



Addendum #4
Southern Wells Community Schools
Raiders Activity Center and Raider Dome Lockers

Date: April 03, 2026
Project: Southern Wells Community Schools Raiders Activity Center and Raider Dome Lockers
Project #: 25048
Pages: 1 of 5 (8-1/2x11) pages
Bid Dates: THURSDAY, April 9, 2026 at 4:00 pm (prevailing local time)

General Note:

The original Specifications and Drawings dated April 03, 2026 for the project referenced above are amended as noted in this Addendum No. 4. Receipt of this Addendum and any subsequent Addenda must be acknowledged on the Bid Form. Items changed or added by this addendum are to take precedence over the items or descriptions of the work in the project manual and the drawings. Items not mentioned in this addendum are to remain as described in the original plans and specifications.

Additional Information:

Matterport 3D Scan of Raider Dome Locker Rooms

1. <https://my.matterport.com/show/?m=Rj21oViDANB>

Questions and Answers:

1. Q: There is no sprinkler scope indicated. Should we carry a number for the sprinkler system on the new building?
 - a. **No fire suppression is included in the project**
2. Q: Where do the foundation drains lead to?
 - a. **No foundation drains are required on the project.**
3. Q: A11-2 indicates we are to follow IHSAA rules and layout regulations. As of when are the requirements that we need to follow dated?
 - a. **All rules and layout regulations are to be per National Federation of State High School Associations rules in 2025-2026 Rule Book publications**
4. Q: C4.0 Utility note 9 references a detail #8 that does not exist on C6.1.
 - a. **Trash rack detail is provided as 9/C6.2**
5. Q: Is there a Geotech report that we can review prior to bidding?
 - a. **Yes, see specification attached to this addendum**
6. Q: Color of the new storefront frames: No color is clearly specified, and the existing High school building has Clear anodized, while the elementary has Dark bronze anodized. Which is required?
 - a. **Clear anodized to match the High School frames.**

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7. Q: Storefront type: Specifications state the frames should be "Centered Glazing Position," but the drawing details all show a "Front Glazing Position." Which is to be used?
 - a. **Please use Front Glazing position for exterior glazing and Centered Glazing position for interior glazing.**
8. Q: The glazing specifications and performance requirements do not align. Is there tint on this project for insulated glazing units?
 - a. **For all exterior glazing, the intent is to follow the physical makeup specifications for IGU-1 (1/4 tempered glazing with SN54 on surface #2, 1/2" black spacer with argon-filled cavity, over 1/4" clear). For all exterior glazing below 8'-4", the design intent is to apply the same level tint as the existing high school building to match the aesthetic and privacy (08 80 00, 2.04.B.2.b. Tint: to match the existing building. Submit samples for review).**
9. Q: S2-1 has details for cold-formed metal framing delegated design, but I'm not seeing any cold-formed metal framing scope.
 - a. **Remove cold-formed metal framing details from S2-1. All wood construction.**
10. Q: C1.0 indicates to remove the abandoned well that we were told not to carry cost to remove/seal during the pre-bid walk. Please confirm.
 - a. **Please see the attached C1-0 sheet. The Contractor is not responsible for removing or sealing the well at this time (not to be included in the bid). The earthwork contractor shall verify that the existing well has been properly sealed and abandoned and report any discrepancies from anticipated conditions to the design team.**
11. Q: Please clarify desired finishes for locker bench/bases. Plan Notes 3 & 5 indicate CMU verticals are to receive EF1 epoxy flake finish, but details on page RD9-1 show EP1 finish. There is no finish designation for Sherwin-Williams 3746. Please confirm this is to be used on only the top and edge of the locker base & benches.
 - a. **Plan Notes 3 & 5 do not refer to CMU verticals. Plan Notes 3 & 5 refer to the concrete bench tops that are to receive EF-1. CMU to receive EP-1 per details.**
12. Q: A9-1 shows a bench on the wall opposite the toilet compartments in both restrooms. Is that intended to stay in that location?
 - a. **No, please remove note 4 from both of those locations.**
13. Q: RA2-0 RCP note 1 only has the words "reflected ceiling plan note" in the description. What is intended to go there?
 - a. **RCP note 1 to read: "PROVIDE LED SURFACE MOUNTED LIGHT FIXTURE IN PLACE OF EXISTING FIXTURES DEMOED. SEE ELECTRICAL." RCP note 2 to read: "PROVIDE LED SURFACE MOUNTED LIGHT FIXTURE IN NEW LOCATION. PROVIDE SURFACE MOUNTED CONDUIT HIGH AND TIGHT TO STRUCTURE TO FIXTURE. SEE ELECTRICAL."**

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Specifications Items:

Section 02 32 00 GEOTECHNICAL INVESTIGATIONS

1. *ADD this section in its entirety.*

Section 10 14 19 DIMENSIONAL LETTER SIGNAGE

1. *ADD this section in its entirety.*

Section 12 66 12 TELESCOPING BLEACHERS

1. *ADD RSL - Kodiak Bleachers to acceptable manufacturers.*
 2. *ADD Interkal LLC – Interkal Bleachers to acceptable manufacturers.*
-

Drawing Items:

Sheet C4-0 SITE UTILITY PLAN

1. *Change note 2 to “2” WATER LINE.”*
2. *Change note 3 to “2” CURB STOP AND BOX.”*

Sheet S1-2 SLAB DETAILS

3. *REMOVE 1/S1-2 TYPICAL FOUNDATION DRAIN detail in its entirety.*

Sheet A8-1 DOOR/FRAME SCHEDULES

1. *REMOVE “SECURITY” FROM IGU-1/WF-1 IN THE GLAZING SCHEDULE.*

Sheet E1-1 ELECTRICAL PLAN

1. *Add a 120-volt 20-amp circuit, power with an exterior junction box for signage. 1 sign on exterior 16’ AFF. Northside directly above where the electrical panels are. Approximate grid between 2, 3 and A. 2nd sign SE corner of gym 20’ AFF. Same power exterior sign. Approximately by grid line 4E.*
2. *Camera locations are shown in the attachment. EC to provide CAT6 cabling to camera locations on the interior and exterior. Each camera location to have a concealed junction box with front flush to the wall with conduit and Cat 6 cable back to the Mechanical Room server location. Final connection by*

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Owner. Cat 6 cable is to have conduit if it passes through a wall or ceiling. All other locations can be exposed, cable-tied up according to detail 10 / E1-2. All conduits passing through the firewall to have fire caulking. Camera locations will be updated on electrical plans in the For Construction Set.

Sheet E2-1 LIGHTING PLANS

Lightsource equal

A1 39 ABL-Lithonia Lighting LDN8 35/150 LO8AR LS

B1 0 ABL-Gotham Lighting IVO6S D 45LM 35K 80CRI WD P AR LSS

B2 26 ABL-Gotham Lighting IVO6S D 20LM 35K 80CRI WD P AR LSS

C1 0 PINNACLE ARCHITECTURAL LIGHTING CCDF N 835X Y3 X U EL 1 1 0 X

*D1 8 ABL-Lithonia Lighting CLX L48 7000LM SEF RDL MVOLT GZ10 35K
80CRI WH*

*D2 3 ABL-Lithonia Lighting CLX L48 5000LM SEF RDL MVOLT GZ10 35K
80CRI WH*

*E1 58 PINNACLE ARCHITECTURAL LIGHTING EV4D HE 835 XXFT XX U FSD 1 0 W needs
1500 lumens/ft*

*FA 0 ABL-Mark Architectural Lighting S4PD LLP 16FT MSL8 80CRI 35K 1000LMF
SCT MIN10 FLL MVOLT WHTT ZT F1/36A*

RDCY WHTCY WCRD

*G1 4 ABL-Gotham Lighting IVO6CYL SC D 30LM 35K 80CRI WD L7 P AR
LSS*

I1 0 ABL-Mark Architectural Lighting S2WD 4FT 80CRI 35K 800LMF STD FLL

J1A 0 ABL-Lithonia Lighting WPX1 LED P2 40K MVOLT DBLXD

J1B 0 ABL-Lithonia Lighting WPX2 LED 40K MVOLT DWHXD

J1C 0 ABL-Lithonia Lighting WPX3 LED 40K MVOLT DWHXD

J2-EM 0 ISOLITE OWL EM X MB 4000K

S1-L 0 ABL-Lithonia Lighting RSX2 LED P2 40K R4

Attachments:

1. *Specification Section 02 32 00 GEOTECHNICAL INVESTIGATIONS (38 pages)*
2. *Specification Section 10 14 19 DIMENSIONAL LETTER SIGNAGE (3 pages)*
3. *Camera location document (5 pages)*
4. *Sheet C4-0 SITE UTILITY PLAN*
5. *Sheet A4-1 EXTERIOR ELEVATIONS*
6. *Sheet A4-2 EXTERIOR ELEVATIONS*
7. *Sheet A10-2 INTERIOR ELEVATIONS*
8. *Sheet A10-3 INTERIOR ELEVATIONS*

END

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**SECTION 023200
GEOTECHNICAL INVESTIGATIONS**

PART 1 GENERAL

1.01 SECTION INCLUDES

- A. Geotechnical investigations were conducted at the site, the results of which can be found in the report issued by Patriot Engineering and Environmental, Inc., Report 25-1836-04G, dated January 14, 2026.
 - 1. A copy of the report is included at the end of this specification section.

1.02 INTERPRETATION

- A. The report is provided only for bidder's information and convenience and is not part of the Contract Documents. Owner and Architect do not warrant the accuracy or extent of the report or locations of the test borings.
- B. Opinions expressed in the report are those of the Geotechnical Engineer and represent the Geotechnical Engineer's interpretation of subsoil conditions, tests, and results of analyses that the Geotechnical Engineer has conducted.
- C. The report is based upon the assumption that uniform variation exists in soil properties between borings. Interpretation of the report is bidder's responsibility. Owner and Architect will not be responsible for interpretation of report by bidders.
- D. Bidders are urged to examine the report and the site.
- E. Additional soil borings or other exploratory operations may be made by bidders at no additional cost to Owner, provided such operations are approved by Owner in advance.
- F. Refer to Conditions of the Contract for additional information.

END OF SECTION

Report of Geotechnical Engineering Exploration

**SWCS RAIDERS ACTIVITY CENTER
PONETO, INDIANA
25-1836-04G**

**Prepared For:
Andrew Jackson, AIA
krM Architecture+
1515 North Pennsylvania Street
Indianapolis, Indiana 46202**

**Patriot Engineering and Environmental, Inc.
1000-C Airport North Office Park
Fort Wayne, Indiana 46825**

January 14, 2026



January 14, 2026

Mr. Andrew Jackson, AIA
krM Architecture+
1515 North Pennsylvania Street
Indianapolis, Indiana 46202

Re: Report of Geotechnical Engineering Exploration
SWCS Raiders Activity Center
9120 South 300 West
Poneto, Indiana
Patriot Project No. 25-1836-04G

Dear Andrew:

Attached is the report of our subsurface exploration for the above referenced project. This exploration was completed in general accordance with our Proposal No. P25-2359-04G revision dated November 3, 2025.

We appreciate the opportunity to perform this geotechnical engineering exploration and are looking forward to working with you during the construction phase of the project. If you have questions regarding this report or if we may be of additional assistance regarding any geotechnical aspect of the project, please do not hesitate to contact our office.

Respectfully submitted,
Patriot Engineering and Environmental, Inc.



Akshat Saxena, PE
Project Engineer



William D. Dubois, PE
Senior Principal Engineer



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APPENDICES

Appendix A:	Site Vicinity Map (Figure 1) Soil Boring & Test Pit Location Map (Figure 2) Test Pit Log Boring Logs Boring Log Key Unified Soil Classification System (USCS)
Appendix B:	General Qualifications Standard Clause for Unanticipated Subsurface Condition

REPORT OF GEOTECHNICAL ENGINEERING EXPLORATION

SWCS RAIDERS ACTIVITY CENTER

PONETO, INDIANA

25-1836-04G

1.0 INTRODUCTION

1.1 General

krM Architecture+ is planning the construction of an activity center building within the school complex located at 9120 South 300 West in Poneto, Indiana. The results of our geotechnical engineering exploration for the project are presented in this report.

1.2 Purpose and Scope

The purpose of this exploration is to determine the general near surface and subsurface conditions within the project area and to develop the geotechnical engineering recommendations necessary for the design and construction of the proposed center. This was achieved by drilling test borings, and by conducting laboratory tests on samples taken from the borings. This report contains the results of our findings, an engineering interpretation of these results with respect to the available project information, and recommendations to aid in the design and construction of the proposed structure.

2.0 PROJECT INFORMATION

The project involves the construction of an 18,000 square-foot (sqft) activity center building for Southern Wells Community Schools (SWCS) located at the above-mentioned address. The new structure will be in the northeast portion of the school complex and will house gymnasium, weight room, wrestling rooms, etc. It is anticipated that the structure will be high single-storied with slab-on-grade construction.

Site grading and structural loading were not provided. We anticipate that loads will be less than 150 kips for columns, less than 3 kips per lineal foot (klf) for walls, and less than 150 pounds per square foot (psf) for floors. We have assumed that the cut and fill depths will be minimal (not exceeding 2 feet).

The above narrative is based upon information provided to *Patriot*. This represents the most current information available at the time of issuance of this report. Our assumptions and the recommendations set forth in this report are therefore directly related to this information. *Patriot* should be notified immediately if the provided information provided to us changes during the design development. We cannot be responsible if changes are made to the project, and we are not allowed to determine if our recommendations remain valid.

3.0 EXPLORATIONAL PROCEDURES

3.1 Field Work – Soil Borings

A total of eight (8) soil borings were drilled, sampled, and tested at the project site on December 5, 2025, at the approximate locations shown on the Soil Boring & Test Pit Location Map in Appendix A. All depths are given as feet below the existing ground surface. Boring elevations were obtained from Google Earth imagery.

The borings were advanced using 3¼" I.D. (inside diameter) hollow-stem augers. Samples were recovered in the undisturbed material below the bottom of the augers using the standard drive sample technique in accordance with ASTM D 1586-74. A 2" O.D. (outside diameter) by 1³/₈" I.D. split-spoon sampler was driven a total of 18 inches with the number of blows of a 140-pound hammer falling 30 inches recorded for each 6 inches of penetration. The sum of blows for the final 12 inches of penetration is the Standard Penetration Test result commonly referred to as the N-value (or blow-count). Where the Split-spoon sampler is advanced less than 6 inches in 50 blows, it is referred as refusal and indicated as: 50 / (number of inches advanced per 6-inch interval). Split-spoon samples were recovered at 2.5-foot intervals, beginning at a depth of 1 foot below the existing surface grade, extending to a depth of 10 feet, and at 5-foot intervals thereafter to the termination of the boring. Water levels were monitored at each borehole location during drilling and upon completion of the boring. The boreholes were backfilled with auger cuttings and patched on surface with asphalt/concrete (as appropriate) prior to demobilization.

Upon completion of the boring program, the samples retrieved during drilling were returned to *Patriot's* soil testing laboratory where they were visually examined and classified. A laboratory-generated log of each boring was prepared based upon the driller's field log, laboratory test results, and our visual examination. Test boring logs and a description of the classification system are included in Appendix A in this report. Indicated on each log are the primary strata encountered, the depth of each stratum change, the depth of each sample, the Standard Penetration Test results, groundwater conditions, and selected laboratory test data. The laboratory logs were prepared for each boring giving the appropriate sample data and the textural description and classification.

3.2 Field Work – Test Pit

A test pit was also performed on site on November 25, 2025; the approximate location is shown on the Soil Boring & Test Pit Location Map in Appendix A. The test pit was performed using a John Deere 350G excavator. After completion, the excavated soils were backfilled in the test pit . The test pit log is included in Appendix A in the report.

3.3 Laboratory Testing

Representative samples recovered in the borings were selected for testing in the laboratory to evaluate their physical properties and engineering characteristics. Laboratory analyses included natural moisture content determinations (ASTM D 2216), Atterberg limits testing (ASTM D 4318), and an estimate of the cohesive soil strength was determined by utilizing a hand penetrometer (qp). The results of all laboratory tests are summarized in Section 4.2 and are shown on the boring logs and laboratory data sheets as appropriate.

4.0 SITE AND SUBSURFACE CONDITIONS

4.1 Site Conditions

The project site is an existing school with buildings, parking lots, and other associated structures, generally surrounded by farmland. The ground cover within the project footprint is predominantly topsoil. The site gently slopes up from north to south and standing water was not observed onsite in the vicinity of the borings.

The *Client* has stated that within the footprint of the proposed activity center there used to be an older structure with a basement; this structure is not visible in historical aerial imagery dating up to 1998. The *Client* has further mentioned that the basement was backfilled when the structure was demolished. Other details about this older structure or backfill records were not available.

4.2 Subsurface Conditions

Our interpretation of the subsurface conditions is based upon widely spaced soil borings drilled at the approximate locations shown on the Boring Location Map in Appendix A. The following discussion is general; for more specific information, please refer to the boring logs presented in Appendix A. The dashed stratification lines shown on the soil boring logs indicate approximate transitions between soil types. In situ stratification changes could occur gradually or at different depths. All depths discussed below refer to depths below the existing ground surface.

Topsoil – Topsoil, a surficial layer of material that is a blend of silts, sands, and clays, with varying amounts of organic matter, was encountered in every boring at ground surface and the thickness varied 10 to 12 inches.

Existing Undocumented Fill Soils – Existing undocumented fill soils were encountered in the test pit (TP-1) and boring B-2 to a depth of 10 and 2.7 feet, respectively. The fill soils were classified as medium stiff sandy and silty clays. The extent of the fill soil is unknown. It should be noted that due to excavator limitations the test pit was terminated at 10 feet; additionally, the boring was terminated at 2.7 feet due to refusal. Consequently, the fill soils may extend to deeper depths.

Native Cohesive Soils (CL) – Native cohesive soils were classified as medium stiff to very stiff silty and sandy clays. The native clayey materials have moisture contents

ranging from 13 to 32 %. Standard Penetration Test N-values in clayey soils varied between 7 and 21 blows per foot (bpf).

4.3 Groundwater Conditions

Groundwater was encountered in B-4 during drilling at 15 feet. Groundwater was not observed in the remaining borings during drilling and upon completion above the cave-in depth when augers were removed from the borehole.

The term groundwater pertains to any water that percolates through the soil found on site. This includes any overland flow that permeates through a given depth of soil, perched water, and water that occurs below the “water table”, a zone that remains saturated and water-bearing year-round.

It should be recognized that fluctuations in the groundwater level should be expected over time due to variations in rainfall and other environmental or physical factors. ***The true static groundwater level can only be determined through observations made in cased holes over a long period of time, the installation of which was beyond the scope of this exploration.***

5.0 DESIGN RECOMMENDATIONS

5.1 Basis

Our recommendations are based on data presented in this report, which include soil borings, laboratory testing and our experience with similar projects. Subsurface variations that may not be indicated by a dispersive exploratory boring program can exist on any site. If such variations or unexpected conditions are encountered during construction, or if the project information is incorrect or changed, we should be informed immediately since the validity of our recommendations may be affected.

5.2 Foundations

We recommend that the proposed structures be supported on shallow foundations (continuous wall footings and isolated column pads), bearing in at least medium stiff native cohesive soils or new compacted and tested structural fill. **Existing undocumented fill soils are unsuitable for foundation support.** Wherever encountered at the proposed footing subgrade level, the fill soils should be completely removed and replaced with compacted and tested structural fill. The structural fill should meet the requirements of this report and must be a suitable structural fill material with a Liquid Limit less than 40 and a Plasticity Index between 10 and 20. Alternatively, the footings can trench through these fill soils so that the foundation subgrade consists of stiff/dense non-expansive native soils. In this case, the overexcavation can be backfilled with new structural fill soils or lean concrete that has a compressive strength of at least 500 psi.

Provided the above recommendations are followed, the footings may be designed for a maximum net allowable bearing pressure of 2,500 pounds per square foot (psf), full dead and full live load for isolated footings and 2,000 psf for continuous footings. Furthermore, we recommend that each footing subgrade be visually inspected by a *Patriot* representative following the excavation of the footing area. In addition, each foundation area should be probed using a dynamic cone penetrometer (DCP) to a depth of 5 feet below the footing subgrade. This is to ensure that the underlying soil has an SPT blow count of 7 or more. Inspection and probing should be performed at multiple locations within each isolated footing subgrade and at every 10 feet along strip footing subgrades.

