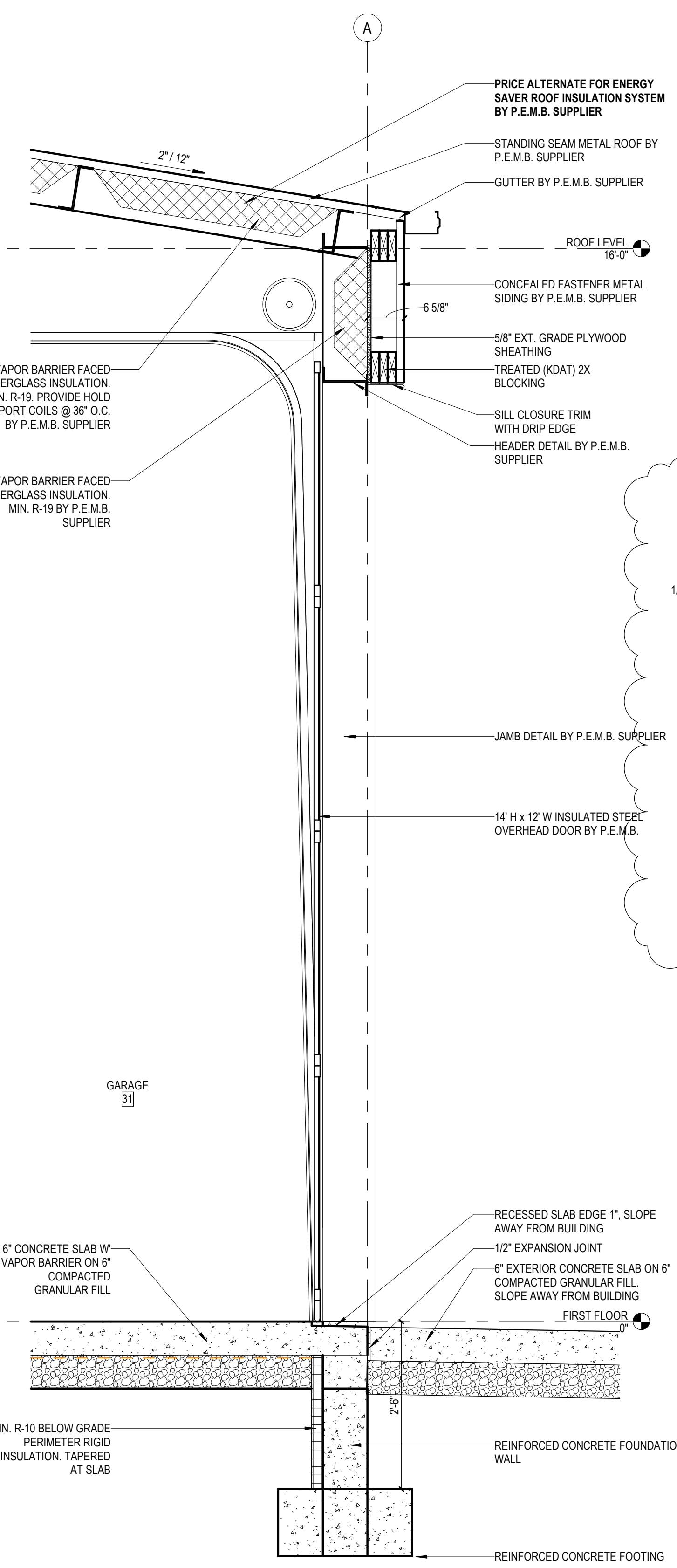
5 MECH. ENCLOSURE TYP. SECTION  
SCALE: 1/2" = 1'-0"



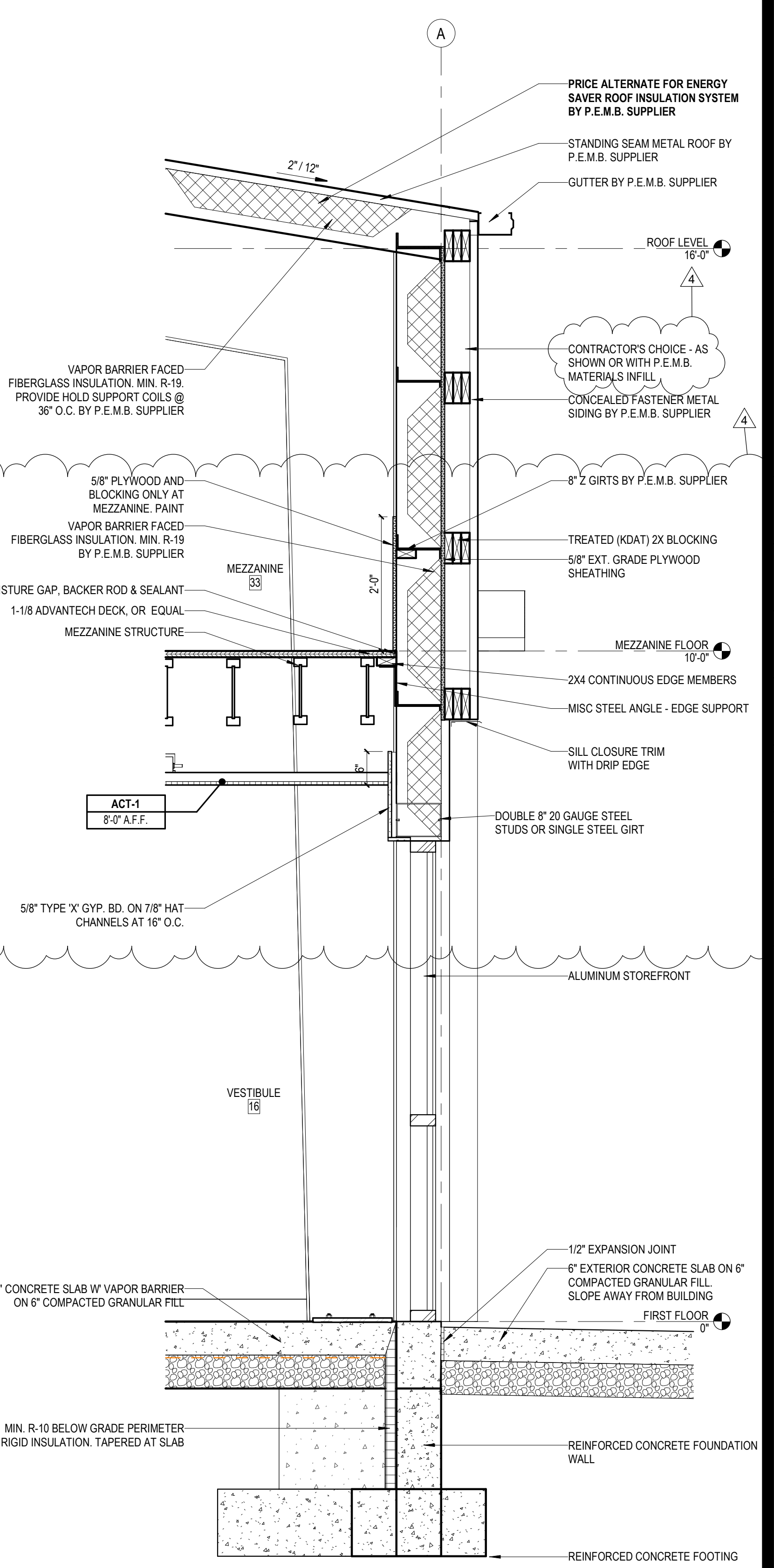
4 TYP. ALUM FRAME SECTION  
SCALE: 3/4" = 1'-0"