All exterior foundations and foundations in unheated areas should be located at a depth of at least 36 inches below final exterior grade for frost protection. Additionally, interior foundations in heated areas can bear at depths of approximately 24 inches below the finished floor. We recommend that wall (strip) footings be at least 18 inches wide and column footings be at least 24 inches wide for bearing capacity considerations.

Care must be taken during the foundation excavation to not disturb the foundation soils and create an unstable condition. It is strongly recommended that *Patriot* be retained to review the soil conditions at the founding level to verify that the soils are similar to those encountered in the soil test borings and the founding material is stable and capable of supporting the intended loading.

For proper performance at the recommended bearing pressure, foundations must be constructed in compliance with the recommendations for footing excavation observation that are discussed in the Construction Considerations section of this report. It may be necessary to undercut the excavation at isolated locations to accommodate the design bearing capacity. Careful field control during construction by *Patriot* will be necessary to confirm that the exposed material is capable of supporting the design bearing pressure and minimize the post construction settlement potential.

In using the above net allowable soil bearing pressures, the weight of the foundation and backfill over the foundation need not be considered. Hence, only loads applied at or above the minimum finished grade adjacent to the footing need to be used for dimensioning the foundations. Each new foundation should be positioned so it does not induce significant pressure on adjacent foundations; otherwise, the stress overlap must be considered in the design.

We estimate that the total foundation settlement should not exceed approximately 1 inch and that differential settlement should not exceed about $\frac{3}{4}$ inch when constructed in accordance with recommendations in this report. Careful field control during construction is necessary to minimize the actual settlement that will occur.

Positive drainage of surface water, including downspout discharge, should be maintained away from structure foundations to avoid wetting and weakening of the foundation soils both during construction and after construction is complete. Water and drainage lines should be located such that if leakage occurs, water will not be readily accessible to foundations, floor slabs, and/or pavement sections. Additionally, the installation and use of an irrigation system at the parcel is discouraged.

5.3 Test Pit

As mentioned earlier, excavated soils from the test pit were backfilled within the test pit upon completion. Although, the soils were tamped down with excavator bucket, adequate compaction would not be achieved. We recommend that these backfilled soils (within the general area of the test pit) be re-excavated and replaced with new structural fill. Alternatively, the overexcavation can be backfilled with lean concrete that has a compressive strength of at least 500 psi.

5.4 Floor Slabs

In general, shallow native soils onsite are suitable for floor slab support. **Existing undocumented fills are unsuitable for floor slab support. Wherever encountered at the proposed slab subgrade level, the fill soils should be completely removed and replaced with new compacted and tested structural fill.**

We recommend that all floor slabs be designed as “floating”, that is, fully ground supported and not structurally connected to walls or foundations. This is to minimize the possibility of cracking and displacement of the floor slabs because of differential movements between the slab and the foundation. Although the movements are estimated to be within the tolerable limits for structural safety, such movements could be detrimental to the slabs if they were rigidly connected to the foundations. Additionally, we recommend that all slabs should be liberally jointed and designed with the appropriate reinforcement for the anticipated loading conditions.

The building floor slabs should be supported on a minimum 6-inch-thick well-compacted granular base course (well-graded crushed stone) bearing on a suitably prepared subgrade (Refer to Section 6.0 “Construction Considerations”). The granular base course is expected to help distribute loads and equalize moisture conditions beneath the slab.

Provided that the recommendations above for floor slab design and construction are followed, a modulus of subgrade reaction, “K₃₀” value of 100 pounds per cubic inch (pci), is recommended for the design of ground supported floor slabs. It should be noted that the “K₃₀” modulus is based on a 30-inch diameter plate load empirical relationship.

5.5 Pavements

The near surface soils encountered onsite during our exploration are generally suitable for pavement support. If existing undocumented fill soils are encountered, we recommend that they be over-excavated a minimum depth of 24 inches below the pavement bearing elevation and replaced with an equal amount of approved compacted structural fill. If softer soils are encountered at the pavement subgrade level, these soils should be undercut and replaced with new structural fill placed, compacted, and tested in lifts to re-develop the subgrade elevation.

If construction is performed during a wet or cold period, the contractor will need to exercise care during the grading and fill placement activities to achieve the necessary subgrade soil support for the pavement system (See Section 6.0 for “Construction Considerations”). The base soil for the pavement section will need to be firm and dry. The subgrade should be sloped properly to provide good base drainage. To minimize the effects of groundwater or surface water conditions, the base section for the roadway should be sufficiently high above adjacent ditches and properly graded to provide pavement surface and pavement base drainage.

Based upon the near surface soil encountered in the borings, we recommend using a California Bearing Ratio (CBR) value of 3 for pavement design purposes. It should be recognized though, that the recommended CBR value is based on empirical relationships only, and laboratory CBR tests may determine a higher allowable CBR value.

Our recommendations assume that the paved areas will be constructed on proofrolled natural soil, or on structural fill overlying the same. Serviceable pavements can be achieved by different combinations of materials and thicknesses, varied to provide roughly equivalent strengths.

5.6 Seismic Considerations

For structural design purposes, we recommend using a **Site Classification of “C”** as defined by current Indiana Building Code (modified International Building Code). Along with using a Site Classification of C, we recommend the use of the maximum considered spectral response acceleration and design spectral response acceleration coefficients provided in Table 4 below.

Table 1. Seismic Design Spectral Response Acceleration Coefficients

Period (seconds)	Maximum Considered Spectral Response Acceleration Coefficient	Soil Factor	Design Spectral Response Acceleration Coefficient
0.2	$S_s = 0.180 \text{ g}$	1.2	$S_{Ds} = 0.120 \text{ g}$
1.0	$S_1 = 0.077 \text{ g}$	1.7	$S_{D1} = 0.071 \text{ g}$

These values were obtained from the “ASCE Hazard Tool” program for seismic design, developed by the American Society of Civil Engineers (ASCE), utilizing the earlier mentioned site address as the designation for identifying the location of the parcel. Other earthquake resistant design parameters should be applied consistent with the minimum requirements of the current Indiana Building Code.

5.7 Subsurface Utilities

For installation of subsurface utilities (i.e., water lines, storm-sewer lines, sanitary-sewer lines, manholes, culverts, etc.) the soil conditions encountered in our borings should be readily excavated using conventional earthwork equipment.

Depending on seasonal conditions and the invert elevations of the proposed subsurface utilities, localized and sporadic groundwater infiltration should be expected to be encountered in the subsurface utility excavations (Refer to Section 6.4 “Groundwater Considerations”).

In regard to bearing and support of the subsurface utilities, the soil conditions encountered in our borings, if properly prepared, are suitable for support of the proposed subsurface utilities.

6.0 CONSTRUCTION CONSIDERATIONS

6.1 Site Preparation

All areas that will support foundations, floors, pavements, or newly placed structural fill must be properly prepared. All loose surficial soil or “topsoil” and other unsuitable materials must be removed. Unsuitable materials include frozen soil, relatively soft material, relatively wet soils, deleterious material, or soils that exhibit a high organic content.

About 10 to 12 inches of loose surficial topsoil was encountered in the borings. The topsoil was measured at discrete locations as shown on the Boring Location Map (Appendix A). The topsoil thickness measured at the boring locations may or may not be representative of the overall average topsoil thickness at the site. Therefore, it is possible that the actual stripping depth will significantly vary from this data. The data presented should be viewed only as a guide to the minimum stripping depth that will be required to remove organic material at the surface.

After stripping or in cut areas where floor slabs or new structural fill is to be placed, the exposed subgrade must be evaluated by a Patriot representative. The upper 12 inches of subgrade should be scarified and compacted to a dry density of at least 100 percent of the Standard Proctor maximum dry density (ASTM D-698); then proofrolling of the subgrade should be performed.

Proofrolling should consist of repeated passes of a loaded, pneumatic-tired vehicle such as a tandem-axle dump-truck or scraper. The proofrolling operations should be observed by a *Patriot* representative, and the proofrolling vehicle should be loaded as directed by *Patriot*. Any area found to rut, pump, or deflect excessively should be compacted in-place or, if necessary, undercut and replaced with structural fill, compacted as specified below.

Care must be exercised during grading and fill placement operations. ***The combination of heavy construction equipment traffic and excess surface moisture can cause pumping and deterioration of the near surface soils. The severity of this potential problem depends to a great extent on the weather conditions prevailing during construction.*** The contractor must exercise discretion when selecting equipment sizes and make a concerted effort to control construction traffic and surface water while the subgrade soils are exposed. We recommend that heavy construction equipment (i.e., dump trucks, scrapers, etc.) be rerouted away from the building and pavement areas. If such problems do arise, the operations in the affected area should be halted and the *Patriot* representative contacted to evaluate the condition.

6.2 Foundation Excavations

Upon completion of the foundation excavations and prior to the placement of reinforcing steel, a *Patriot* representative should observe the exposed subgrade to confirm that a bearing surface of adequate strength has been achieved. Any localized soft soil zones encountered at the bearing elevations should be further excavated until adequate support soils are encountered. The cavity should be backfilled with structural fill as defined below, or the footing can be poured at the excavated depth. Structural fill used as backfill beneath footings should be limited to lean concrete, well-graded sand and gravel, or crushed stone placed and compacted in accordance with Section 6.4.

If it is necessary to support spread footings on structural fill, the fill pad must extend laterally a minimum distance beyond the edge of the footing. The minimum structural pad width would correspond with a point at which an imaginary line extending downward from the outside edge of the footing at a 1H:2V slope intersects the surface of the natural soils. For example, if the depth to the bottom of excavation is 4 feet below the bottom of the foundation, the excavation would need to extend laterally beyond the edge of the footing at least 2 feet, as shown in Illustration A, found at the conclusion of this report.

Excavation slopes should be maintained within OSHA requirements. We recommend that any surcharge fill or heavy equipment be kept at least 5 feet away from the edge of the excavation. In addition, excavations that occur near existing in-use foundations should be carefully performed making a conscious effort not to undermine the support of the in-use foundations. If it is necessary to excavate soil adjacent to and below the bearing elevation of any in-use foundations, *Patriot* should be contacted to make further recommendations regarding these excavations. Please refer to Illustration B at the end of this report for further details.

Construction traffic on the exposed surface of the bearing soil will potentially cause some disturbance of the subgrade and consequently loss of bearing capacity. However, the degree of disturbance can be minimized by proper protection of the exposed surface.

6.3 Structural Fill and Fill Placement Control

Structural fill, defined as any fill which will support structural loads, should be clean and free of organic material, debris, deleterious materials, and frozen soils. Samples of the proposed fill materials should be tested prior to initiating the earthwork and backfilling operations to determine the classification, the natural and optimum moisture contents and maximum dry density and overall suitability as a structural fill. **Structural fill should have a Liquid Limit (LL) less than 40 and a Plasticity Index (PI) between 10 and 20.**

6.3.1 Existing Site Materials

Regarding the suitability of the on-site soils for use as structural fill, the upper few feet of these soils are expected to be wetter than the estimated optimum moisture contents, depending on the prevailing weather conditions at the time of construction. Therefore, scarification and/or drying may be required to reduce the moisture content of the soils to achieve adequate compaction of the clays and proper strength.

6.3.2 Fill Placement Control

All structural fill beneath floor slabs, adjacent to foundations and over foundations, should be compacted to at least 98 percent of its maximum Standard Proctor dry density (ASTM D-698). This minimum compaction requirement should be increased to 100 percent of the maximum Standard Proctor dry density for fill supporting footings, provided these are designed as outlined in Recommendations, Section 5.0.

Structural fill supporting, around and over utilities should be compacted to at least 95 % of its maximum Standard Proctor dry density (ASTM D-698) for utilities underlying structural areas (i.e., buildings, pavements, sidewalks, etc.). However, the minimum compaction requirement can be reduced for backfill around and over the utilities to 90 % of the maximum Standard Proctor dry density where utilities underlie greenbelt areas (i.e., grassy lawns, landscaping, etc.). It is recommended that a clean well-grade granular material be utilized as the bedding material, as well as the backfill material around and over the utility lines.

To achieve the recommended compaction of the structural fill, we suggest that the fill be placed and compacted in layers not exceeding 8 inches in loose thickness (the loose lift thickness should be reduced to 6 inches when utilizing small hand compactors) and within the range of 2 percentage (%) points below or above the optimum moisture content value. All fill placement should be monitored by a *Patriot* representative. **Each lift should be tested for proper compaction at a frequency of at least one (1) test every 2,500 square feet (ft²) per lift for the building areas, and at a frequency of at least one (1) test for every 50 lineal feet of utility installation.**

6.4 Groundwater Considerations

Groundwater was encountered in B-4 during drilling at 15 feet. Groundwater was not observed in the remaining borings during drilling and upon completion. Localized and sporadic groundwater infiltration may occur into the building foundation excavations on this site, depending on seasonal conditions.

Groundwater inflow into shallow excavations **above** the groundwater table is expected to be adequately controlled by conventional methods such as gravity drainage and/or pumping from sumps. More significant inflow can be expected in deeper excavations **below** the groundwater table requiring more aggressive dewatering techniques, such as well or wellpoint systems. For groundwater to have minimal effects on the construction, foundation excavations should be constructed and poured in the same day.

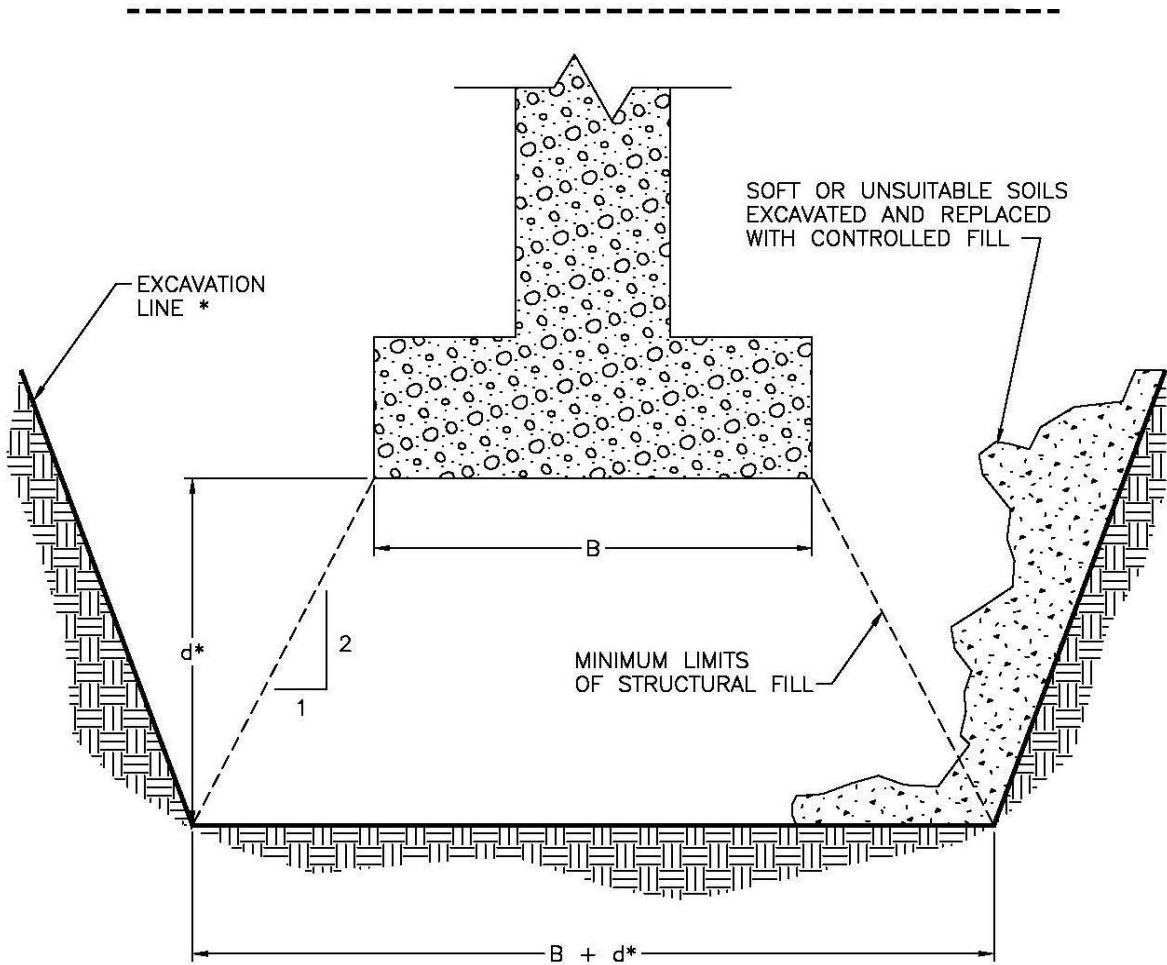
6.5 Drainage Control

We recommend that positive drainage be established around the proposed structure to promote the quick drainage of surface water away from the structures and to prevent the ponding of water adjacent to the structure. Finish grading in grass and landscaped areas should be sloped down and away from the structures at 10 percent for at least 10 feet, and then at a gradient of at least 2 percent beyond the initial 10 feet from the structures. **The final grades should direct the surface water to storm water collection systems.**

7.0 ILLUSTRATIONS

See Illustrations A and B on the following pages. These illustrations are presented to further visually clarify several of the construction considerations presented in Section 6.2.

FUTURE GRADE



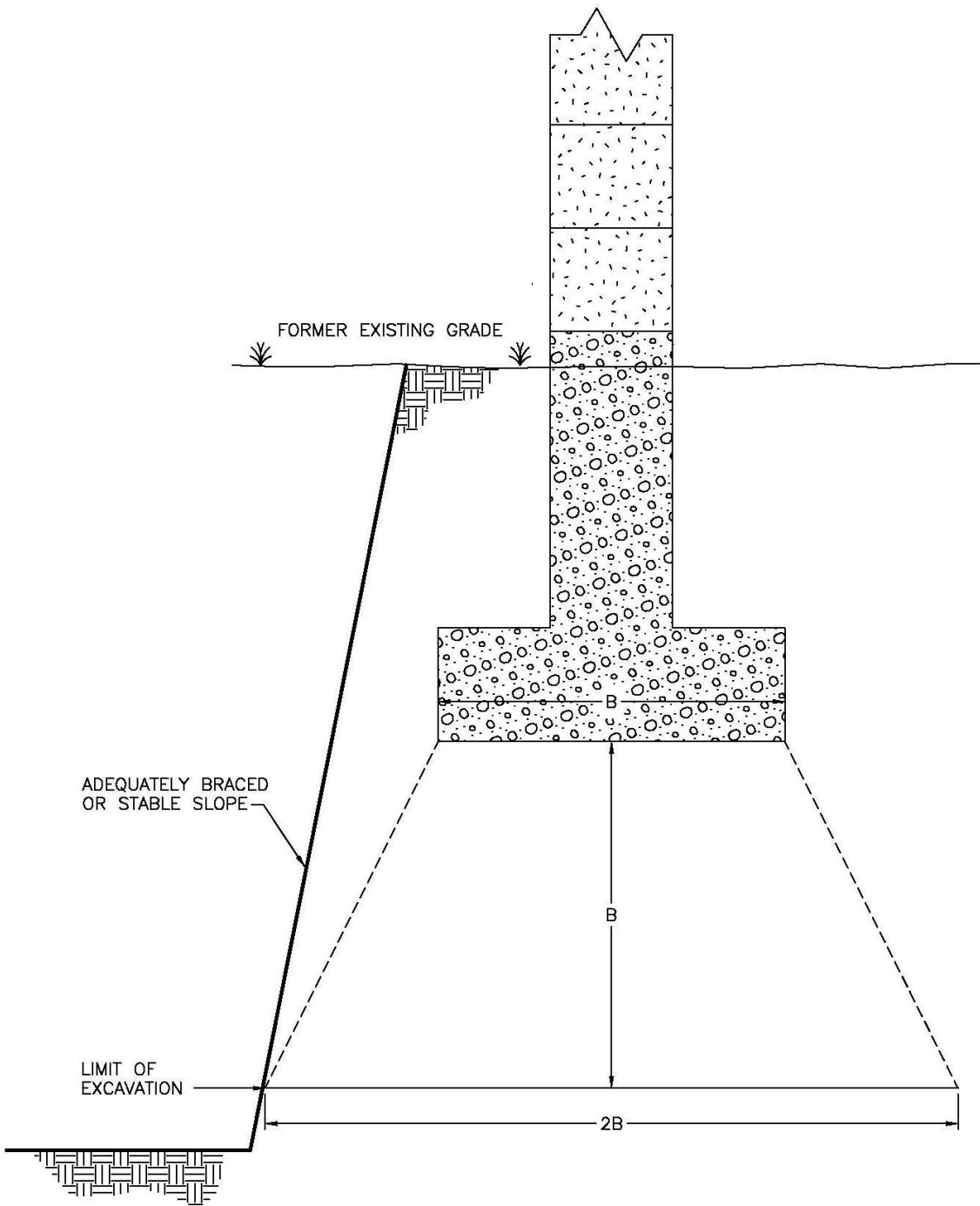
*d IS DEPTH TO SUITABLE SOILS

* IN COMPLIANCE WITH OSHA STANDARDS



**PATRIOT ENGINEERING
and Environmental, Inc.**
2568 Kohnle Drive
Miamisburg, Ohio 45342
(937) 847-9707 Fax: (937) 847-9720