3 Wall Section 3  
SCALE: 3/4" = 1'-0"



2 Wall Section 2  
SCALE: 3/4" = 1'-0"



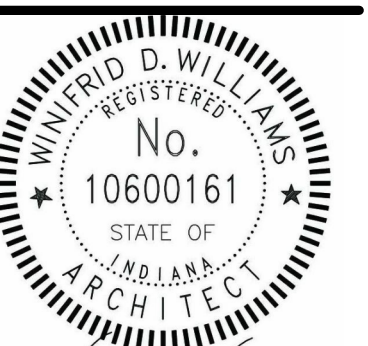
1 Wall Section 1  
SCALE: 3/4" = 1'-0"



REVISIONS

4	06/04/26	Addendum 4
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04/10/26  
LAKELAND SCHOOL CORPORATION  
**25082 - PROJECT B.1 - MAINTENANCE BUILDING**  
E 075 N. LAGRANGE, IN 46761  
PRICING SET

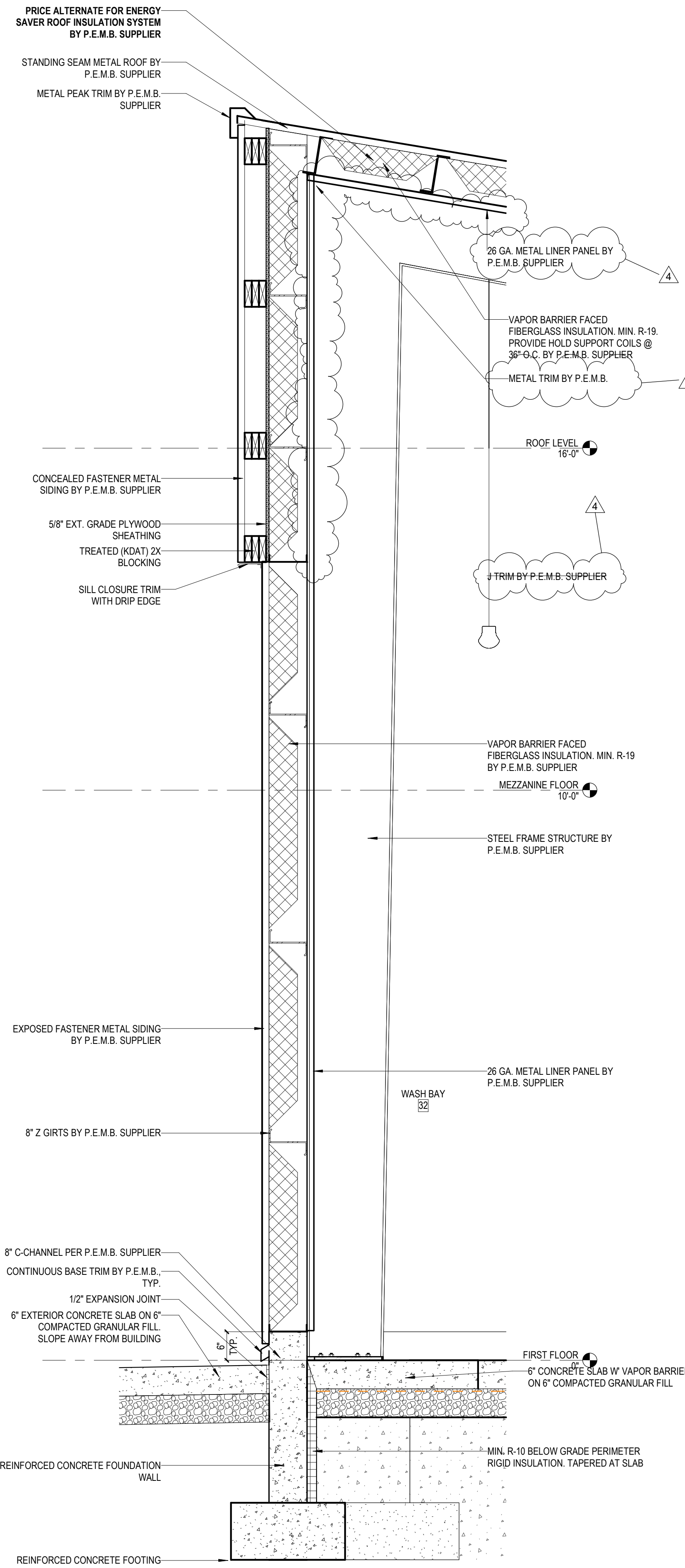


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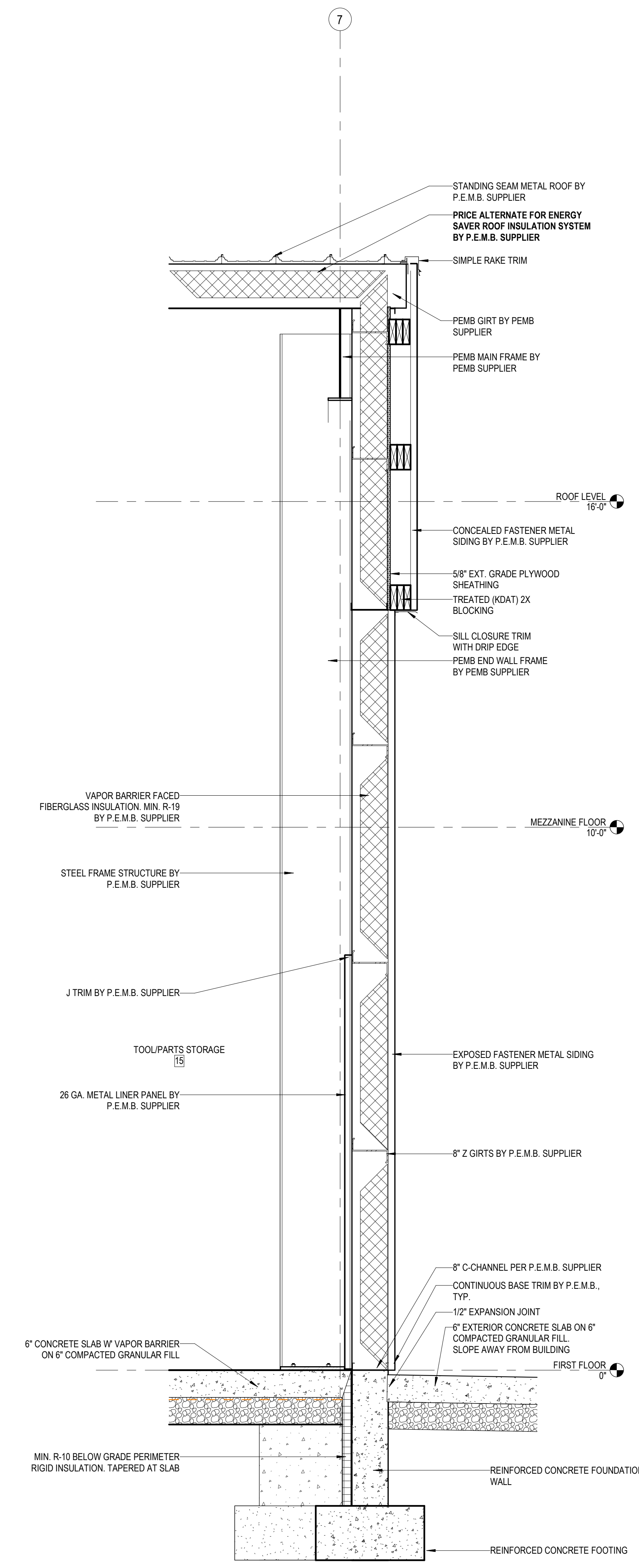
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04/10/26  
kM JOB NO.  
25082  
DRAWN BY  
BWD

DRAWING NAME  
**WALL SECTIONS & DETAILS**

DRAWING NO.  
**A6-2**



3 Wall Section 4  
SCALE: 3/4" = 1'-0"



1 Wall Section 7  
SCALE: 3/4" = 1'-0"



REVISIONS	
4	05/04/26 Addendum 4

**GENERAL NOTES - ENLARGED PLANS**

- A. SEE SHEET A9-1 FOR TYPICAL ADA MOUNTING HEIGHTS FOR ACCESSORIES.
- B. CONTRACTOR TO PROVIDE BLOCKING IN WALLS FOR ALL TOILET ROOM ACCESSORIES AND PARTITIONS FOR OWNER AND CONTRACTOR SUPPLIED ITEMS.
- C. COORDINATE FLOOR DRAIN LOCATIONS WITH MEP.
- D. CENTER ALL TILE PATTERNS, LEAVING EQUAL SIZE TILES ON EACH END OF PARTITION. SEE SPECIFICATIONS.
- E. C.F.C.I. = CONTRACTOR FURNISHED, CONTRACTOR INSTALLED.
- F. O.F.C.I. = OWNER FURNISHED, CONTRACTOR INSTALLED.
- G. O.F.O.I. = OWNER FURNISHED, OWNER INSTALLED.
- H. T.F.O.I. = TENANT FURNISHED, CONTRACTOR INSTALLED.
- I. G.C. AND ALL SUB-CONTRACTORS PERFORMING WORK IN AND NEAR THE RESTROOMS SHALL CONFIRM THEIR WORK EFFORTS, MAINTAIN ALL CLEARANCES NOTED, AND COORDINATE CLEARANCES REQUIRED WITH ALL OTHER TRADES.
- J. FIXTURES SHALL NOT OVERLAP INTO AREAS OF OTHER FIXTURE'S CLEARANCES.
- K. SINK CLEARANCE SHALL BE 2'-6" W. X 4'-0" D., ALLOWING CLEARANCE AREA TO EXTEND UNDER THE SINK BY 8".
- L. TOILET CLEARANCE AREA REQUIRED SHALL BE 5'-0" W. X 5'-6" D. THE TOILET MAY OVERLAP THIS CLEARANCE AREA.
- M. SHOWER CLEARANCE AREA REQUIRED SHALL BE 3'-0" D X 4'-0" W.
- N. 60" TURNING RADIUS SHALL BE PROVIDED WITHIN THE RESTROOM. THE TURNING RADIUS MAY OVERLAP THE FIXTURE'S CLEARANCE AREA, BUT MAY NOT OVERLAP THE ACTUAL FIXTURES.

**ELEVATION NOTES - INTERIOR**

- 1 25" DEEP COUNTERTOP WITH BACKSPLASH & SIDE SPLASHES.
- 2 24" DEEP ADA SINK BASE.
- 3 24" DEEP 3-DRAWER BASE.
- 4 24" DEEP 2 DOOR, 1 DRAWER BASE CABINET.
- 5 EQUIPMENT BY OWNER.

04/10/26  
 LAKELAND SCHOOL CORPORATION  
**25082 - PROJECT B.1 - MAINTENANCE BUILDING**  
 E 075 N. LAGRANGE, IN 46761  
 PRICING SET