**Excavation for Footings
In an Area of Fill
ILLUSTRATION A**



**PATRIOT ENGINEERING
and Environmental, Inc.**

2568 Kohnle Drive
Miamisburg, Ohio 45342
(937) 847-9707 Fax: (937) 847-9720

**Excavation Near Existing
In Use Foundation
ILLUSTRATION B**

APPENDIX A

Site Vicinity Map (Figure 1)

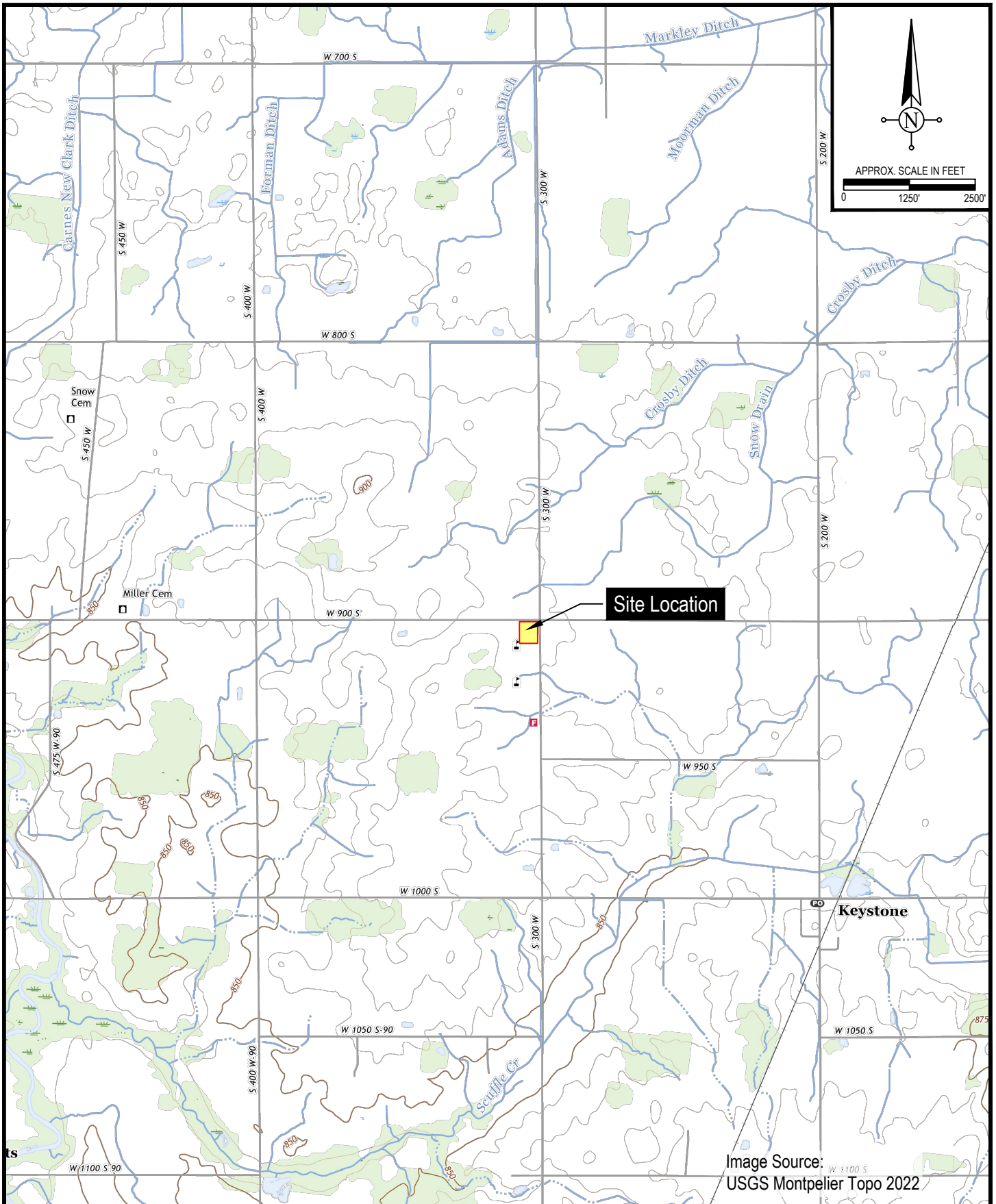
Soil Boring & Test Pit Location Map (Figure 2)

Boring Logs

Test Pit Log

Boring Log Key

Unified Soil Classification System (USCS)

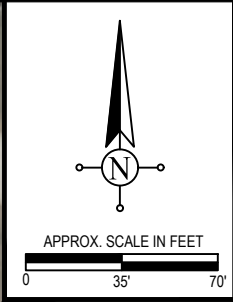


Project: SWCS Raiders Activity Center
 9120 South 300 West
 Poneto, Indiana

Project Number: 25-1836-04	Drawn By: T. Humphreys
Date: December 19, 2025	Approved: A. Saxena
	DWG: 25-1836-04_geo

Figure 1

Site Vicinity Map



LEGEND

	PATRIOT Soil Boring
	PATRIOT Test Pit
B-1	Soil Boring/Test Pit ID



- NOTES:**
1. Boring locations were staked by PATRIOT. All locations are shown as approximate.
 2. All locations were determined in the field with references to existing landmarks.
 3. Image Source: Bing Maps
 4. Scale as shown.

Project: SWCS Raiders Activity Center 9120 South 300 West Poneto, Indiana	
Project Number: 25-1836-04	Drawn By: T. Humphreys
Date: December 19, 2025	Approved: A. Saxena
	DWG: 25-1836-04_geo

Figure 2
Soil Boring & Test Pit Location Map



LOG OF BORING B-1

SWCS Raiders Activity Center
9120 South 300 West, Poneto, IN

Client Name : krM Architecture+
Project Number : 25-1836-04G
Logged By : C. Moreno
Start Date : 12/05/2025
Drilling Method : HSA

Driller : B. Roberts
Sampling : Splitspoon
Approx. Elevation : +/- 878 feet
Latitude :
Longitude :

Depth (Feet)	Elevation (Feet)	Water Level	USCS	GRAPHIC	Water Levels					REMARKS	
					▼ During Drilling - Dry	▽ After Completion - Dry	◆ After 24 Hours - N/A	Samples	Rec %		SPT Results
DESCRIPTION											
0	878				TOPSOIL (10")						
			CL		Brown and gray, moist to very moist, medium stiff, SILTY CLAY with trace to little sand	1	72	3/4/4	1.1	32	
5						2	67	2/3/5	2.8	23	
			CL		Brown, slightly moist to moist, stiff, SANDY CLAY with trace gravel	3	67	4/4/5	5.5	16	
10						4	67	4/4/5	>6.0	15	
			CL		Gray, moist, medium stiff to stiff, SANDY CLAY with trace gravel	5	72	3/3/5	2.0	16	
15						6	72	4/4/6	1.7	15	
20					Boring terminated at 20 feet.					Boring caved to 16 feet upon auger removal.	
25										Groundwater was not encountered during, nor upon completion.	
30											



LOG OF BORING B-2

SWCS Raiders Activity Center
9120 South 300 West, Poneto, IN

Client Name : krM Architecture+
Project Number : 25-1836-04G
Logged By : C. Moreno
Start Date : 12/05/2025
Drilling Method : HSA

Driller : B. Roberts
Sampling : Splitspoon
Approx. Elevation : +/- 877 feet
Latitude :
Longitude :

Depth (Feet)	Elevation (Feet)	Water Level	USCS	GRAPHIC	Water Levels					REMARKS					
					▼ During Drilling - Dry	▽ After Completion - Dry	◆ After 24 Hours - N/A	Samples	Rec %		SPT Results	qp tsf	w %		
DESCRIPTION															
0					TOPSOIL (10")										
875	877		CL		Brown, moist, medium stiff, SANDY CLAY with trace gravel (FILL)					1	67	4/4/4	5.5	14	Boring caved to 2.5 feet upon auger removal. Auger refusal encountered at 2.5 feet. Groundwater was not encountered during, nor upon completion.
					Splitspoon refusal encountered at 2.67 feet.					2	11	50-2"	11		
5															
870															
10															
865															
15															
860															
20															
855															
25															
850															
30															



LOG OF BORING B-3

SWCS Raiders Activity Center
9120 South 300 West, Poneto, IN

Client Name : krM Architecture+
Project Number : 25-1836-04G
Logged By : C. Moreno
Start Date : 12/05/2025
Drilling Method : HSA

Driller : B. Roberts
Sampling : Splitspoon
Approx. Elevation : +/- 877 feet
Latitude :
Longitude :

Depth (Feet)	Elevation (Feet)	Water Level	USCS	GRAPHIC	Water Levels					REMARKS		
					▼ During Drilling - Dry	▽ After Completion - Dry	◆ After 24 Hours - N/A	Samples	Rec %		SPT Results	qp tsf
DESCRIPTION												
0	877				TOPSOIL (10")							
875			CL		Brown, moist, medium stiff, SANDY CLAY with trace gravel	1	67	2/4/4	4.3	23		
5			CL		Brown and gray, moist, stiff, SANDY CLAY with trace gravel	2	67	2/3/7	>6.0	17		
870			CL		Brown, very moist, stiff, SILTY CLAY with trace sand and trace gravel	3	28	5/7/8	1.4	20		
10			CL		Brown, slightly moist, very stiff, SANDY CLAY with trace gravel	4	72	3/7/10	>6.0	13		
865												
15			CL		Gray, slightly moist, stiff, SANDY CLAY with trace to little gravel	5	72	3/4/5	2.5	15		
860											Boring caved to 16 feet upon auger removal.	
20						6	72	2/4/5	1.6	15		
855			Boring terminated at 20 feet.					Groundwater was not encountered during, nor upon completion.				
25												
850												
30												



SWCS Raiders Activity Center
9120 South 300 West, Poneto, IN

Client Name : krM Architecture+
Project Number : 25-1836-04G
Logged By : C. Moreno
Start Date : 12/05/2025
Drilling Method : HSA

Driller : B. Roberts
Sampling : Splitspoon
Approx. Elevation : +/- 876 feet
Latitude :
Longitude :

Depth (Feet)	Elevation (Feet)	Water Level	USCS	GRAPHIC	DESCRIPTION	Samples	Rec %	SPT Results	qp tsf	w %	REMARKS
	876										
					Water Levels ▼ During Drilling - 15.0 feet ▽ After Completion - Dry ◆ After 24 Hours - N/A						
0					TOPSOIL (10")						
0	875		CL		Brown and gray, moist, stiff, SANDY CLAY with trace gravel	1	56	3/6/6	>6.0	16	
5			CL		Gray and brown, moist, medium stiff, SILTY CLAY with trace to little sand and trace gravel	2	56	3/4/4	1.3	24	
5	870		CL		Gray and brown, moist, medium stiff, SILTY CLAY with trace to little sand and trace gravel	3	44	2/3/5	3.5	16	
10			CL		Gray and brown, slightly moist, very stiff, SANDY CLAY with trace gravel	4	83	4/8/13	>6.0	13	
10	865		CL		Gray and brown, slightly moist, very stiff, SANDY CLAY with trace gravel	4	83	4/8/13	>6.0	13	
15		▼	CL		Gray, moist, stiff, SANDY CLAY with trace gravel	5	83	2/4/6	2.2	16	
15	860		CL		Gray, moist, stiff, SANDY CLAY with trace gravel	5	83	2/4/6	2.2	16	Boring caved to 16 feet upon auger removal.
20			CL		Gray, moist, very stiff, SILTY CLAY with some gravel and sand	6	22	4/7/12	0.2	20	
20	855				Boring terminated at 20 feet.						
25											
25	850										
30											



LOG OF BORING B-5

SWCS Raiders Activity Center
9120 South 300 West, Poneto, IN

Client Name : krM Architecture+
Project Number : 25-1836-04G
Logged By : C. Moreno
Start Date : 12/05/2025
Drilling Method : HSA

Driller : B. Roberts
Sampling : Splitspoon
Approx. Elevation : +/- 878 feet
Latitude :
Longitude :

Depth (Feet)	Elevation (Feet)	Water Level	USCS	GRAPHIC	Water Levels					REMARKS
					▼ During Drilling - Dry	▽ After Completion - Dry	◆ After 24 Hours - N/A	Samples	Rec %	
DESCRIPTION										
0	878				TOPSOIL (10")					
					Brown and gray, slightly moist to moist, stiff to very stiff, SANDY CLAY with trace to little gravel					
875					1	72	3/4/5	5.1	21	
5					2	67	3/6/9	>6.0	15	
			CL		3	67	5/7/8	5.6	15	
870					4	67	5/8/9	>6.0	15	
10										
					Gray, slightly moist to moist, stiff, SANDY CLAY with trace gravel					
865					5	72	2/4/5	3.8	13	
15			CL							
860					6	56	1/5/4	1.3	16	
20					Boring terminated at 20 feet.					
					Groundwater was not encountered during, nor upon completion.					
855										
25										
850										
30										

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LOG OF BORING P-1

SWCS Raiders Activity Center
9120 South 300 West, Poneto, IN

Client Name : krM Architecture+
Project Number : 25-1836-04G
Logged By : C. Moreno
Start Date : 12/05/2025
Drilling Method : HSA

Driller : B. Roberts
Sampling : Splitspoon
Approx. Elevation : +/- 874 feet
Latitude :
Longitude :

Depth (Feet)	Elevation (Feet) 874	Water Level	USCS	GRAPHIC	Water Levels					REMARKS		
					▼ During Drilling - Dry	▽ After Completion - Dry	◆ After 24 Hours - N/A	Samples	Rec %		SPT Results	qp tsf
DESCRIPTION												
0					TOPSOIL (10")							
			CL		Brown and gray, moist, stiff, SILTY CLAY with trace sand and trace gravel	1	56	3/4/7	4.1	21	Boring caved to 7 feet upon auger removal.	
870			CL		Brown and gray, moist, medium stiff to stiff, SANDY CLAY with trace gravel	2	56	3/3/5	4.6	23		
5			CL			3	67	2/3/7	5.2	16		
865			CL		Gray, very moist, very stiff, SILTY CLAY with trace gravel and some sand	4	67	10/14/7	3.0	30		
10			Boring terminated at 10 feet.									Groundwater was not encountered during, nor upon completion.
860												
15												
855												
20												
850												
25												
845												
30												




LOG OF BORING P-2

SWCS Raiders Activity Center
9120 South 300 West, Poneto, IN

Client Name : krM Architecture+
Project Number : 25-1836-04G
Logged By : C. Moreno
Start Date : 12/05/2025
Drilling Method : HSA

Driller : B. Roberts
Sampling : Splitspoon
Approx. Elevation : +/- 875 feet
Latitude :
Longitude :

Depth (Feet)	Elevation (Feet)	Water Level	USCS	GRAPHIC	Water Levels					REMARKS
					▼ During Drilling - Dry	▽ After Completion - Dry	◆ After 24 Hours - N/A	Samples	Rec %	
DESCRIPTION										
0	875				TOPSOIL (10")					
					Brown and gray, slightly moist to moist, stiff to very stiff, SANDY CLAY with trace gravel					
5	870		CL		1	56	3/6/8	>6.0	12	Boring caved to 7 feet upon auger removal.
					2	72	3/5/7	>6.0	13	
					3	67	3/7/11	>6.0	15	
					4	67	3/8/9	>6.0	16	
10	865				Boring terminated at 10 feet.					Groundwater was not encountered during, nor upon completion.
15	860									
20	855									
25	850									
30	845									



LOG OF BORING P-3

SWCS Raiders Activity Center
9120 South 300 West, Poneto, IN

Client Name : krM Architecture+
Project Number : 25-1836-04G
Logged By : C. Moreno
Start Date : 12/05/2025
Drilling Method : HSA

Driller : B. Roberts
Sampling : Splitspoon
Approx. Elevation : +/- 875 feet
Latitude :
Longitude :

Depth (Feet)	Elevation (Feet)	Water Level	USCS	GRAPHIC	Water Levels					REMARKS	
					▼ During Drilling - Dry	▽ After Completion - Dry	◆ After 24 Hours - N/A	Samples	Rec %		SPT Results
DESCRIPTION											
0	875				TOPSOIL (12")						
			CL		Brown and gray, moist, medium stiff, SILTY CLAY with trace sand and trace gravel	1	44	2/2/5	1.7	24	
			CL		Brown and gray, slightly moist, stiff to very stiff, SANDY CLAY with trace gravel	2	56	2/4/5	>6.0	14	
5	870					3	56	2/7/9	>6.0	15	Boring caved to 7 feet upon auger removal.
10	865					4	83	3/6/9	4.0	15	
					Boring terminated at 10 feet.						Groundwater was not encountered during, nor upon completion.
15	860										
20	855										
25	850										
30	845										

BORING LOG KEY

UNIFIED SOIL CLASSIFICATION SYSTEM FIELD CLASSIFICATION SYSTEM FOR SOIL EXPLORATION

NON COHESIVE SOILS

(Silt, Sand, Gravel and Combinations)

Density		Grain Size Terminology		
		<u>Soil Fraction</u>	<u>Particle Size</u>	<u>US Standard Sieve Size</u>
Very Loose	-4 blows/ft. or less	Boulders	Larger than 12"	Larger than 12"
Loose	-5 to 10 blows/ft.	Cobbles	3" to 12"	3" to 12"
Medium Dense	-11 to 30 blows/ft.	Gravel: Coarse	¾" to 3"	¾" to 3"
Dense	-31 to 50 blows/ft.	Small	4.76mm to ¾"	#4 to ¾"
Very Dense	-51 blows/ft. or more	Sand: Coarse	2.00mm to 4.76mm	#10 to #4
		Medium	0.42mm to 2.00mm	#40 to #10
		Fine	0.074mm to 0.42mm	#200 to #40
		Silt	0.005mm to 0.074 mm	Smaller than #200
		Clay	Smaller than 0.005mm	Smaller than #200

RELATIVE PROPORTIONS FOR SOILS

<u>Descriptive Term</u>	<u>Percent</u>
Trace	1 - 10
Little	11 - 20
Some	21 - 35
And	36 - 50

COHESIVE SOILS

(Clay, Silt and Combinations)

<u>Consistency</u>	<u>Unconfined Compressive Strength (tons/sq. ft.)</u>	<u>Field Identification (Approx.) SPT Blows/ft.</u>
Very Soft	Less than 0.25	0 - 2
Soft	0.25 - < 0.5	3 - 4
Medium Stiff	0.5 - < 1.0	5 - 8
Stiff	1.0 - < 2.0	9 - 15
Very Stiff	2.0 - < 4.0	16 - 30
Hard	Over 4.0	> 30

Classification on logs are made by visual inspection.

Standard Penetration Test - Driving a 2.0" O.D., 1^{3/8}" I.D., sampler a distance of 1.0 foot into undisturbed soil with a 140 pound hammer free falling a distance of 30.0 inches. It is customary for **Patriot** to drive the spoon 6.0 inches to seat into undisturbed soil, then perform the test. The number of hammer blows for seating the spoon and making the tests are recorded for each 6.0 inches of penetration on the drill log (Example - 6/8/9). The standard penetration test results can be obtained by adding the last two figures (i.e. 8 + 9 = 17 blows/ft.).