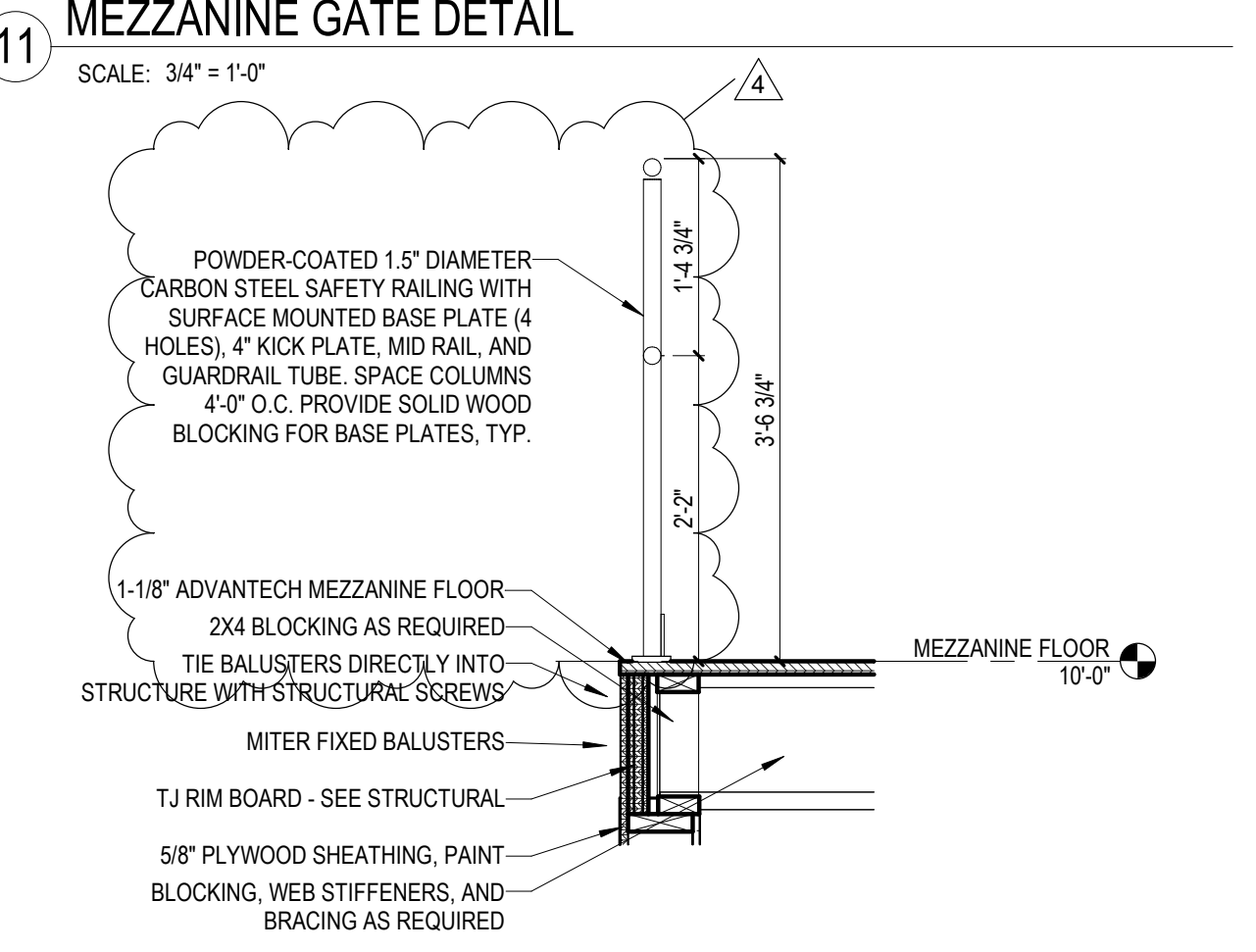
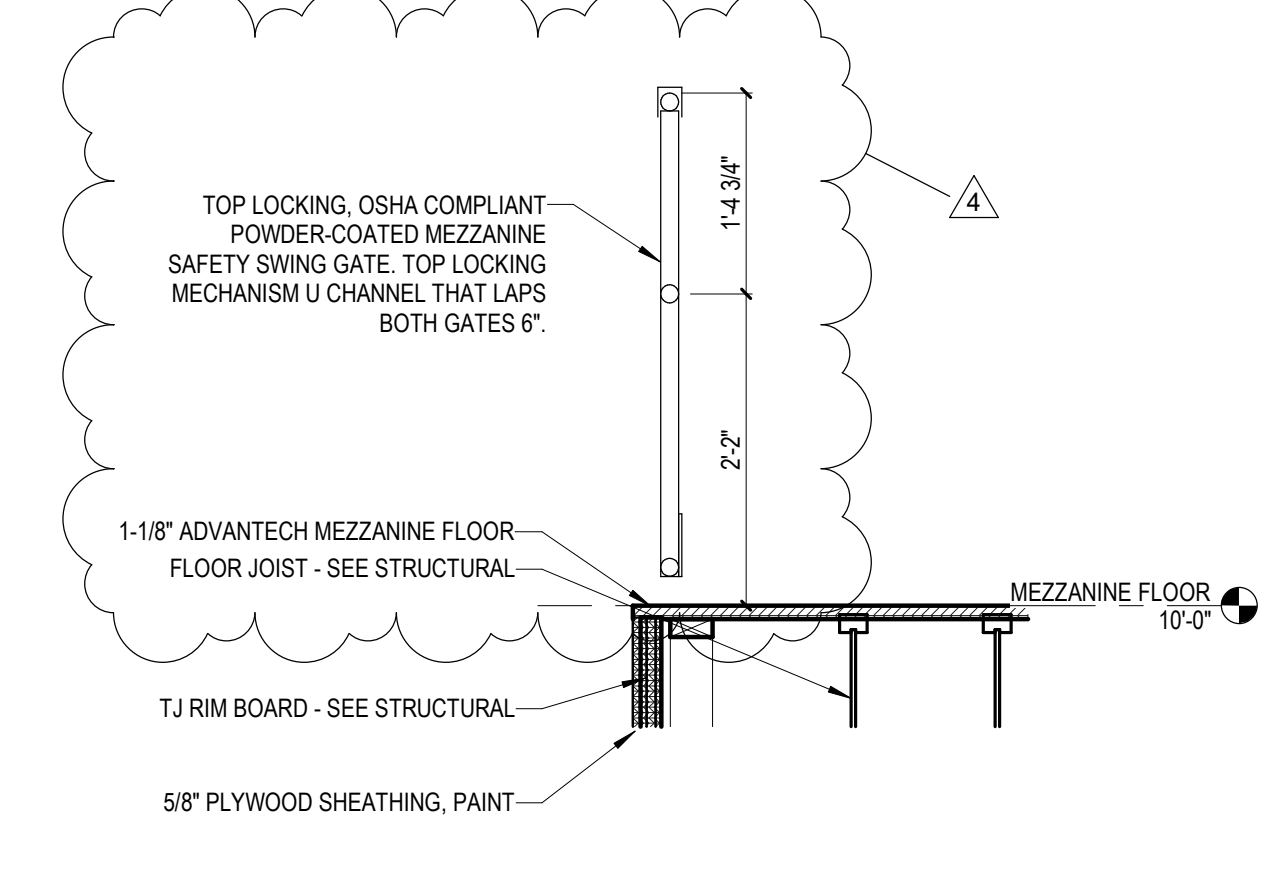
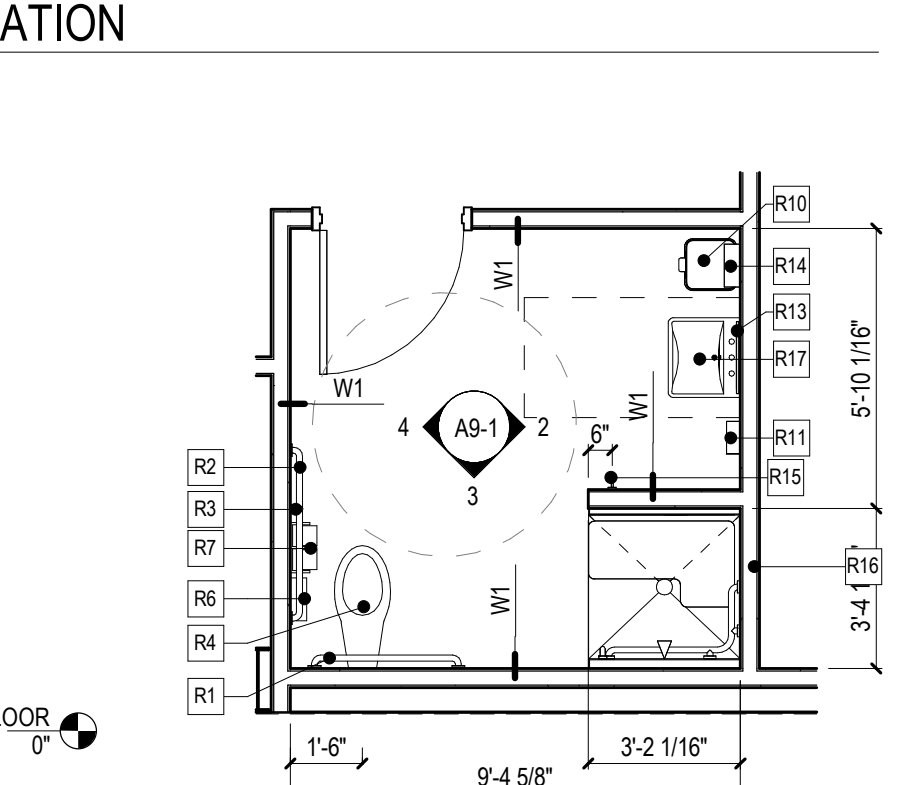
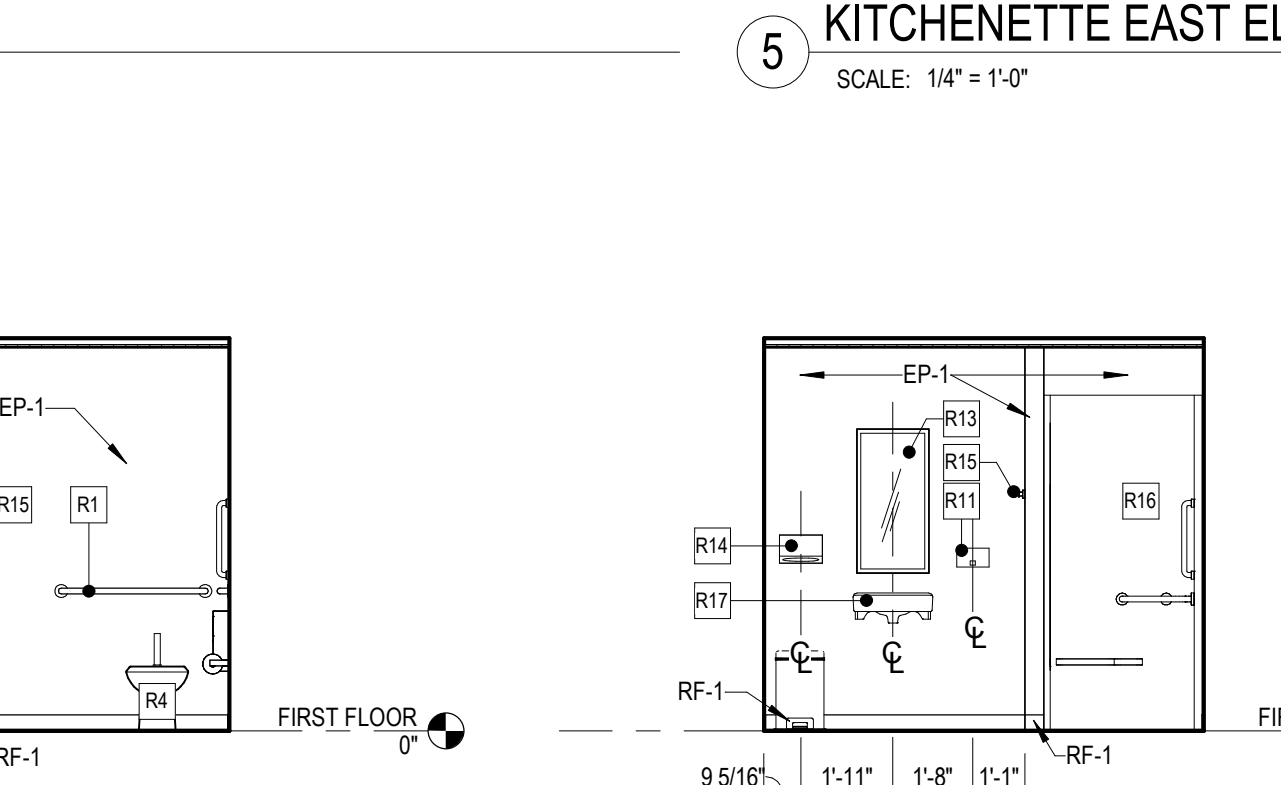
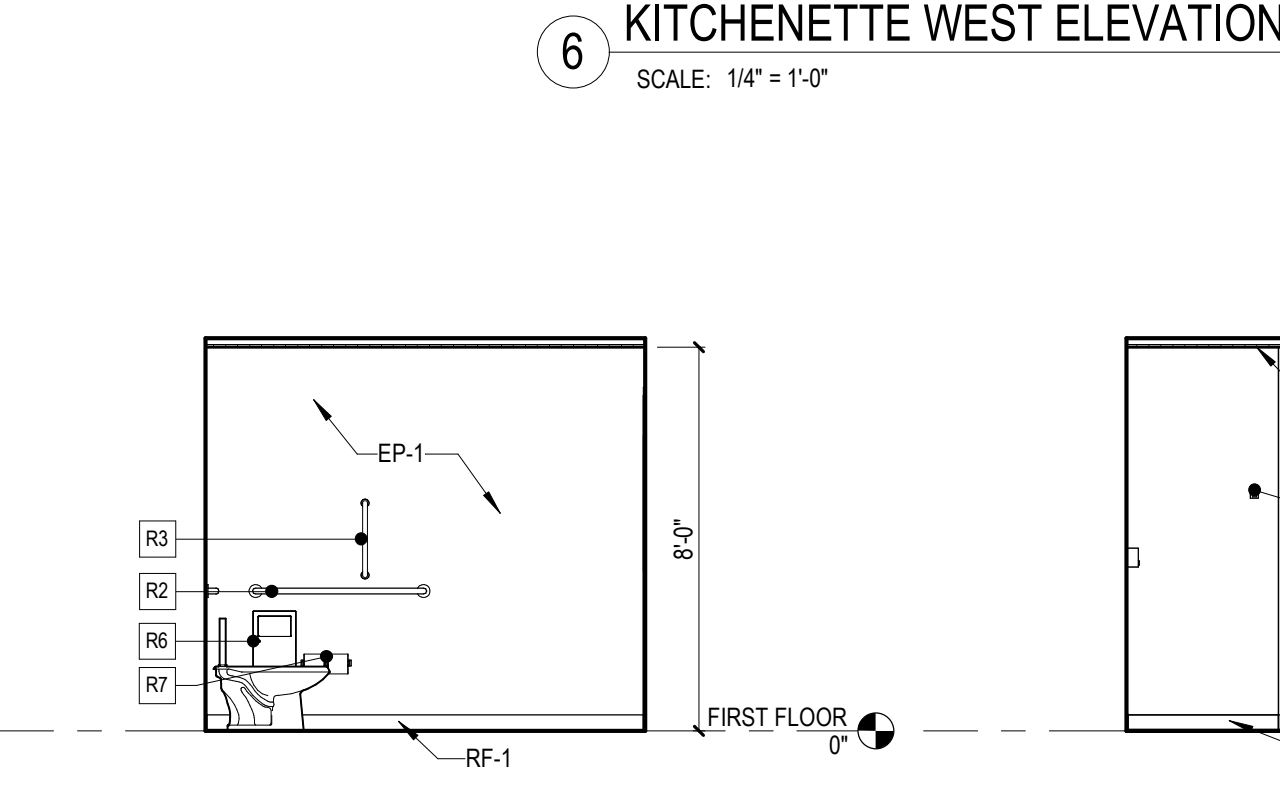