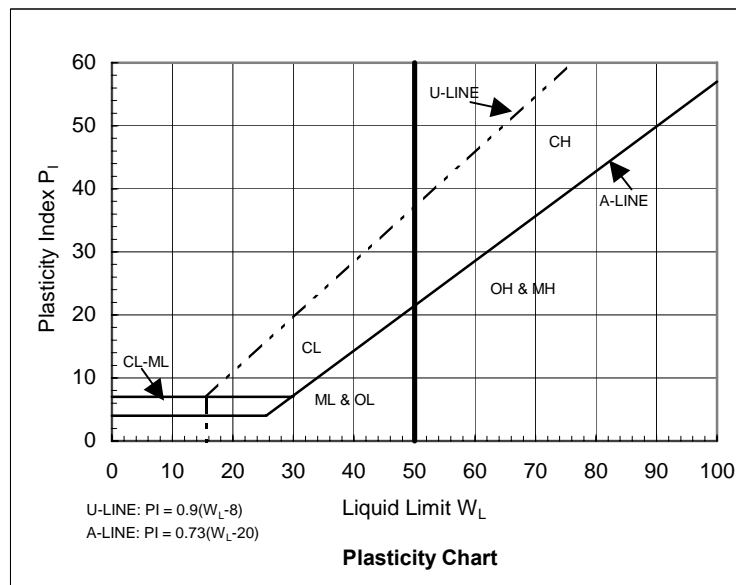
Strata Changes - In the column "Soil Descriptions" on the drill log the horizontal lines represent strata changes. A solid line (————) represents an actually observed change, a dashed line (- - - -) represents an estimated change.

Groundwater observations were made at the times indicated. Porosity of soil strata, weather conditions, site topography, etc., may cause changes in the water levels indicated on the logs.

Groundwater symbols: ▼-observed groundwater elevation, encountered during drilling; ▽-observed groundwater elevation upon completion of boring.

Unified Soil Classification System

Major Divisions		Group Symbol	Typical Names	Classification Criteria for Coarse-Grained Soils			
Coarse-grained soils (more than half of material is larger than No. 200)	Gravels (more than half of coarse fraction is larger than No. 4 sieve size)	Clean gravels (little or no fines)	GW	Well-graded gravels, gravel-sand mixtures, little or no fines	$C_u \geq 4$ $1 \leq C_c \leq 3$	$C_u = \frac{D_{60}}{D_{10}}$	$C_c = \frac{D_{30}^2}{D_{10} D_{60}}$
			GP	Poorly graded gravels, gravel-sand mixtures, little or no fines	Not meeting all gradation requirements for GW ($C_u < 4$ or $1 > C_c > 3$)		
		Gravels with fines (appreciable amount of fines)	GM	Clayey gravels, gravel-sand-clay mixtures	Atterberg limits below A line or $P_i < 4$		Above A line with $4 < P_i < 7$ are borderline cases requiring use of dual symbols
			GC		Atterberg limits above A line or $P_i > 7$		
	Sands (more than half of coarse fraction is smaller than No. 4 sieve size)	Clean sands (little or no fines)	SW	Well-graded sands, gravelly sands, little or no fines	$C_u \geq 6$ $1 \leq C_c \leq 3$	$C_u = \frac{D_{60}}{D_{10}}$	$C_c = \frac{(D_{30})^2}{D_{10} D_{60}}$
			SP	Poorly graded sands, gravelly sands, little or no fines	Not meeting all gradation requirements for SW ($C_u < 6$ or $1 > C_c > 3$)		
		Sands with fines (appreciable amount of fines)	SM	Clayey sands, sand-clay mixtures	Atterberg limits below A line or $P_i < 4$		Limits plotting in hatched zone with $4 \leq P_i \leq 7$ are borderline cases requiring use of dual symbols
			SC		Atterberg limits above A line with $P_i > 7$		
	Fine-grained soils (more than half of material is smaller than No. 200)	Silt and clays (liquid limit <50)	ML	Inorganic silts and very fine sands, rock flour, silty or clayey fine sands, or clayey silts with slight plasticity	<ol style="list-style-type: none"> Determine percentages of sand and gravel from grain size curve. Depending on percentages of fines (fraction smaller than 200 sieve size), coarse-grained soils are classified as follows: Less than 5% - GW, GP, SW, SP More than 12% - GM, GC, SM, SC 5-12% - Borderline cases requiring dual symbols 		
			CL	Inorganic clays of low to medium plasticity, gravelly clays, sandy clays, silty clays, lean clays			
OL			Organic silts and organic silty clays of low				
Silt and clays (liquid limit >50)		MH	Inorganic silts, micaceous or diatomaceous fine sandy or silty soils, elastic silts				
		CH	Inorganic clays or high plasticity, fat clays				
		OH	Organic clays of medium to high plasticity, organic silts				
Highly organic soils		PT	Peat and other highly organic soils				



APPENDIX B

GENERAL QUALIFICATIONS

**STANDARD CLAUSE FOR UNANTICIPATED
SUBSURFACE CONDITIONS**

GENERAL QUALIFICATIONS
of Patriot Engineering's Geotechnical Engineering Investigation

This report has been prepared at the request of our client for his use on this project. Our professional services have been performed, findings obtained, and recommendations prepared in accordance with generally accepted geotechnical engineering principles and practices. This warranty is in lieu of all other warranties either expressed or implied.

The scope of our services did not include any environmental assessment or investigation for the presence or absence of wetlands, hazardous or toxic materials in the soil, groundwater, or surface water within or beyond the site studied. Any statements in this report or on the test borings logs regarding vegetation types, odors or staining of soils, or other unusual conditions observed are strictly for the information of our client and the owner.

This report may not contain sufficient information for purposes of other parties or other uses. This company is not responsible for the independent conclusions, opinions or recommendations made by others based on the field and laboratory data presented in this report. Should there be any significant differences in structural arrangement, loading or location of the structure, our analysis should be reviewed.

The recommendations provided herein were developed from the information obtained in the test borings, which depict subsurface conditions only at specific locations. The analysis, conclusions, and recommendations contained in our report are based on site conditions as they existed at the time of our exploration. Subsurface conditions at other locations may differ from those occurring at the specific drill sites. The nature and extent of variations between borings may not become evident until the time of construction. If, after performing on-site observations during construction and noting the characteristics of any variation, substantially different subsurface conditions from those encountered during our explorations are observed or appear to be present beneath excavations, we must be advised promptly so that we can review these conditions and reconsider our recommendations where necessary.

If there is a substantial lapse of time between the submission of our report and the start of work at the site, or if conditions have changed due to natural causes or construction operations at or adjacent to the site, we urge that our report be reviewed to determine the applicability of the conclusions and recommendations considering the changed conditions and time lapse.

We urge that Patriot be retained to review those portions of the plans and specifications that pertain to earthwork and foundations to determine whether they are consistent with our recommendations. In addition, we are available to observe construction, particularly the compaction of structural backfill and preparation of the foundations, and such other field observations as may be necessary.

In order to fairly consider changed or unexpected conditions that might arise during construction, we recommend the following verbiage (Standard Clause for Unanticipated Subsurface Conditions) be included in the project contract.

STANDARD CLAUSE FOR UNANTICIPATED SUBSURFACE CONDITIONS

"The owner has had a subsurface exploration performed by a soils consultant, the results of which are contained in the consultant's report. The consultant's report presents his conclusions on the subsurface conditions based on his interpretation of the data obtained in the exploration. The contractor acknowledges that he has reviewed the consultant's report and any addenda thereto, and that his bid for earthwork operations is based on the subsurface conditions as described in that report. It is recognized that a subsurface exploration may not disclose all conditions as they actually exist and further, conditions may change, particularly groundwater conditions, between the time of a subsurface exploration and the time of earthwork operations. In recognition of these facts, this clause is entered in the contract to provide a means of equitable additional compensation for the contractor if adverse unanticipated conditions are encountered and to provide a means of rebate to the owner if the conditions are more favorable than anticipated.

At any time during construction operations that the contractor encounters conditions that are different than those anticipated by the soils consultant's report, he shall immediately (within 24 hours) bring this fact to the owner's attention. If the owner's representative on the construction site observes subsurface conditions which are different than those anticipated by the consultant's report, he shall immediately (within 24 hours) bring this fact to the contractor's attention. Once a fact of unanticipated conditions has been brought to the attention of either the owner or the contractor, and the consultant has concurred, immediate negotiations will be undertaken between the owner and the contractor to arrive at a change in contract price for additional work or reduction in work because of the unanticipated conditions. The contract agrees that the following unit prices would apply for additional or reduced work under the contract. For changed conditions for which unit prices are not provided, the additional work shall be paid for on a time and materials basis."

Another example of a changed conditions clause can be found in paper No. 4035 by Robert F. Borg, published in ASCE Construction Division Journal, No. CO2, September 1964, page 37.

**SECTION 101419
DIMENSIONAL LETTER SIGNAGE**

PART 1 GENERAL

1.01 SECTION INCLUDES

- A. Dimensional letter signage.
- B. Exterior Applied Signs.

1.02 REFERENCE STANDARDS

- A. 36 CFR 1191 - Americans with Disabilities Act (ADA) Accessibility Guidelines for Buildings and Facilities; Architectural Barriers Act (ABA) Accessibility Guidelines; current edition.
- B. ADA Standards - 2010 ADA Standards for Accessible Design; 2010.
- C. ICC A117.1 - Accessible and Usable Buildings and Facilities; 2017.
- D. NFPA 70 - National Electrical Code; Most Recent Edition Adopted by Authority Having Jurisdiction, Including All Applicable Amendments and Supplements.
- E. UL 879 - Electric Sign Components; Current Edition, Including All Revisions.

1.03 SUBMITTALS

- A. See Section 013000 - Administrative Requirements for submittal procedures.
- B. Product Data: Manufacturer's product literature for each type of dimensional letter sign, indicating style, font, colors, locations, and overall dimensions of each sign.
- C. Shop Drawings:
 - 1. Include dimensions, locations, elevations, materials, text and graphic layout, and attachment details.
 - 2. Show locations of electrical service connections.
 - 3. Include diagrams for power, signal, and control wiring.
- D. Samples: Submit two samples of each type of dimensional letter sign of size similar to that required for project, indicating sign style, font, and method of attachment.
- E. Selection Samples: Where materials, colors, and finishes are not specified, submit two sets of selection charts or chips.
- F. Maintenance Materials: Furnish the following for Owner's use in maintenance of project.

1.04 DELIVERY, STORAGE, AND HANDLING

- A. Package dimensional letter signs as required to prevent damage before installation.
- B. Store under cover and elevated above grade.
- C. Store tape adhesive at a normal room temperature of 68 to 72 degrees F (20 to 22 degrees C).

1.05 FIELD CONDITIONS

- A. Do not install tape adhesive when ambient temperature is lower than recommended by manufacturer.
- B. Maintain minimum ambient temperature during and after installation.

PART 2 PRODUCTS

2.01 MANUFACTURERS

- A. Manufacturers"
 - 1. Best Sign Systems, Inc; ____: www.bestsigns.com/#sle.
 - 2. Seton Identification Products; ____: www.seton.com.
 - 3. APCO Graphics; ____: www.apcosigns.com
 - 4. Westlund Concepts: dwestlund@westlundconcepts.com
 - 5. Essential Architectural Signs www.essentialsigns.com.
 - 6. MD Designs, Inc; ____: 15372 Mystic Rock Dr. Carmel In .

7. ASI-Modulex, Inc; ____: www.2017 W 18th Street, Indianapolis,IN
8. Substitutions: See Section 01 60 00 - Product Requirements.

B. Dimensional Letter Signs:

1. FASTSIGNS International, Inc; ____: www.fastsigns.com/#sle.
2. Inpro Corporation; ____: www.inprocorp.com/#sle.
3. Substitutions: See Section 016000 - Product Requirements.

2.02 REGULATORY REQUIREMENTS

- A. Accessibility Requirements: Comply with ADA Standards and ICC A117.1 and applicable building codes, unless otherwise indicated; in the event of conflicting requirements, comply with the most restrictive requirements.

2.03 DIMENSIONAL LETTERS

A. Applications: Building identification.

1. Use individual metal letters.
2. Mounting Location: Exterior as indicated on drawings.
3. Design and style as indicated on drawings.

B. Metal Letters (Exterior):

1. Material: Aluminum casting.
2. Thickness: As indicated on drawings.
3. Letter Height: ____ inches (____ mm). As indicated on drawings.
4. Text and Typeface:
 - a. Character Font: Helvetica, Arial, or other sans serif font.
5. Finish: Powder coated.
6. Color: Selected by Architect from full custom range.
7. Mounting: Projected mount with collars.
8. Illumination System: led backlit for each letter to provide uniform glow.
 - a. Provide products that are listed and labeled as complying with UL 879, where applicable.
 - b. Power: 120 V, 60 Hz, 1 phase, 15 A.

C. Plastic Letters (Interior):

1. Material: Acrylic plastic sheet, flat.
2. Thickness: Manufacturer's standard for letter size.
3. Letter Height: ____ inches (____ mm). As indicated on drawings.
4. Text and Typeface:
 - a. Character Font: Helvetica, Arial, or other sans serif font.
5. Finish: Semi-gloss.
6. Color: As selected.
7. Mounting: Tape adhesive.

D. CUSTOM ACRYLIC FACE W/ ALUMINUM BODY INTERNALLY LIT CABINET STYLE SIGN

1. Black cast Aluminum Retainer border.
2. Material: 3/16" impact modified acrylic or polycarbonate face sheet(s). Custom printed logo provided by Architect.
3. Material housing: cast channel aluminum housing for LED lights along perimeter of the sign
4. Mounting: Securely fasten to (3) continuous 2" wide x 1/8" thick horizontal galvanized steel tubes bolted through to building structure.
5. Electrical: LED lights to meet min. standard of 200 LM/FT.

E. Custom Acrylic "SW" logo interior signage

1. Custom color acrylic selected by Architect. Flat sheet.
2. Thickness: Manufacturer's standard for sign size.
3. Finish: Semi-gloss.
4. Join acrylic seams for large pieces.

5. Mounting: Tape adhesive.
6. Size: As indicated on drawings.

2.04 ACCESSORIES

- A. Concealed Screws: Noncorroding metal; stainless steel, galvanized steel, or other non-corroding metal.
- B. Tape Adhesive: Double-sided tape, permanent adhesive.
- C. Electrical Components and Devices: Listed and labeled as defined in NFPA 70 by a qualified testing agency.

PART 3 EXECUTION

3.01 EXAMINATION

- A. Verify that substrate surfaces are ready to receive work.
- B. Verify that electrical service is correctly sized and located to accommodate dimensional letter signs.
- C. Notify Architect if conditions are not suitable for installation of signs; do not proceed until conditions are satisfactory.

3.02 INSTALLATION

- A. Install in accordance with manufacturer's instructions.
- B. Install with horizontal edges level.
- C. Locate dimensional letter signs and mount at heights indicated on drawings and in accordance with ADA Standards and ICC A117.1.
- D. Protect from damage until 06-01-2027; repair or replace damaged items.