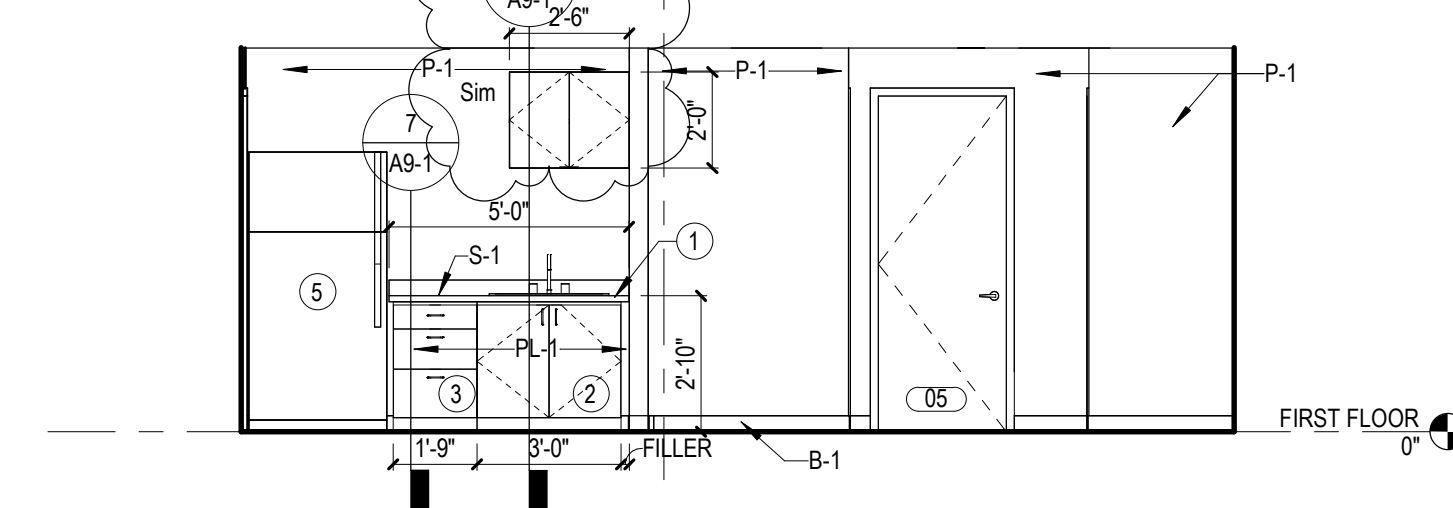
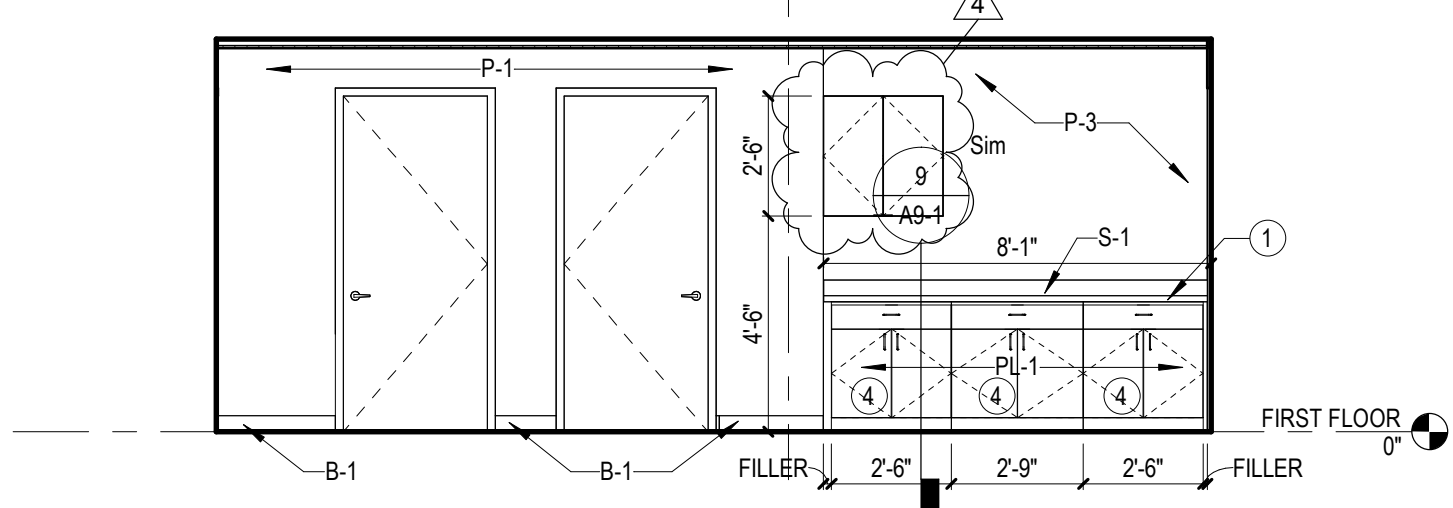
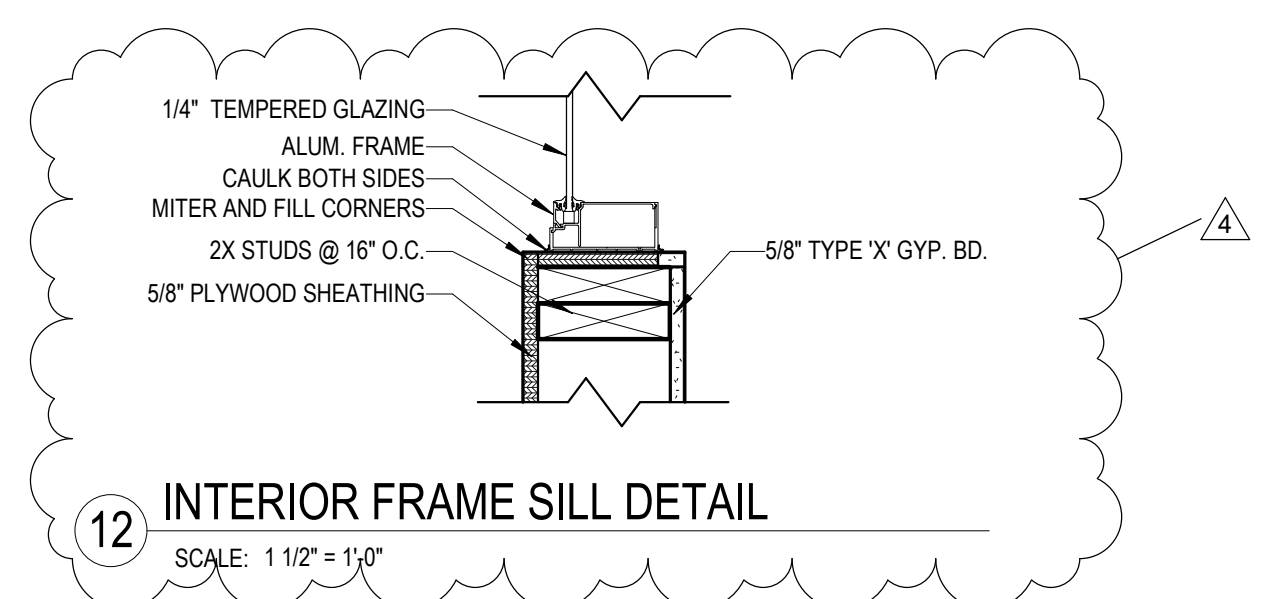


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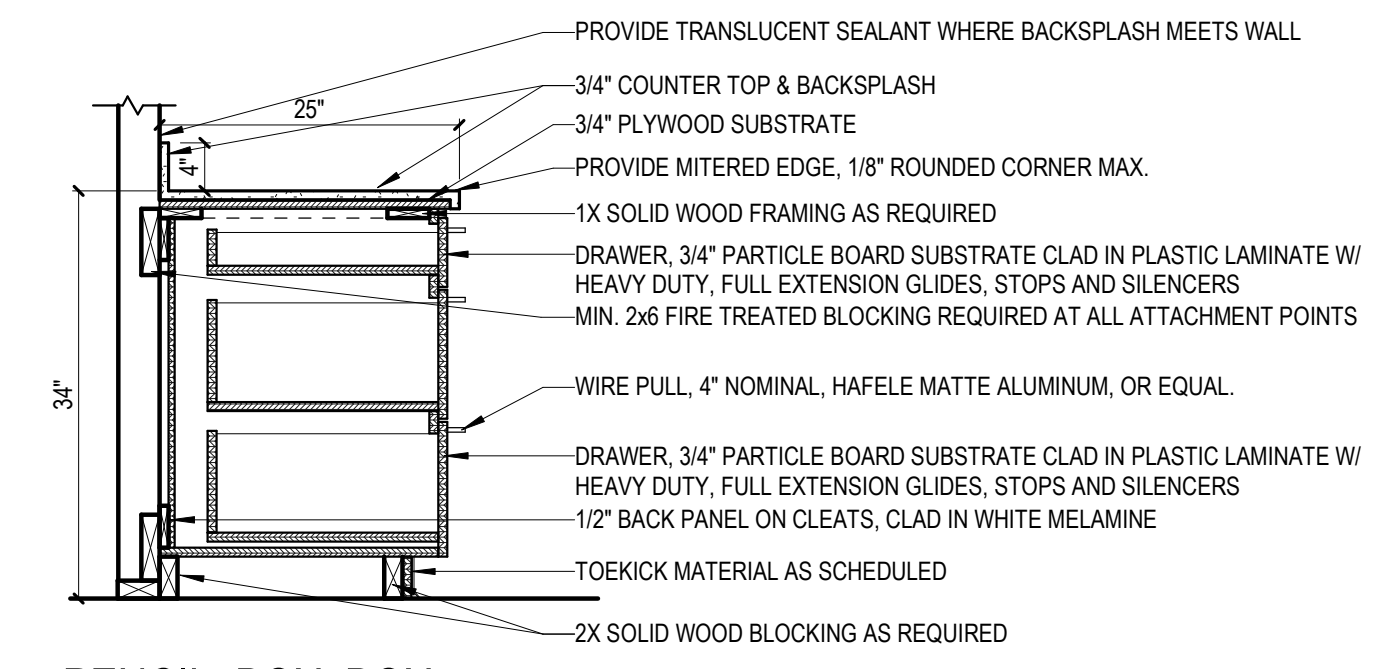
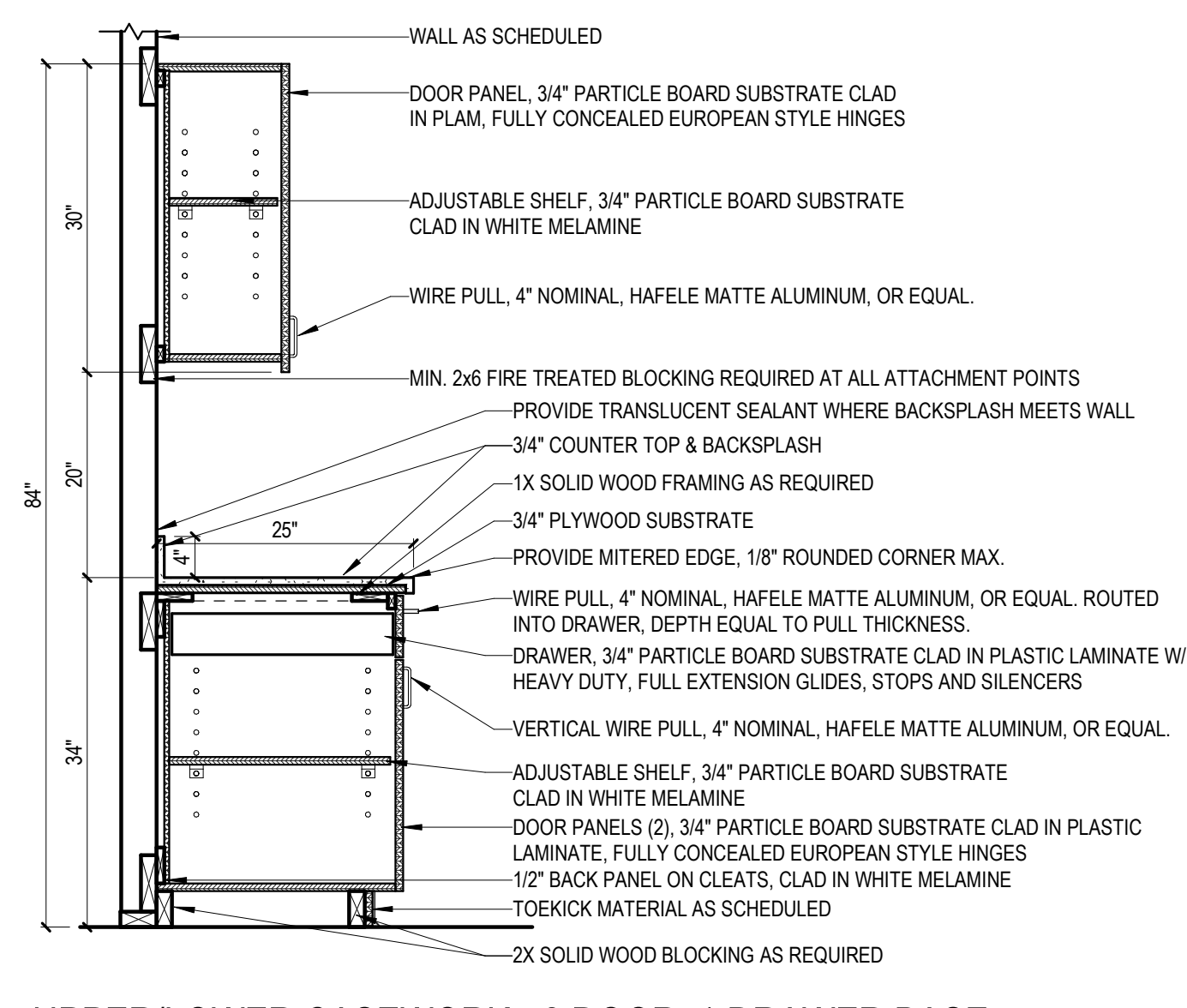
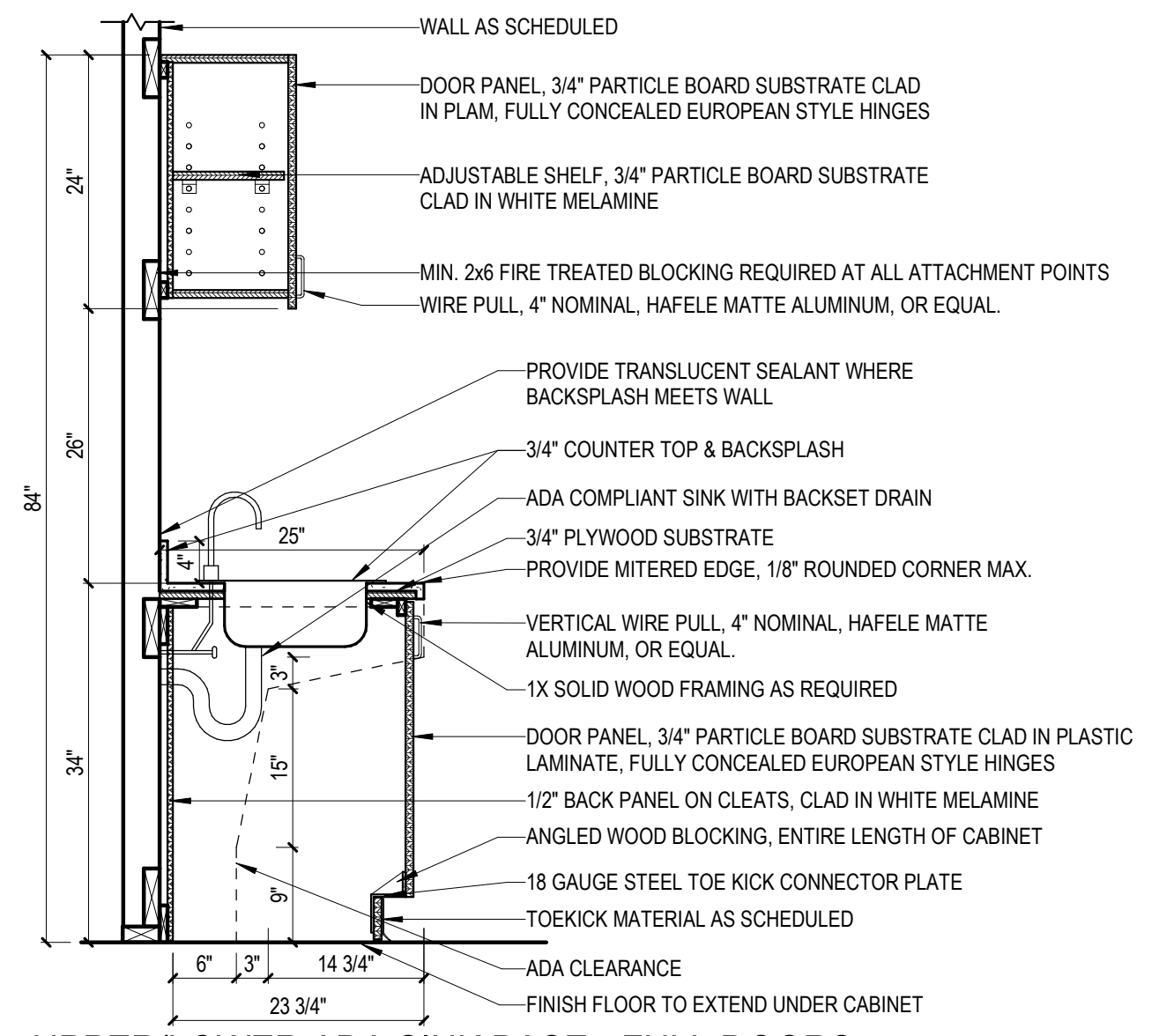
PRICING SET  
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 kM JOB NO.  
 25082  
 DRAWN BY  
 MK