END OF SECTION

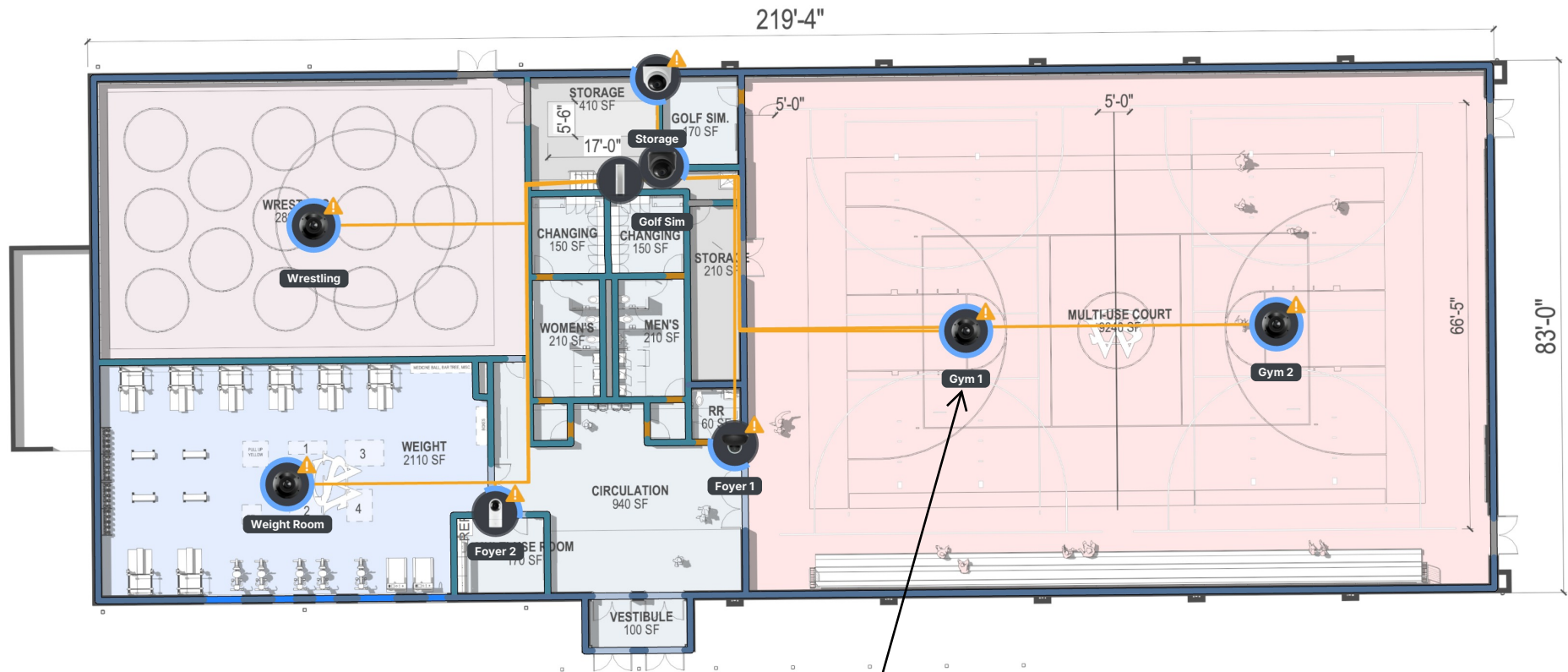


The RAC

Southern Wells Schools

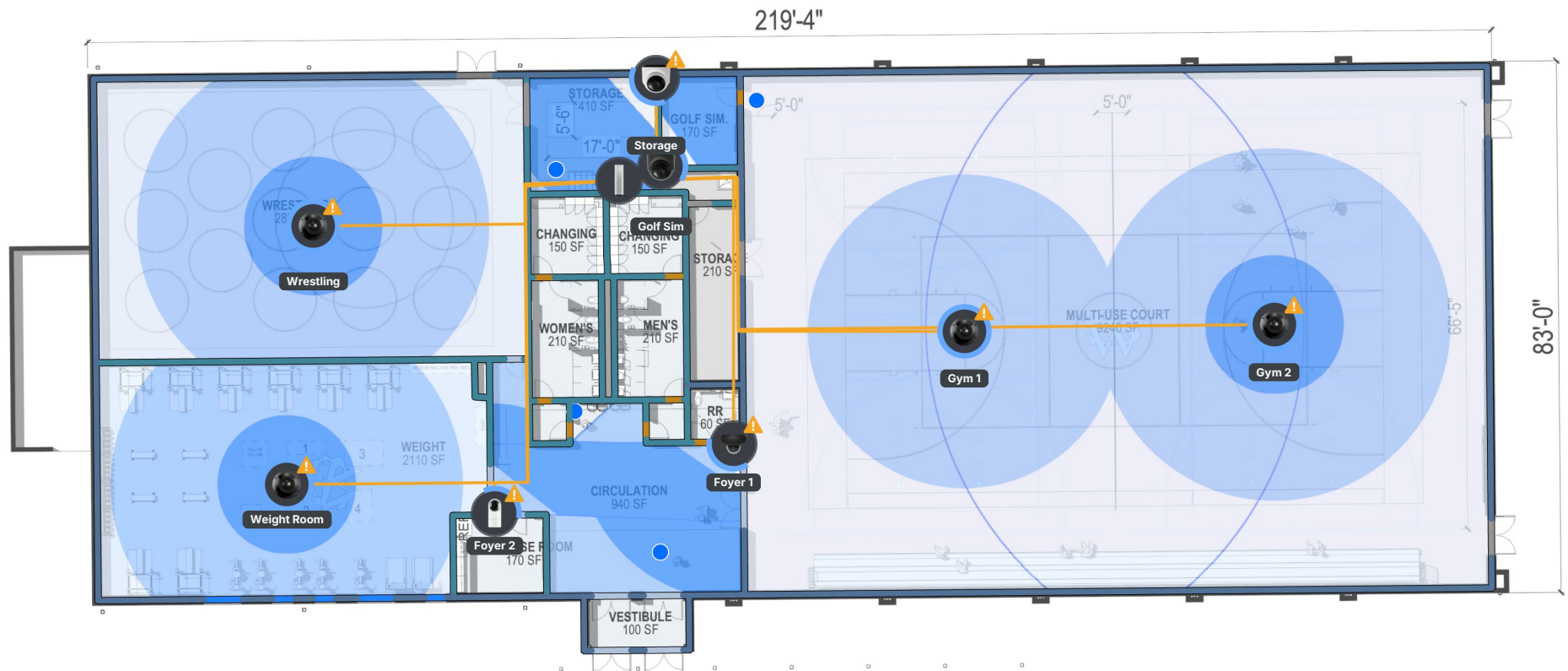
April 3, 2026

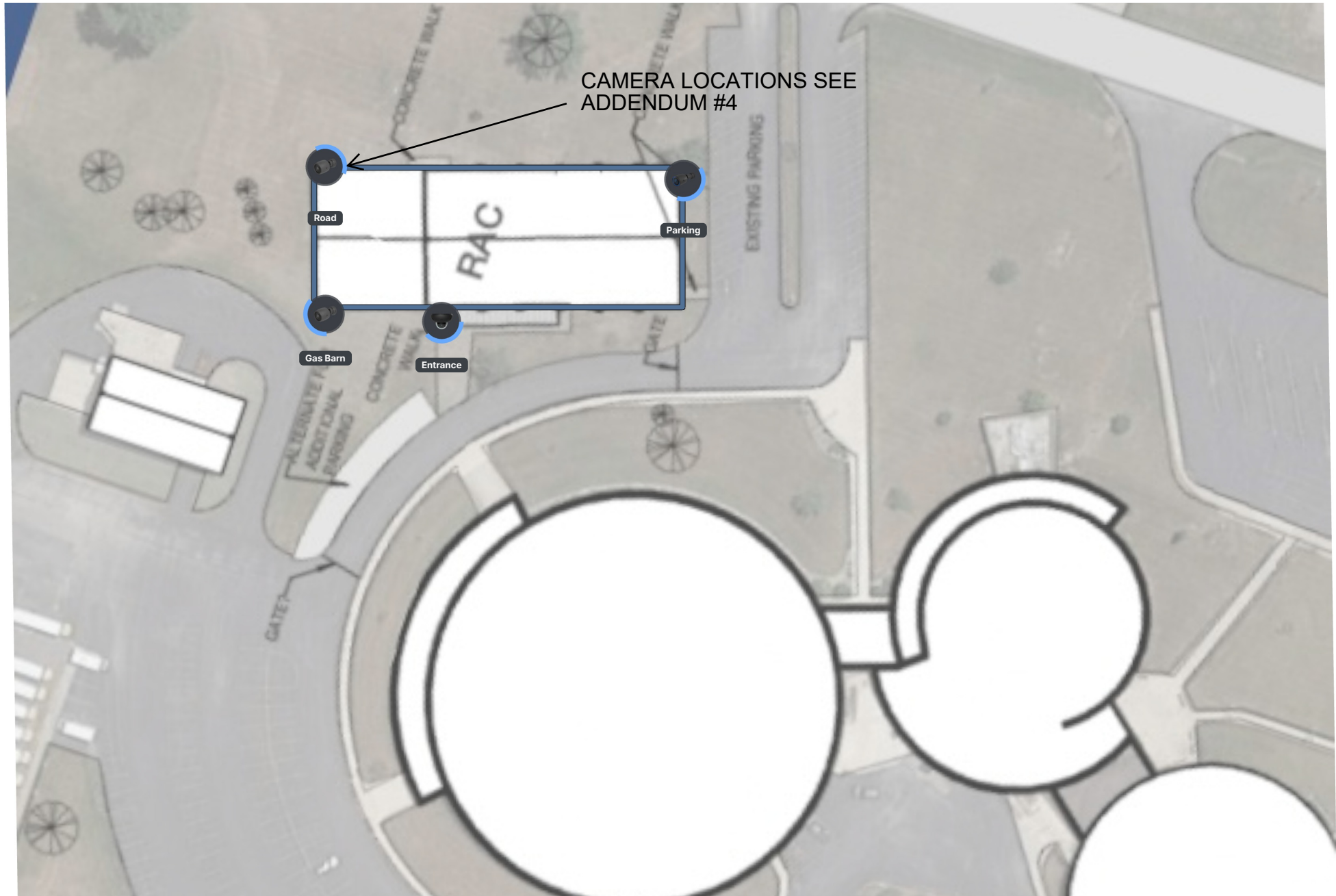
Inside



CAMERA LOCATIONS, SEE
ADDENDUM #4

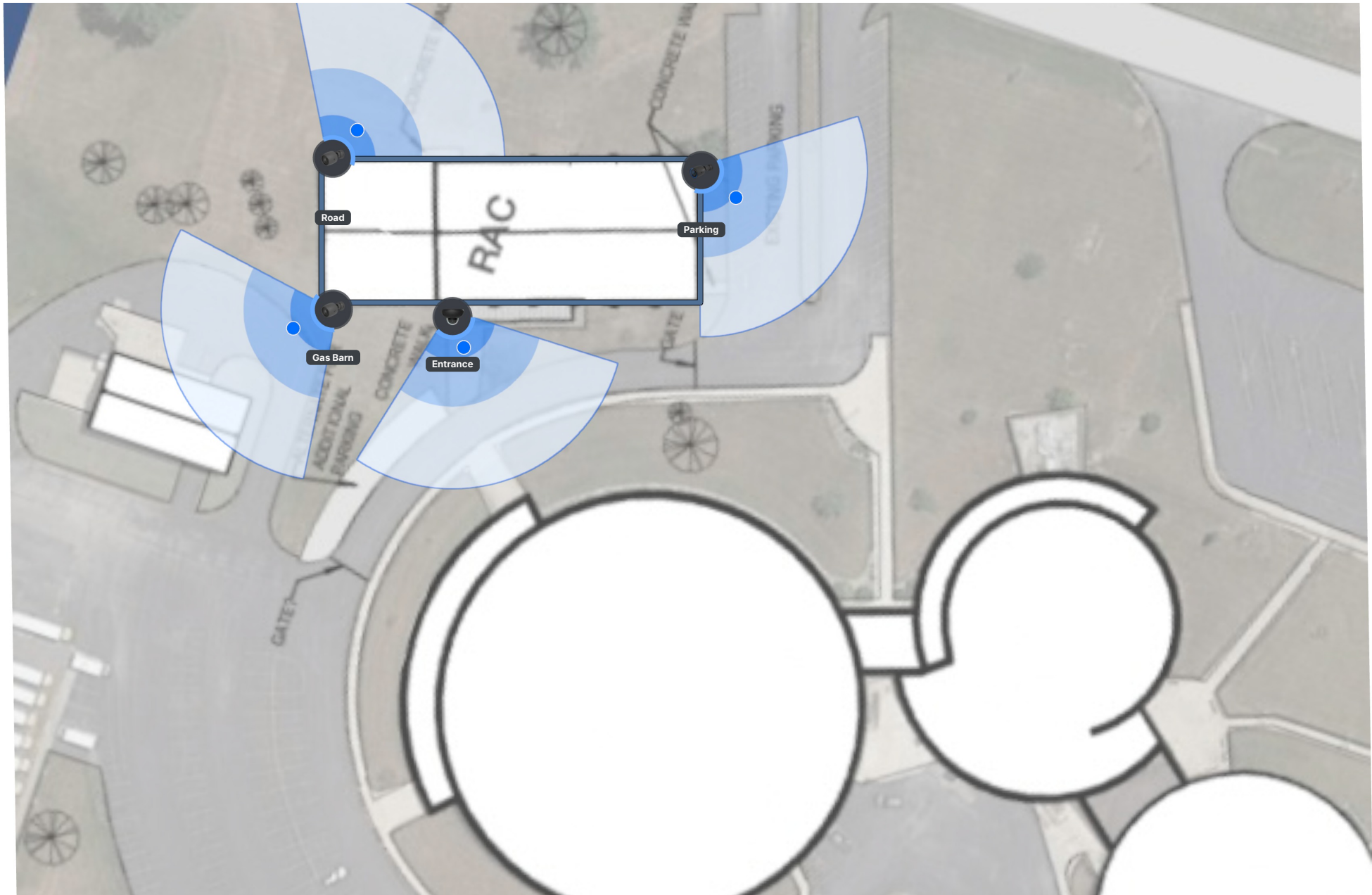
Camera Coverage Inside





Camera Coverage

Arial





4175 New Vision Drive, Fort Wayne, IN 46845
Ph: (260) 490-1025 www.eri.com/US

REVISIONS		
NO.	DATE	DESCRIPTION
1	03/27/2026	ADDENDUM #3

03.05.2026
SOUTHERN WELLS COMMUNITY SCHOOLS
Raiders Activity Center
6120 S 300th W
POMEROY, IN 46761
BIDDING SET

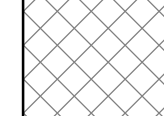
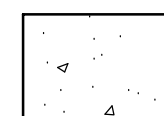


Mark D. Reinhard

PERMIT SET
03.27.2026
ERI JOB NO.
6698
DRAWN BY
CMF
DRAWING NAME
SITE DEMOLITION PLAN

DRAWING NO.
C1.0

DEMOLITION LEGEND:

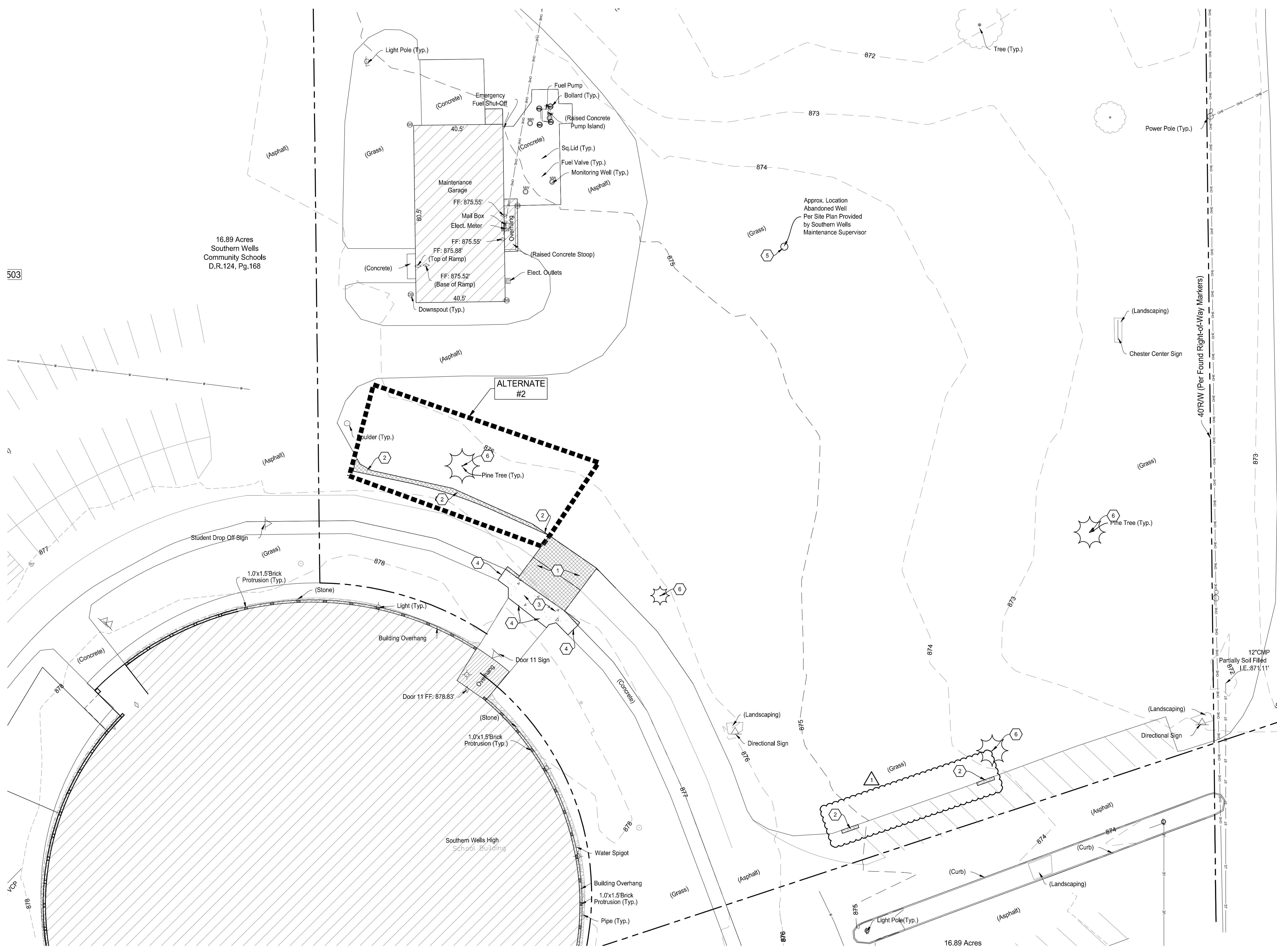
-  SAWCUT AND REMOVE ASPHALT PAVEMENT.
-  SAWCUT AND REMOVE CONCRETE SIDEWALK.

DEMOLITION NOTES:

- 1 SAWCUT AND REMOVE ASPHALT PAVEMENT.
- 2 SAWCUT AND REMOVE 18" OF ASPHALT PAVEMENT TO PROVIDE CLEAN EDGE.
- 3 SAWCUT AND REMOVE CONCRETE SIDEWALK/PAVEMENT AND STONE BASE.
- 4 SAWCUT AND REMOVE CONCRETE SIDEWALK AT STREET FRONT.
- 5 CONTRACTOR IS TO VERIFY THAT EXISTING WELL HAS BEEN SEALED AND ABANDONED. IF THE WELL HAS NOT BEEN SEALED AND ABANDONED, CONTRACTOR IS TO REMOVE AND SEAL ABANDONED WELL BY LICENSED WELL CONTRACTOR.
- 6 REMOVE TREE(S), INCLUDING ROOT BALLS.

GENERAL NOTES:

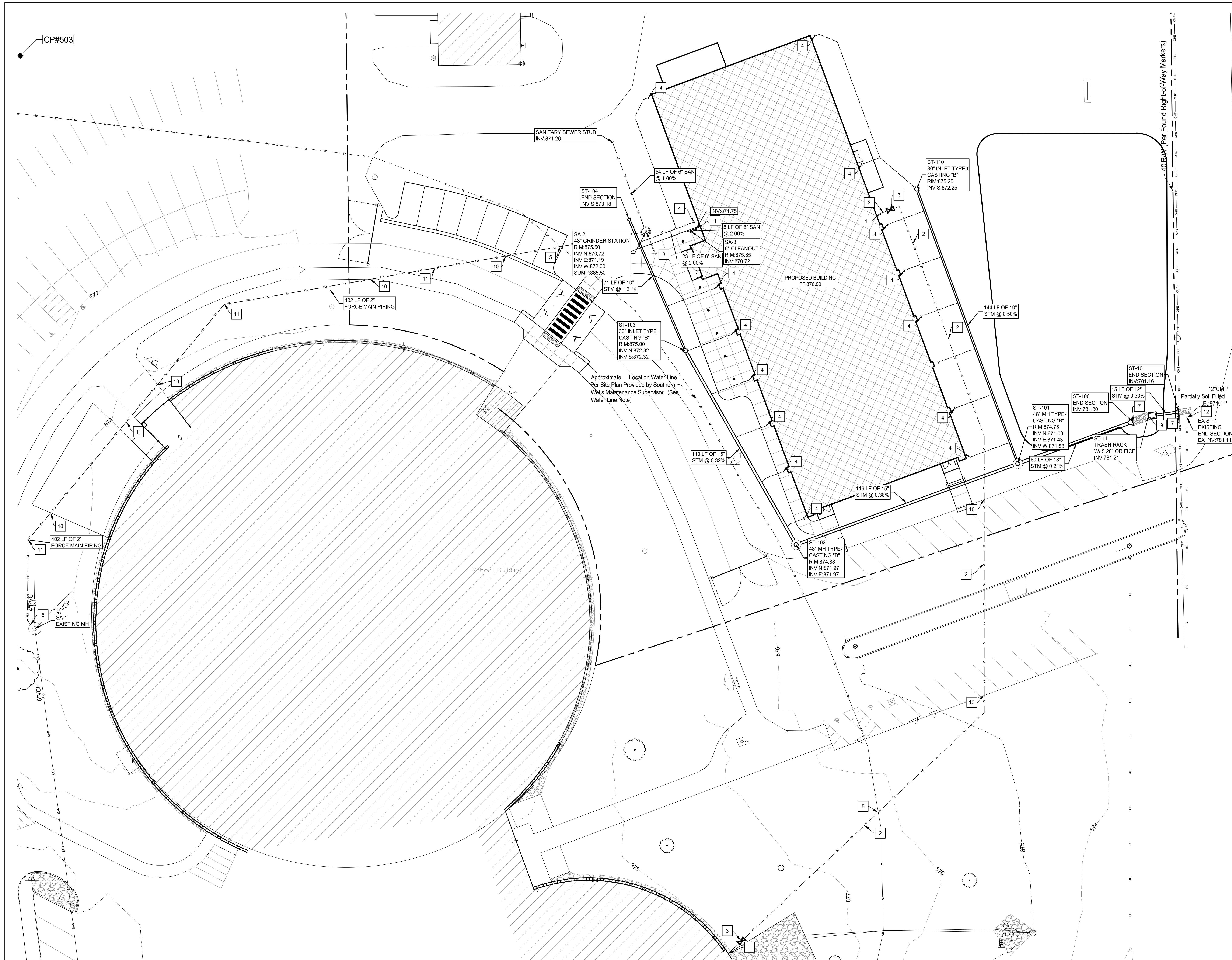
1. OBTAIN ALL REQUIRED PERMITS AND COORDINATE INSPECTIONS FROM AUTHORITIES HAVING JURISDICTION.
2. CONTRACTOR SHALL NOT INTERRUPT ANY SERVICE TO ADJACENT PROPERTIES WITHOUT WRITTEN AUTHORIZATION FROM PROPERTY OWNER. AN EMERGENCY PLAN SHALL BE PROVIDED TO THE ENGINEER PRIOR TO CONSTRUCTION TO OUTLINE CORRECTIVE MEASURES IN THE EVENT OF ANY UNAUTHORIZED UTILITY SHUTDOWN.
3. CONTRACTOR SHALL STUDY ALL DRAWINGS PRIOR TO CONSTRUCTION. RESEARCH PUBLIC UTILITY RECORDS, CONTACT THE LOCAL UTILITY LOCATOR SERVICE, AND FIELD VERIFY ALL EXISTING STRUCTURES PRIOR TO CONSTRUCTION. CONTACT ENGINEER FOR DIRECTION IF EXISTING UTILITY CONDITIONS CONFLICT WITH PROPOSED WORK OR ANY ALTERATIONS SHALL BE THE CONTRACTORS RESPONSIBILITY.
4. EXISTING UTILITIES ARE APPROXIMATIONS BASED ON BEST AVAILABLE DATA. CAUTION SHALL BE EXERCISED TO NOT INTERRUPT SERVICE TO ANY BUILDING. EXPLORATORY TRENCH TO VERIFY DEPTH AND LOCATION OF SEWERS PRIOR TO CONSTRUCTION OF NEW SEWER UTILITIES. ASSURE ALL SANITARY FLOW IS DIRECTED INTO THE SANITARY SEWER ON-SITE AND ALL STORM WATER IS DIRECTED INTO THE STORM SEWER SYSTEM.
5. CONTRACTOR IS RESPONSIBLE FOR ALL COORDINATION REQUIRED BY UTILITY OWNERS TO CONSTRUCT PROJECT.
6. PROVIDE RECORD DRAWINGS TO THE OWNER FOR BELOW GRADE IMPROVEMENTS. INCLUDE MATERIALS OF CONSTRUCTION, SIZE, ELEVATIONS, AND LOCATION DESCRIPTIONS IN THE RECORD. RECORD DRAWINGS SHALL BE CERTIFIED BY A LAND SURVEYOR REGISTERED IN THE STATE OF INDIANA.
7. CONTRACTOR SHALL COORDINATE WITH EACH UTILITY PROVIDER TO DETERMINE TOTAL COST OF SERVICE TO BUILDING AND TO INCLUDE IN THE COST OF THE PROJECT.
8. CONTRACTOR SHALL LOCATE ALL PRIVATE UTILITIES NOT COVERED BY THE PUBLIC LOCATING SERVICE.
9. CONSTRUCTION DE-WATERING AS NECESSARY BY CONTRACTOR.
10. ADJUST ANY EXISTING MANHOLES, VALVES, HYDRANTS, AND HANDHOLES, LOCATED WITHIN PROJECT LIMITS, TO PROPOSED GRADES.
11. CONTRACTOR SHALL SUPPORT AND PROTECT ALL EXISTING UTILITIES DURING CONSTRUCTION OF ADJACENT WORK.
12. SEE SITE SURVEY FOR EXISTING CONDITIONS. IF ABANDONED WELLS ARE DISCOVERED ON THE PROPERTY, THEY MUST BE CAPPED BY A LICENSED WELL DRILL DRILLER ACCORDING TO INDIANA WELL DRILLING CODES AND REGULATIONS.
13. COORDINATE ALL DEMOLITION WORK WITH OWNER.
14. CONTRACTOR IS RESPONSIBLE FOR ALL PERMIT FEES, TAPPING FEES, INSPECTION FEES, ETC.