DRAWING NAME  
**ENLARGED FLOOR PLANS & ELEVATIONS**

DRAWING NO.  
**A9-1**



NOTE: INSULATE ALL PIPING UNDER CASEWORK TO PREVENT INJURIES



RESTROOM EQUIPMENT			
R1	BACK WALL GRAB BAR: BOBRICK B-5806 SERIES	CONTRACTOR	CONTRACTOR
R2	SIDE WALL GRAB BAR: BOBRICK B-5806 SERIES	CONTRACTOR	CONTRACTOR
R3	SIDE WALL VERTICAL GRAB BAR: BOBRICK B-5806 SERIES	CONTRACTOR	CONTRACTOR
R4	TOILET, REF. PLUMB	CONTRACTOR	CONTRACTOR
R6	SANITARY NAPKIN DISPOSAL: BOBRICK B-9254, SURFACE-MOUNTED	CONTRACTOR	CONTRACTOR
R7	TOILET PAPER DISPENSER: ASI 0030 TWIN HIDE-A-ROLL, SURFACE-MOUNTED, STAINLESS STEEL	OWNER	CONTRACTOR
R10	TRASH CAN: FREESTANDING, STAINLESS STEEL	OWNER	OWNER
R11	SOAP DISPENSER: GOJO REFILLABLE FOAM SOAP DISPENSER	OWNER	CONTRACTOR
R13	MIRROR: BOBRICK B-165 SERIES	CONTRACTOR	CONTRACTOR
R14	PAPER TOWEL DISPENSER: BOBRICK B-9262 SURFACE-MOUNTED	CONTRACTOR	CONTRACTOR
R15	ROBE HOOK: BOBRICK B-76717 CLASSIC SERIES SINGLE HOOK, SATIN FINISH	CONTRACTOR	CONTRACTOR
R16	ADA-COMPLIANT SHOWER: INPRO 36" X 36" FINISHED ADA TRANSFER SHOWER BASE W/ PRISM SOLID SURFACE SHOWER WALL PANELS WITH BEVELED EDGE TRIM, OUTSIDE CORNER PERIMETER TRIM, AND PANEL MOLDING. INCLUDE HORIZONTAL CORNER GRAB BAR AND 18" VERTICAL GRAB BAR. PROVIDE: INPRO L-SHAPE FOLDING PHENOLIC SEAT WITH LEGS, 8X16 RECESSED SHOWER NICHE, AND 36" SHOWER CURTAIN SYSTEM KIT.	CONTRACTOR	CONTRACTOR
R17	WALL-MOUNTED LAVATORY, REF. PLUMB.	CONTRACTOR	CONTRACTOR
RRR			

**MOUNTING HEIGHT LEGEND**