 **SITE DEMOLITION PLAN**
1" = 20'

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April 13, 2026 11:00 AM
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SITE UTILITY PLAN
 SCALE: 1" = 20'
 NORTH

- UTILITY NOTES:**
- COORDINATE UTILITY CONNECTION, LOCATION, SIZE, MATERIAL AND INVERT WITH BUILDING PLUMBING PLANS. MAKE FINAL CONNECTIONS.
 - 2" CURB STOP AND BOX
 - 6" DOWNSPOUT DRAIN @ MINIMUM 1.00% SLOPE. CONNECT TO DOWNSPOUTS AS REQUIRED. MAINTAIN 18" MINIMUM COVER. SEE BUILDING PLANS. CONNECT TO STORM NETWORK WITH INSERT-A-TEE.
 - FIELD VERIFY DEPTH AND LOCATION OF EXISTING WATER LINE. NOTIFY ENGINEER AND WAIT FOR INSTRUCTION IF CONFLICTS WITH PROPOSED CONSTRUCTION.
 - CORE HOLE IN EXISTING STRUCTURE. PROVIDE WATER TIGHT CONNECTION. REMOVE AND RECONSTRUCT STRUCTURE INVERT AS NEEDED.
 - PIPE OUTFALL PER DETAIL #4/C5.1.
 - SANITARY GRINDER STATION PER DETAIL #7/C5.1.
 - TRASH RACK PER DETAIL #8/C6.1.
 - CONTRACTOR IS TO DIRECTIONAL DRILL PROPOSED UTILITY LINE.
 - CONTRACTOR IS TO CONFIRM NO INTERMITTENT LOW POINTS IN FORCE MAIN LINES.
 - CONTRACTOR IS TO CLEAR EXISTING STORM CULVERT PIPE TO PROVIDE POSITIVE DRAINAGE.
- NOTE: CONTRACTOR SHALL PROVIDE AS-BUILT SURVEY OF FINAL UTILITIES. PROVIDE COPY TO LOCAL AUTHORITIES AS REQUIRED.
- NOTE: PIPE LENGTHS ARE MEASURED TO THE CENTER OF STRUCTURES AND THE END OF END SECTIONS/TRASH RACKS UNLESS OTHERWISE NOTED.
- NOTE: ALL EXISTING UTILITIES SHOWN IN PROFILE ARE APPROXIMATE LOCATION AND SIZE.
- NOTE: ADJUST ALL EXISTING MANHOLES, VALVES, HYDRANTS AND HANDHOLES TO PROPOSED GRADES.

- PROPOSED LEGEND:**
- STORM INLET / MANHOLE
 - STORM END SECTION
 - STORM TRASH RACK
 - LIFT STATION
 - SANITARY CLEANOUT / TYPE IV INLET
 - GATE VALVE
 - STORM SEWER
 - DOWNSPOUT DRAIN
 - SUBDRAIN
 - SANITARY SEWER
 - FORCE MAIN
 - WATER LINE

- STORM SEWER NOTES:**
- MATERIAL AND WORKMANSHIP SHALL COMPLY WITH THE WELLS COUNTY STANDARDS AND SPECIFICATIONS.
 - ALL PIPE 12" AND SMALLER SHALL BE SDR 35 PVC. OR ADS N-12 HDPE UNLESS OTHERWISE NOTED. ALL PIPE LARGER THAN 12" SHALL BE ADS N-12 HDPE OR C76 C-11 RCP UNLESS OTHERWISE NOTED. ALL PIPE SHALL BE INSTALLED ACCORDING TO SPECIFICATIONS AND PIPE TRENCH DETAIL #1/C6.1.
 - MAINTAIN 10'-0" MINIMUM HORIZONTAL AND 18" MINIMUM VERTICAL SEPARATION BETWEEN ALL SEWER PIPING AND POTABLE WATER PIPING. WHEN MINIMUM TOLERANCES CAN'T BE MAINTAINED, USE WATERWORKS GRADE PIPE AND FITTINGS OF MATERIAL SELECTED.
 - COORDINATE TAP LOCATIONS FOR ROOF DRAINS WITH BUILDING PLUMBING DRAWINGS. ASSURE ALL REQUIRED FITTINGS ARE INSTALLED ON THE MAIN LINE PRIOR TO BACKFILLING. INCLUDE ADAPTER FITTING FOR DOWNSPOUTS.
 - ALL UNDERGROUND PIPING FOR DOWNSPOUT COLLECTION SYSTEM SHALL BE 6" SDR 35 PVC STORM @ 1.00% MIN. SLOPE UNLESS NOTED OTHERWISE.

- WELLS COUNTY SANITARY SEWER NOTES:**
- ALL CONSTRUCTION SHALL COMPLY WITH BOTH THE WELLS COUNTY AND THE INDIANA DEPARTMENT OF ENVIRONMENTAL MANAGEMENT CURRENT STANDARDS AND SPECIFICATIONS.
 - IT IS RECOMMENDED THAT THE CONTRACTOR NOTIFY INDIANA UNDERGROUND (1-800-382-5544) 48 HOURS PRIOR TO CONSTRUCTION FOR THE EXACT LOCATION OF EXISTING UTILITIES.
 - ALL POLYVINYL CHLORIDE PIPE (PVC) SHALL CONFORM TO 327 IAC 3-6-8-5 SPECIFICATION ASTM D-3034-97, CLASS 12454-B, D-1785-96b.
 - ALL PVC JOINTS SHALL CONFORM TO ASTM SPECIFICATION D-3212-96a FOR ELASTOMERIC GASKET JOINTS CONFORMING TO ASTM F-477-99.
 - SEWER TO WATER MAIN SEPARATION DISTANCES SHALL CONFORM TO 327 IAC 3-6-9. WHENEVER SEWERS MUST CROSS UNDER WATER MAINS, THE SEWER SHALL BE LAID AT SUCH AN ELEVATION THAT THE TOP OF THE SEWER IS AT LEAST 18" BELOW THE WATER MAIN. WHEN THE ELEVATION OF THE SEWER CANNOT BE BURIED TO MEET SAID REQUIREMENTS, THE SEWER SHALL BE RECONSTRUCTED WITH MECHANICAL JOINT DUCTILE IRON PIPE OR AWWA C-900 CLASS 150, SDR 15, PVC FOR A DISTANCE OF 10 FEET ON EACH SIDE OF THE WATER MAIN. ONE FULL LENGTH OF SEWER SHOULD BE CENTERED OVER THE WATER MAIN SO THAT BOTH JOINTS WILL BE AS FAR FROM THE WATER MAIN AS POSSIBLE. IN ALL OCCURRENCES, WHERE WATER LINES AND SANITARY SEWER LINES DO NOT PERMIT A MINIMUM OF TEN (10) FEET HORIZONTAL SEPARATION, THE SEWER MUST BE CONSTRUCTED EQUAL TO WATER PIPE WITH COMPRESSION JOINTS AND SHALL BE PRESSURE TESTED TO ASSURE WATER TIGHTNESS PRIOR TO BACKFILLING.
 - INSPECTION MUST BE PROVIDED FOR ALL SEWER CONSTRUCTION AND PAID FOR BY THE CONTRACTOR.
 - THE SANITARY SEWER PIPE SHALL BE POLYVINYL CHLORIDE (PVC), ASTM D-3034, SDR 35, UNLESS OTHERWISE NOTED.
 - THE CONTRACTOR SHALL BE RESPONSIBLE FOR PROVIDING ALL REQUIRED TESTING FOR THE SEWER INSTALLATION AS REQUIRED BY THE UTILITY.
 - CONTRACTOR SHALL SUPPLY BY-PASS PUMPING AS REQUIRED FOR CONSTRUCTION.
 - ALL PIPE SHALL BE INSTALLED ACCORDING TO SPECIFICATIONS AND PIPE TRENCH DETAIL #1/C6.1.

- WELLS COUNTY WATER MAIN NOTES:**
- WATER TO BE SUPPLIED BY THE WELLS COUNTY WATER UTILITY
 - WATER MAINS SHALL BE INSTALLED ACCORDING TO WELLS COUNTY STANDARDS AND SPECIFICATIONS.
 - THE WATER MAIN IS TO BE INSTALLED WITH A MINIMUM COVER OF 5'-0"
 - WHERE SANITARY AND/OR STORM SEWER(S) AND WATER MAIN CROSS, ONE FULL LENGTH OF WATER MAIN SHOULD BE CENTERED OVER THE SANITARY AND/OR STORM SEWER. AND THE VERTICAL DISTANCE TO BE A MINIMUM OF 18 INCHES. WHERE WATER LINES AND SEWER CROSS AND THE CLEARANCE CANNOT BE MAINTAINED, THE SEWER MUST BE CONSTRUCTED OF WATERWORKS GRADE DUCTILE IRON PIPE WITH MECHANICAL JOINTS OF SDR 21 PVC PRESSURE SEWER PIPE WITH COMPRESSION FITTINGS WITHIN TEN FEET OF THE WATER LINE.
 - WHERE A WATER MAIN CROSSES UNDER A SEWER, THE MAIN SHALL USE 22" ELBOWS TO MINIMIZE THE LENGTH OF WATER MAIN INSTALLED IN EXCESS OF 5.0 FEET COVER.
 - THE MINIMUM HORIZONTAL DISTANCE BETWEEN THE WATER MAIN AND THE STORM OR SANITARY SEWER MAIN IS 10.0 FEET.
 - ALL TEES, CROSSES, BENDS, AND ELBOWS EXCEEDING 1 1/2" SHALL BE RESTRAINED.
 - WATER SERVICE TO BE DUCTILE IRON TO MEET BUILDING, MECHANICAL, AND LOCAL UTILITY REQUIREMENTS AND CODES. FOR MATERIAL AND JOINTS.
 - ALL PIPE JOINTS SHALL BE IN ACCORDANCE WITH ANSI SPECIFICATIONS OF A21.11 (AWWA C-111).
 - ALL MATERIALS ARE CERTIFIED IN ACCORDANCE WITH THE AMERICAN NATIONAL STANDARDS INSTITUTE (ANSI) NATIONAL SANITATION FOUNDATION (NSF) INTERNATIONAL STANDARD 61.
 - ALL WATER MAINS AND THEIR ACCESSORIES SHALL BE INSTALLED AND PRESSURE AND LEAK TESTED IN ACCORDANCE WITH THE APPLICABLE PROVISIONS OF AWWA STANDARD C600-93, C602-89, C603-90, C605-94, OF C506-87.
 - THE CONTRACTOR SHALL GUARANTEE THE INSTALLATION FOR ONE YEAR FROM THE DATE OF SUBSTANTIAL COMPLETION.
 - ALL WORK TO CONFORM TO STATE AND LOCAL PLUMBING BACKFLOW PREVENTION CODES AND THE SPECIFICATIONS OF THE WELLS COUNTY WATER UTILITY. PER STATE CODE, BACKFLOW DEVICES ARE TO BE TESTED UPON INSTALLATION AND THEN PERIODICALLY THEREAFTER. SUBMIT COPIES OF TESTS TO THE UTILITY ENGINEERING DEPARTMENT.
 - VACUUM BREAKERS MUST BE INSTALLED ON ALL EXISTING OR PROPOSED HOSE BIBBS, MOP/SERVICE SINKS, WALL/YARD HYDRANTS.
 - ALL PIPE SHALL BE INSTALLED ACCORDING TO SPECIFICATIONS AND PIPE TRENCH DETAIL #1/C6.1.

Southern Wells Community Schools
Raiders Activity Center

03/05/2026
 5120 S 300 W
 P.O. BOX 146761
 BIDDING SET

03-05-2026

 No. 10911278
 STATE OF INDIANA
 EXPIRES 03/05/2028
Al D. D. D.

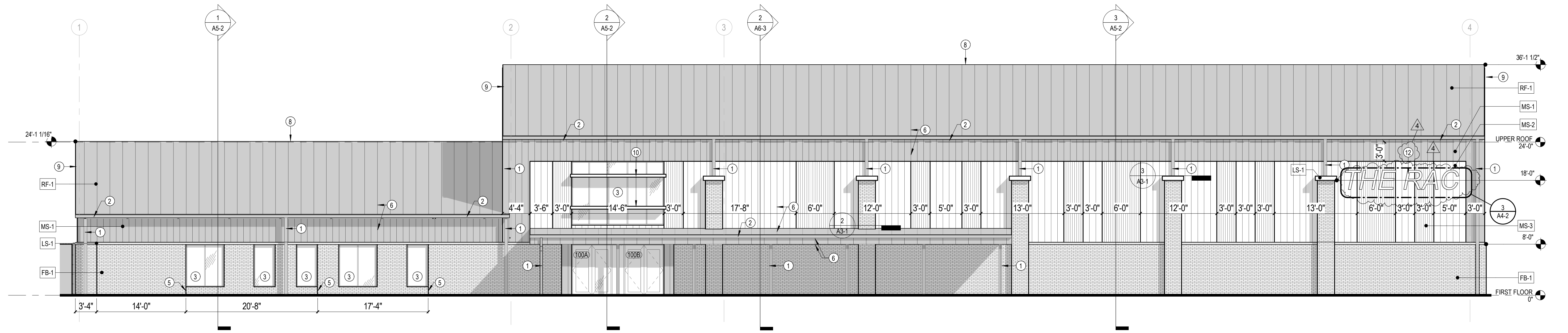
PERMIT SET
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 6898
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C4.0

krM Architecture+

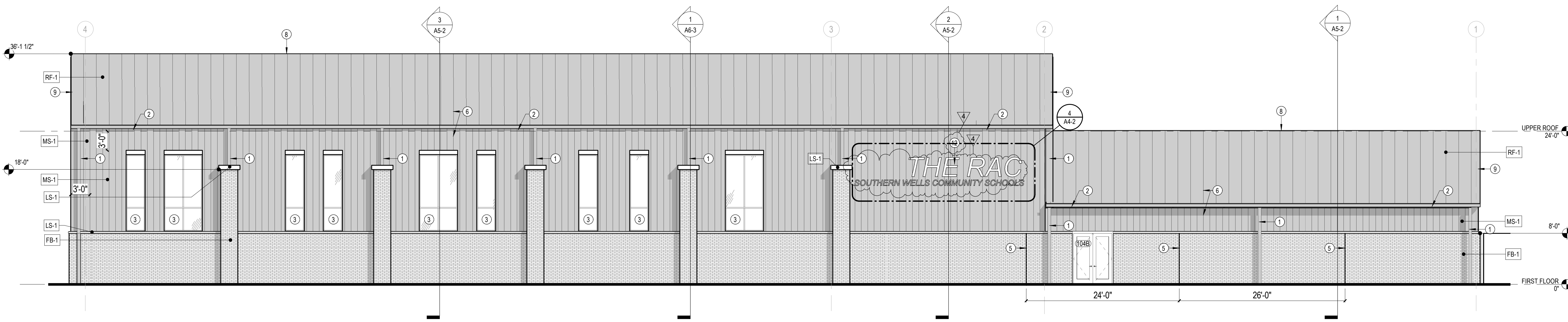
ENGINEERING RESOURCES, INC.
 4175 New Vision Drive, Fort Wayne, IN 46845
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REVISIONS

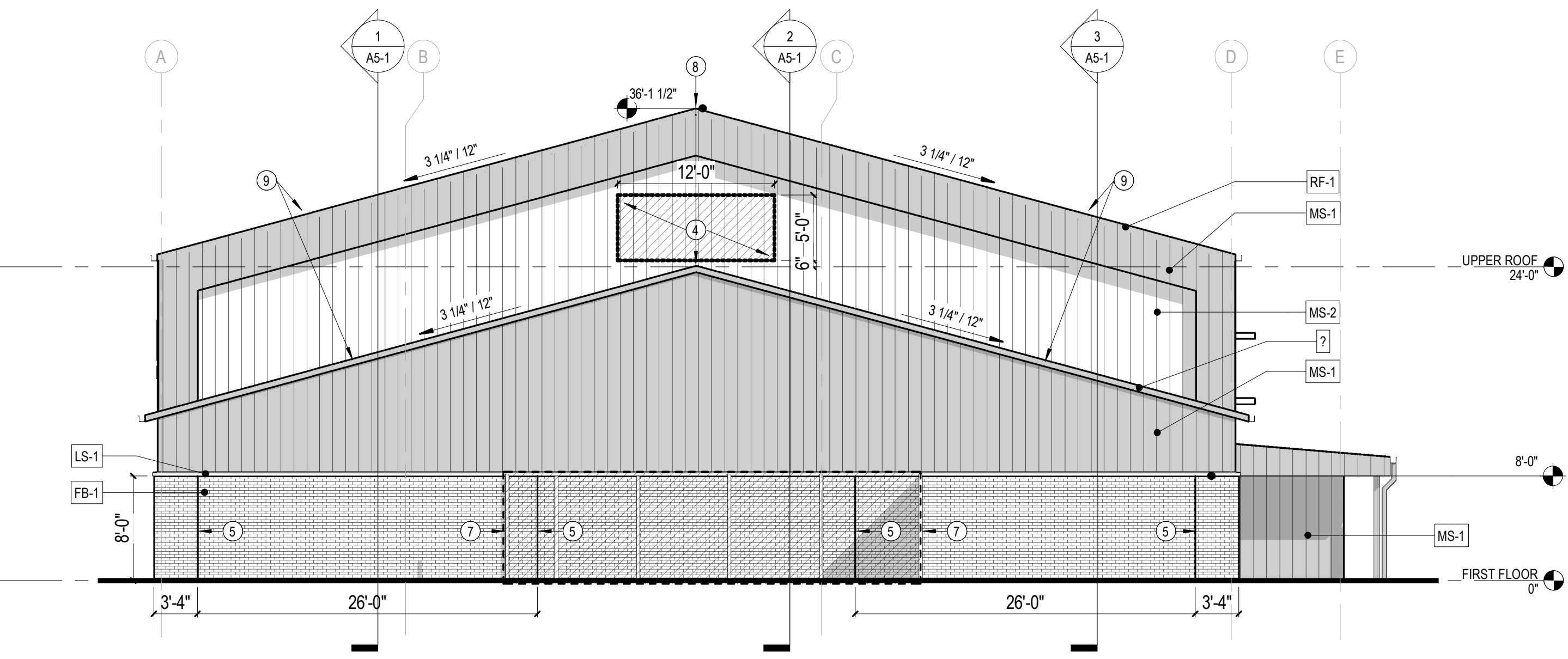
NO.	DATE	DESCRIPTION
1	03/27/2026	ADDENDUM #3
4	04/03/2026	ADDENDUM #4



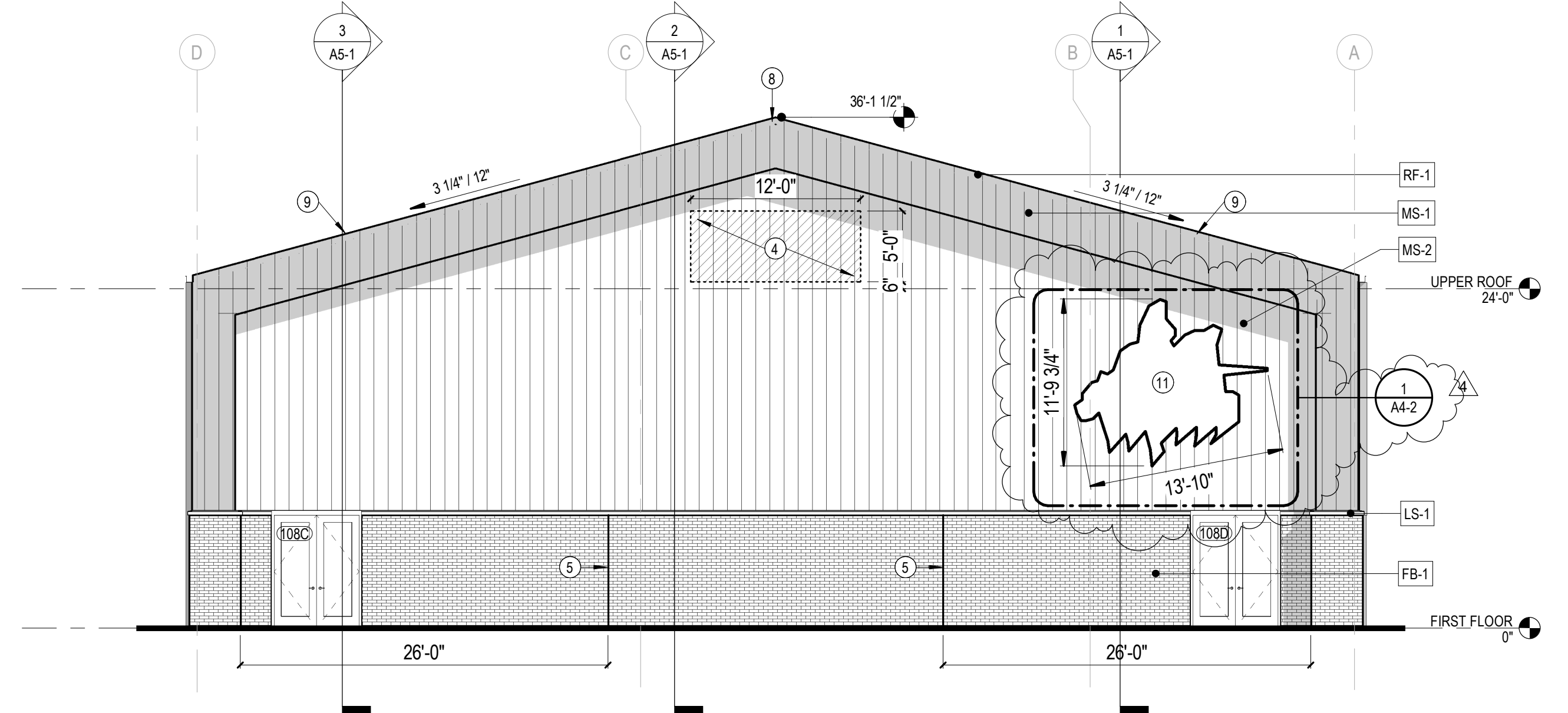
1 SOUTHWEST
SCALE: 1/8" = 1'-0"



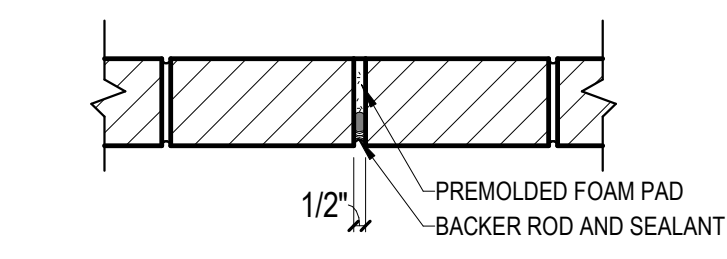
2 NORTHEAST
SCALE: 1/8" = 1'-0"



4 NORTHWEST
SCALE: 1/8" = 1'-0"



3 SOUTHEAST
SCALE: 1/8" = 1'-0"



5 BRICK EXPANSION JOINT TYP.
SCALE: NTS

ELEVATION NOTES - EXTERIOR

- 1 DOWNSPOUT INTO UNDERGROUND DRAINAGE SYSTEM. FINISH TO MATCH ADJACENT WALL PANEL.
- 2 RECTANGULAR GUTTER BY PEMB SUPPLIER. FINISH TO MATCH ADJACENT WALL PANEL.
- 3 STOREFRONT SYSTEM. REF. FRAME ELEVATIONS AND SECTIONS FOR DETAILS.
- 4 THIS AREA IS THE ONLY ACCEPTABLE LOCATION FOR MEP LOUVER PENETRATIONS.
- 5 BRICK EXPANSION JOINT.
- 6 24"W PANELS TO ALIGN WITH 12"W PANELS.
- 7 EQUIPMENT ENCLOSURE. REFERENCE DETAILS.
- 8 RIDGE VENT, CAP AND TRIM TO MATCH ROOF PANEL COLOR.
- 9 RAKE TRIM. COLOR TO MATCH ROOF PANEL COLOR.
- 10 ALUM. AIR/OIL SUNSHADE DEVICE. REF. FRAME ELEVATIONS. SECTIONS. SPECIFICATIONS FOR DETAILS.
- 11 CUSTOM 3D EXTERIOR GRADE SIGNAGE. INTERIOR LIT. SEE ENLARGED FOR DETAILS.
- 12 CUSTOM 3D EXTERIOR GRADE SIGNAGE. PROVIDE POWER FOR BACKLIT LIT. SEE ENLARGED FOR DETAILS.

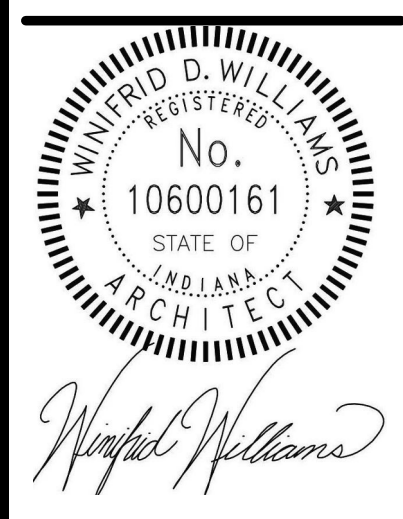
EXTERIOR MATERIAL FINISH SCHEDULE

MATERIAL TAG	MATERIAL TYPE	BASIS OF DESIGN MANUF.	COLOR / TYPE	ADDITIONAL REMARKS	SPEC #
FB-1	BRICK VENEER	GLEN-GERY - SIOUX CITY BRICK	BADLANDS SMOOTH MODULAR	MATCH EXISTING BRICK ON CAMPUS	
LS-1	LIMESTONE CAP		BUFF FINISH, SEALANTS TO MATCH		
MS-1	METAL SIDING	McELROY METAL	FW PANEL; 12" FLUSH / CHARCOAL	ARCHITECT TO SELECT COLOR FROM MANUFACTURERS FULL RANGE	
MS-2	METAL SIDING	McELROY METAL	FW PANEL; 12" FLUSH / WHITE	ARCHITECT TO SELECT COLOR FROM MANUFACTURERS FULL RANGE	
MS-3	METAL SIDING	McELROY METAL	WAVE PANEL / WHITE	ARCHITECT TO SELECT COLOR FROM MANUFACTURERS FULL RANGE	
RF-1	STANDING SEAM METAL ROOF SYSTEM	McELROY METAL	238T; 24"W PLANK / CHARCOAL	ARCHITECT TO SELECT COLOR FROM MANUFACTURERS FULL RANGE	
SF-1	STOREFRONT SYSTEM			REF. FRAME ELEVATIONS	



REVISIONS
4 10/2/2026 [redacted]

3/12/2026
SOUTHERN WELLS COMMUNITY SCHOOLS
25048 - RAIDERS ACTIVITY CENTER
9120 S 300 W
PONECO, IN 46761
BID DOCUMENTS



BID DOCUMENTS
3/12/2026
M/J JOB NO.
25048
DRAWN BY
EW
DRAWING NAME
EXTERIOR
ELEVATIONS
DRAWING NO.
A4-1

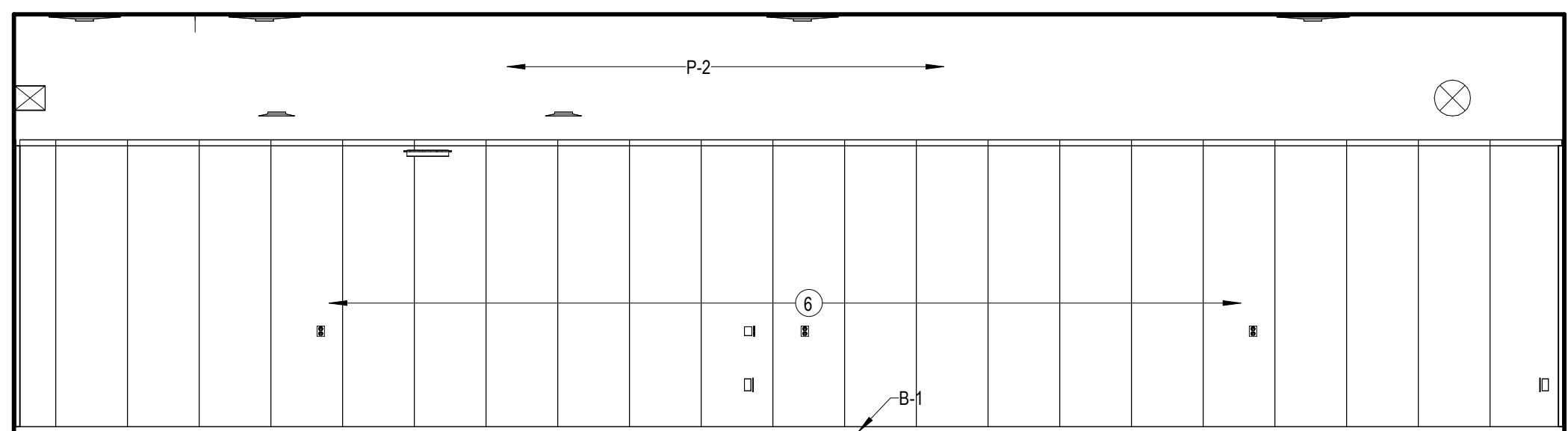
ELEVATION NOTES - INTERIOR

- 1 ACRYLIC SIGNAGE, CUSTOM COLOR, SEE SPECIFICATION
- 2 DRYWALL CONTROL JOINT
- 3 6'-0" FRAMELESS MIRROR, MOUNT 1'-4" AFF.
- 4 FINISHED END
- 5 SIDESPLASH
- 6 8'-0" ATHLETIC WALL PADDING, MOUNT 4" AFF. COORDINATE LOCATION OF ALL DEVICES PRIOR TO CUTTING PADS.
- 7 ACOUSTIC WALL PANELS
- 8 8'-0" ATHLETIC WALL PADDING WITH CUSTOM LOGO, MOUNT 4" AFF.
- 9 ALIGN CONTROL JOINT WITH DOOR.
- 10 FIRE EXTINGUISHER CABINET, COORDINATE SIZE AND LOCATION PRIOR TO CUTTING ATHLETIC WALL PADDING.
- 11 DUCTWORK, REFER TO MEP
- 12 P-5 FINISH TO 8'-4" AFF. MATCH COLOR OF WALL PADDING. FLAG BY OWNER.
- 13 8'-0" ATHLETIC WALL PADDING, MOUNT 4" AFF. PADS TO MATCH P-4. PROVIDE RETURNS TO WALL.
- 14 ALL CASEWORK TO BE LOCKING I.U.O.
- 15 BUILDING DEDICATION PLAQUE, REFER TO SPECS.
- 16 DRYWALL CONTROL JOINT, ALIGN WITH CEILING CONTROL JOINT.
- 17 RETURN GRILLE, COORDINATE SIZE AND LOCATION PRIOR TO CUTTING ATHLETIC WALL PADDING, REFER TO MEP.
- 18 PROVIDE ATHLETIC WALL PADDING TO COVER SCHEDULED DOOR - AWP-1

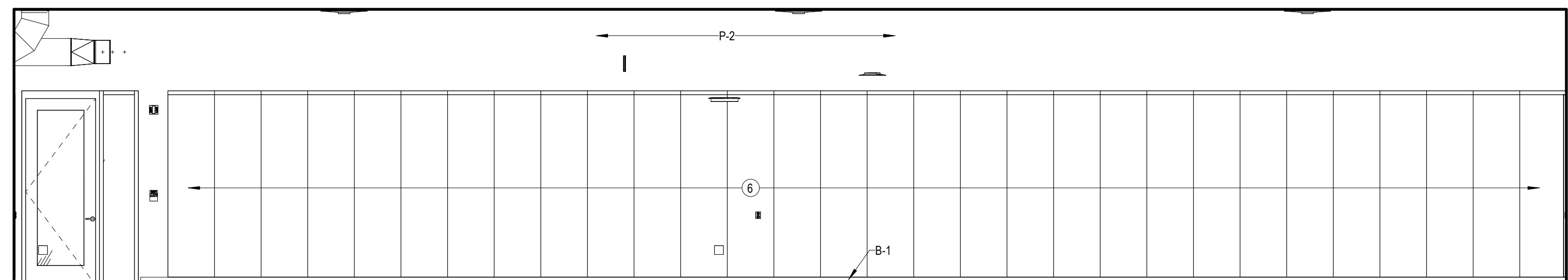


REVISIONS

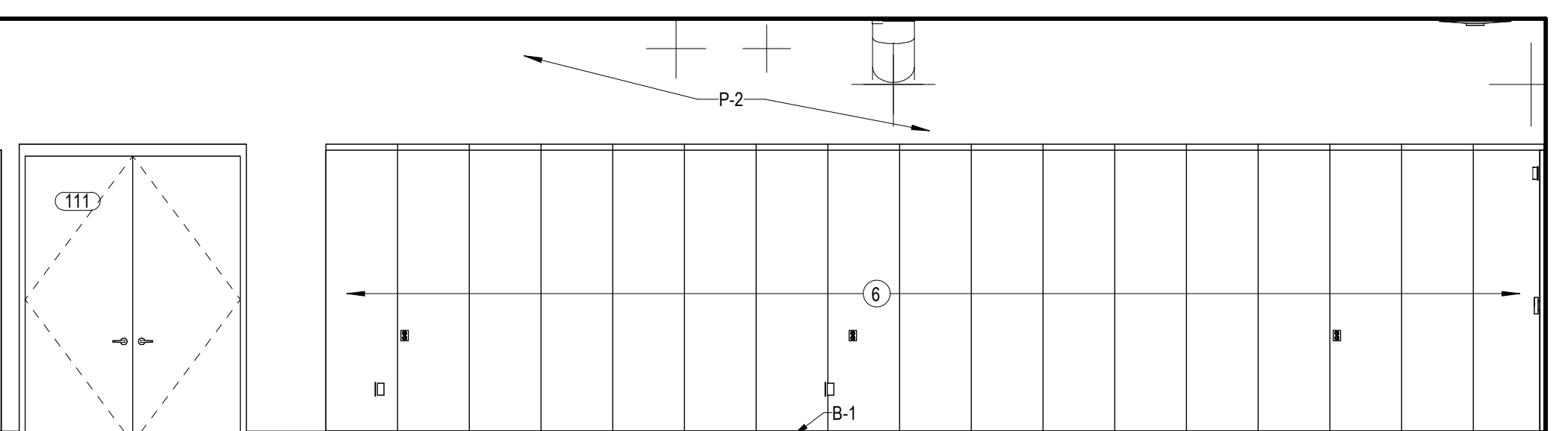
1	10/2/2026	Admitted
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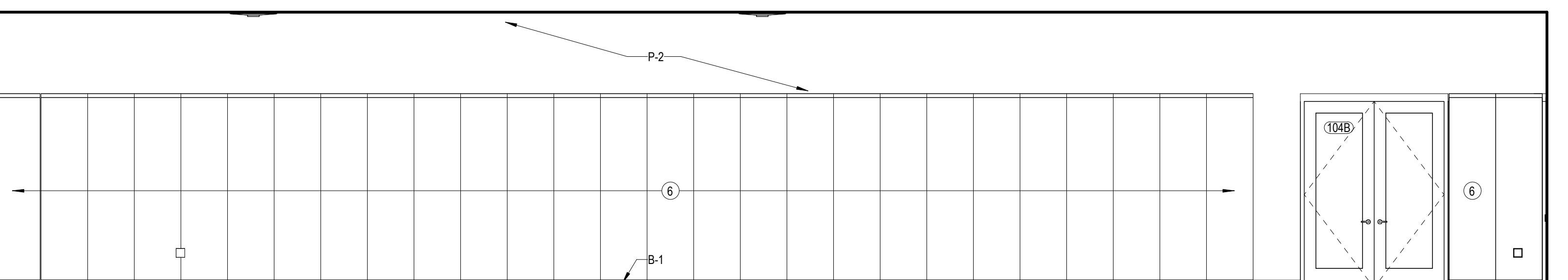
15 WRESTLING 104 - WEST
SCALE: 1/4" = 1'-0"



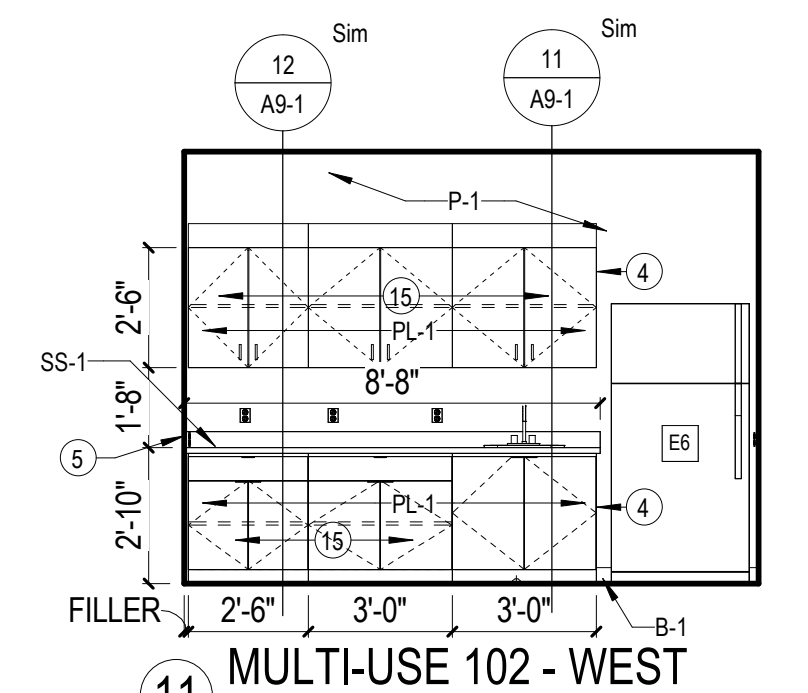
14 WRESTLING 104 - SOUTH
SCALE: 1/4" = 1'-0"



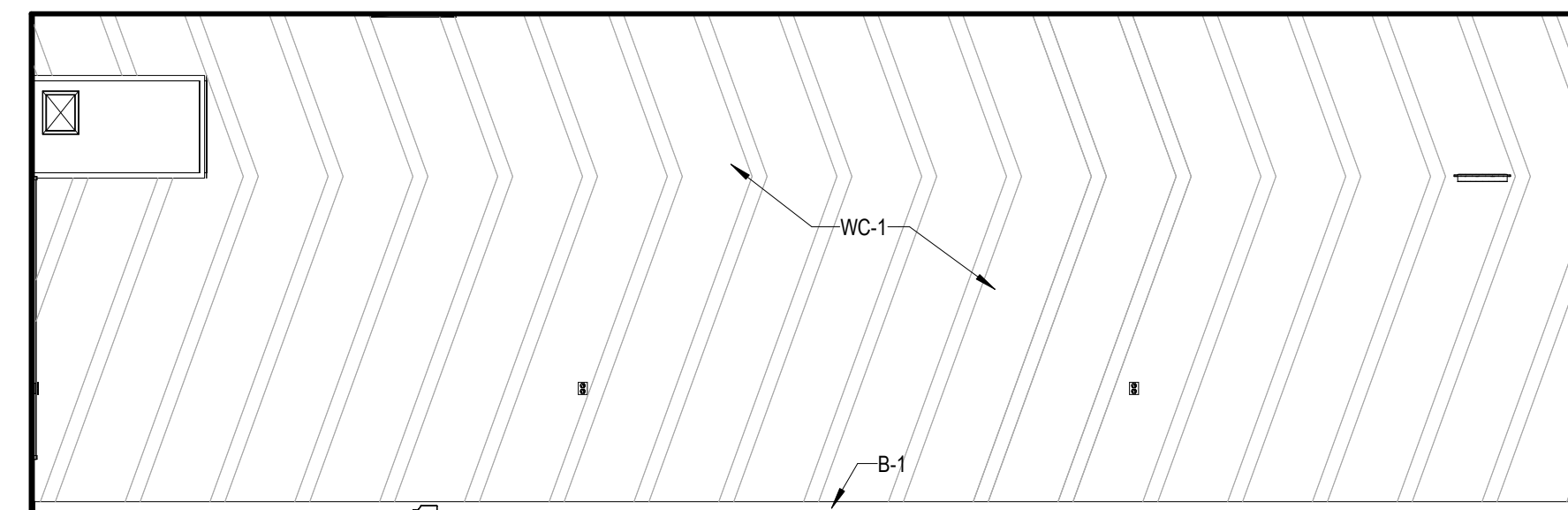
13 WRESTLING 104 - EAST
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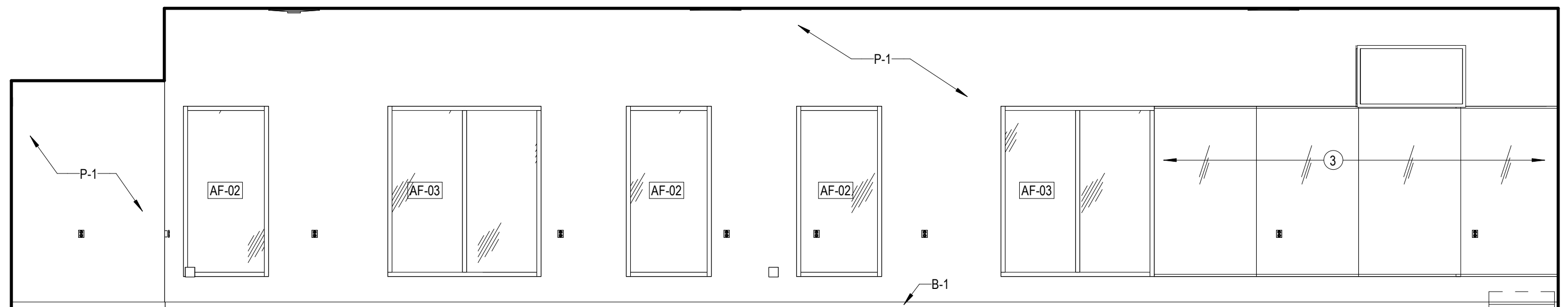
12 WRESTLING 104 - NORTH
SCALE: 1/4" = 1'-0"



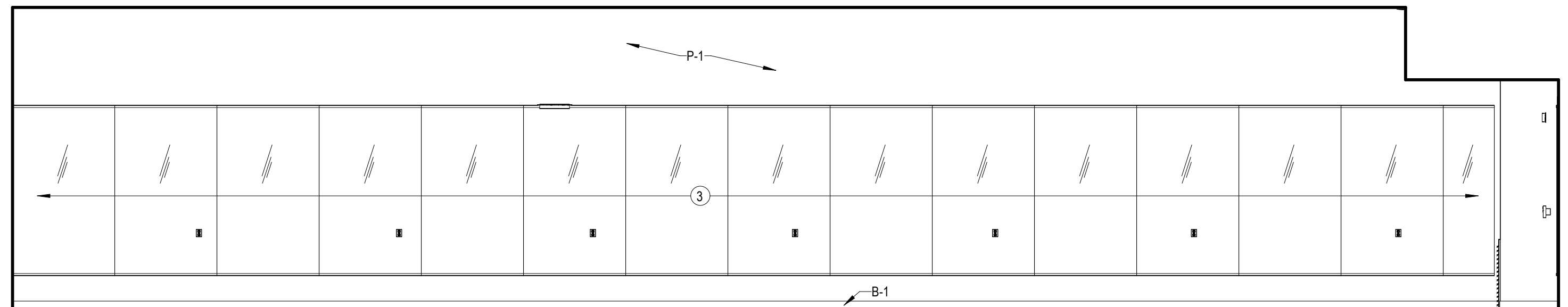
11 MULTI-USE 102 - WEST
SCALE: 1/4" = 1'-0"



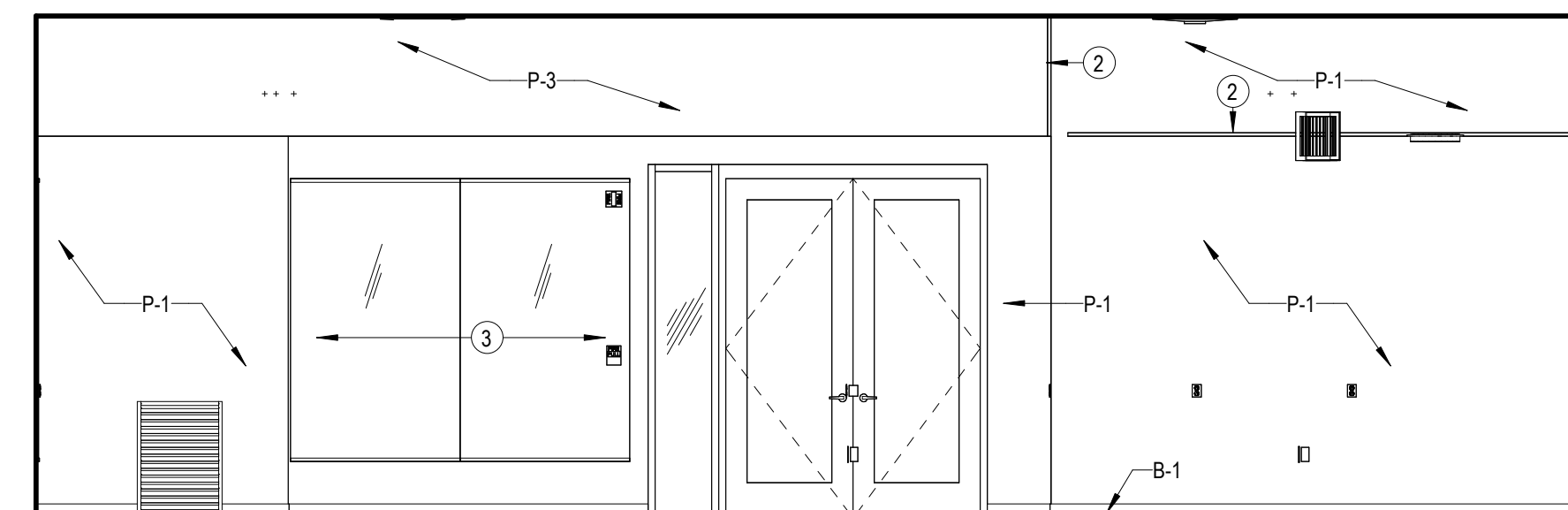
10 WEIGHT 103 - WEST
SCALE: 1/4" = 1'-0"



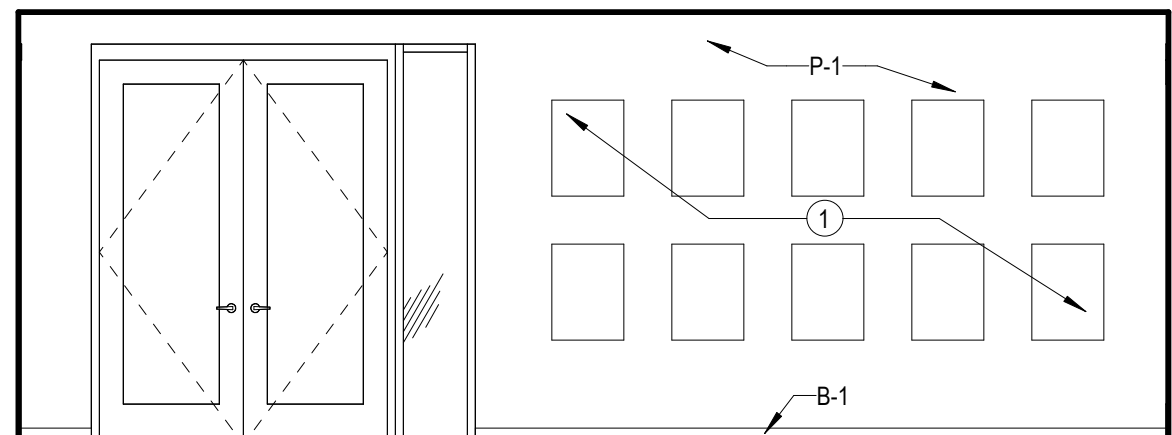
9 WEIGHT 103 - SOUTH
SCALE: 1/4" = 1'-0"



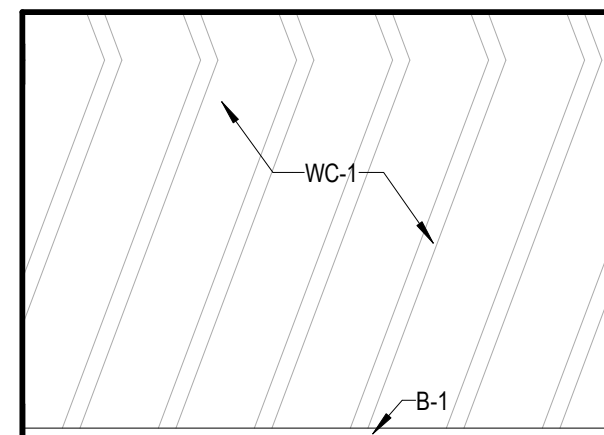
8 WEIGHT 103 - NORTH
SCALE: 1/4" = 1'-0"



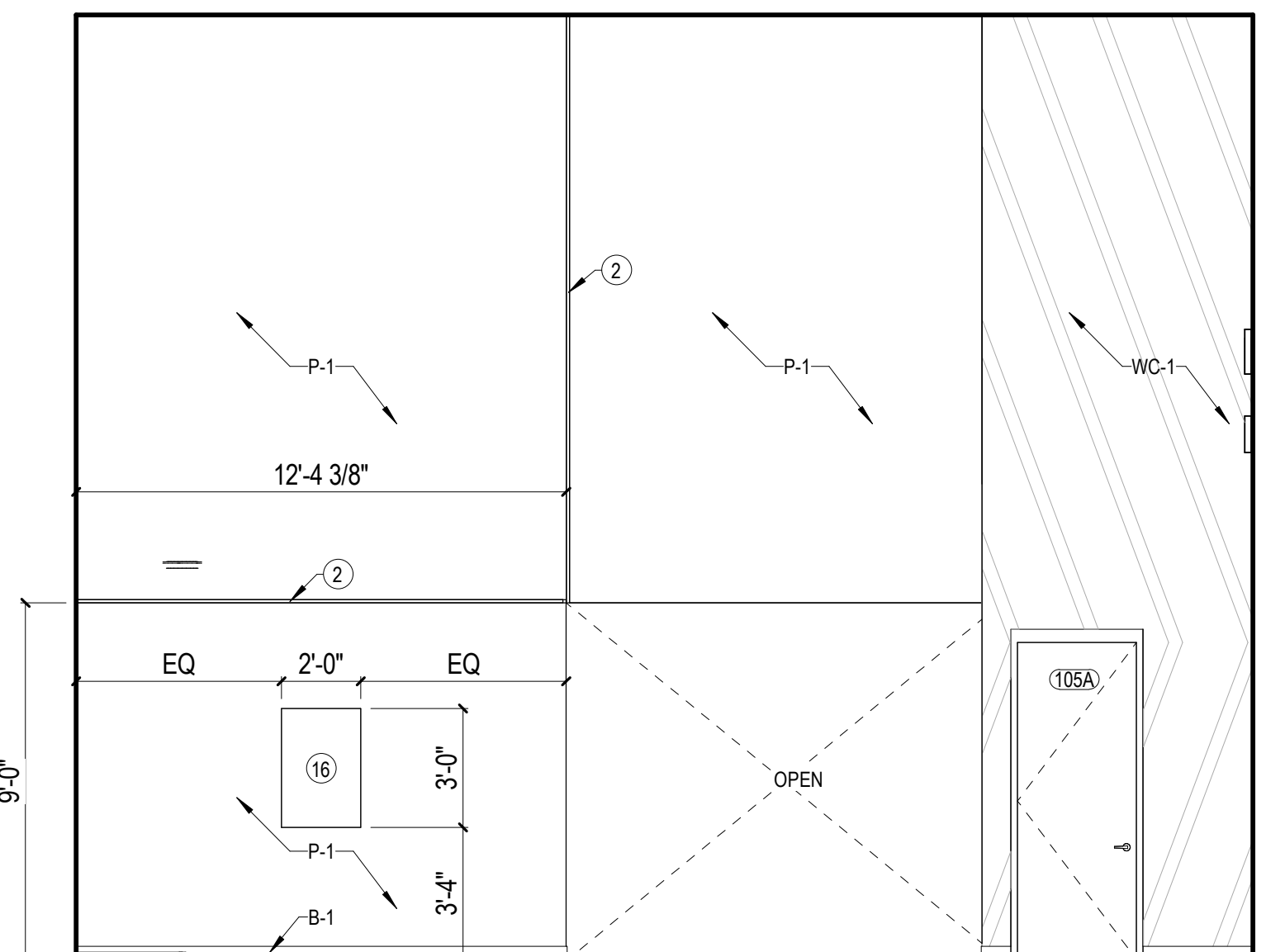
7 WEIGHT 103 - EAST
SCALE: 1/4" = 1'-0"



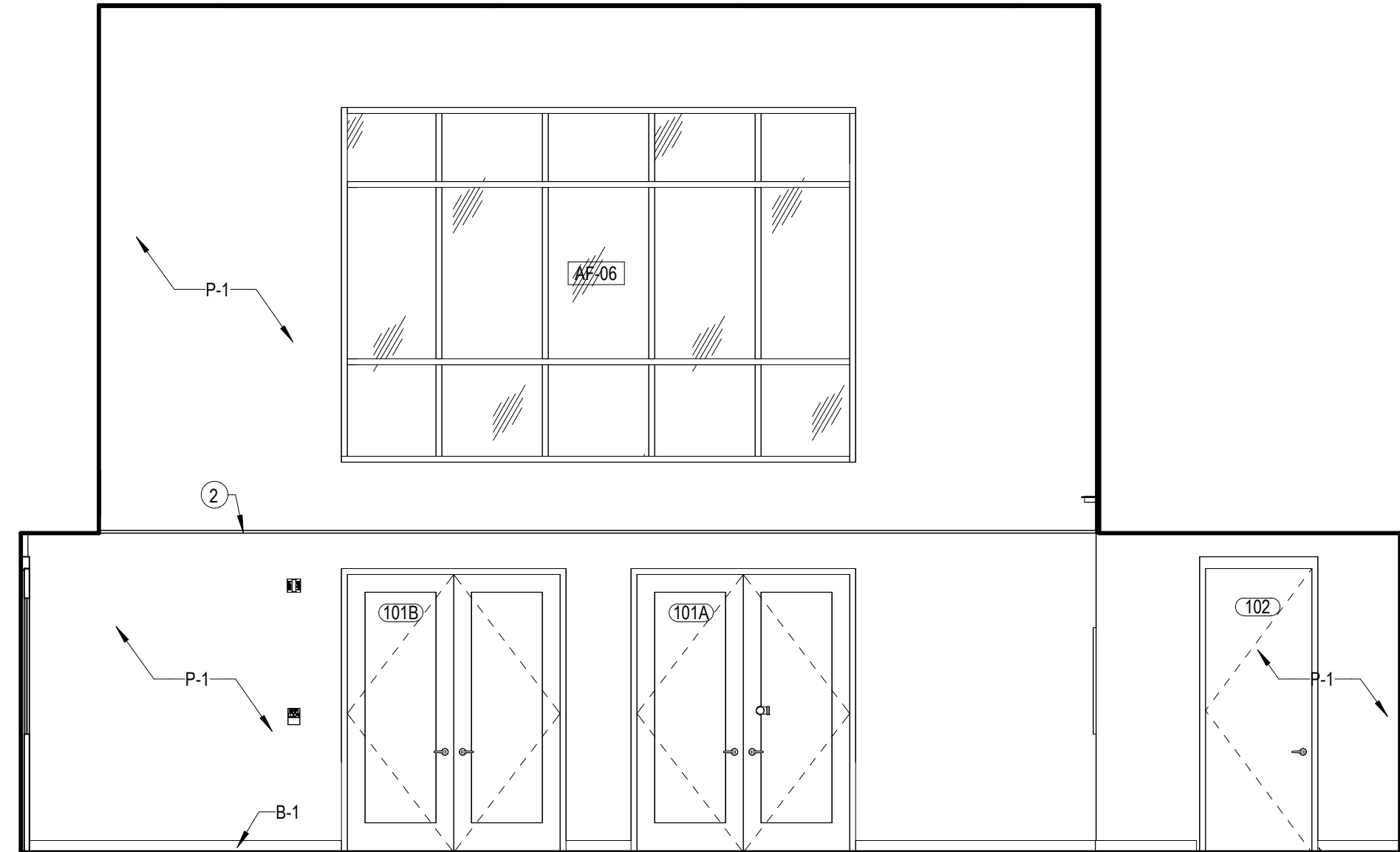
6 HALL 101 - WEST
SCALE: 1/4" = 1'-0"



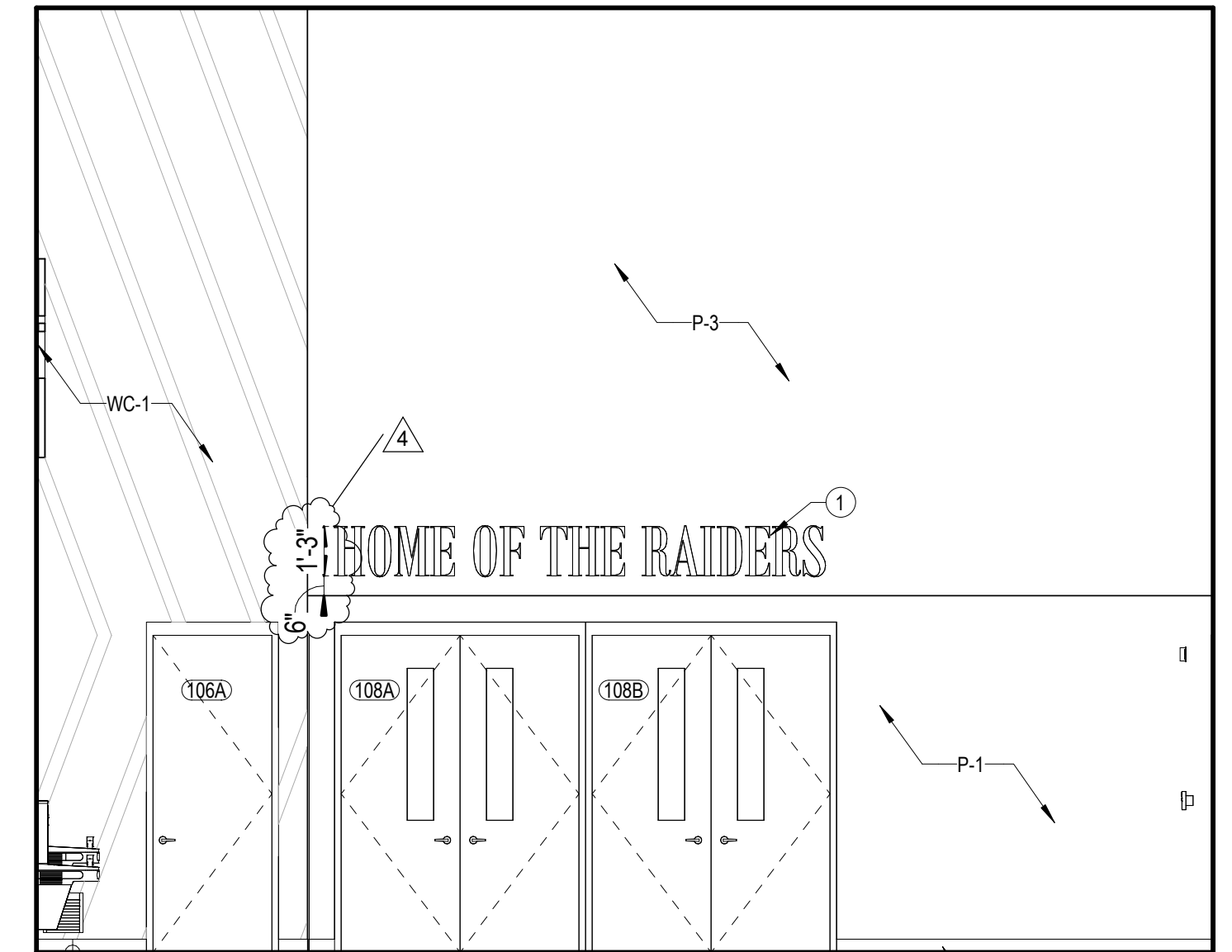
5 HALL 101 - EAST
SCALE: 1/4" = 1'-0"



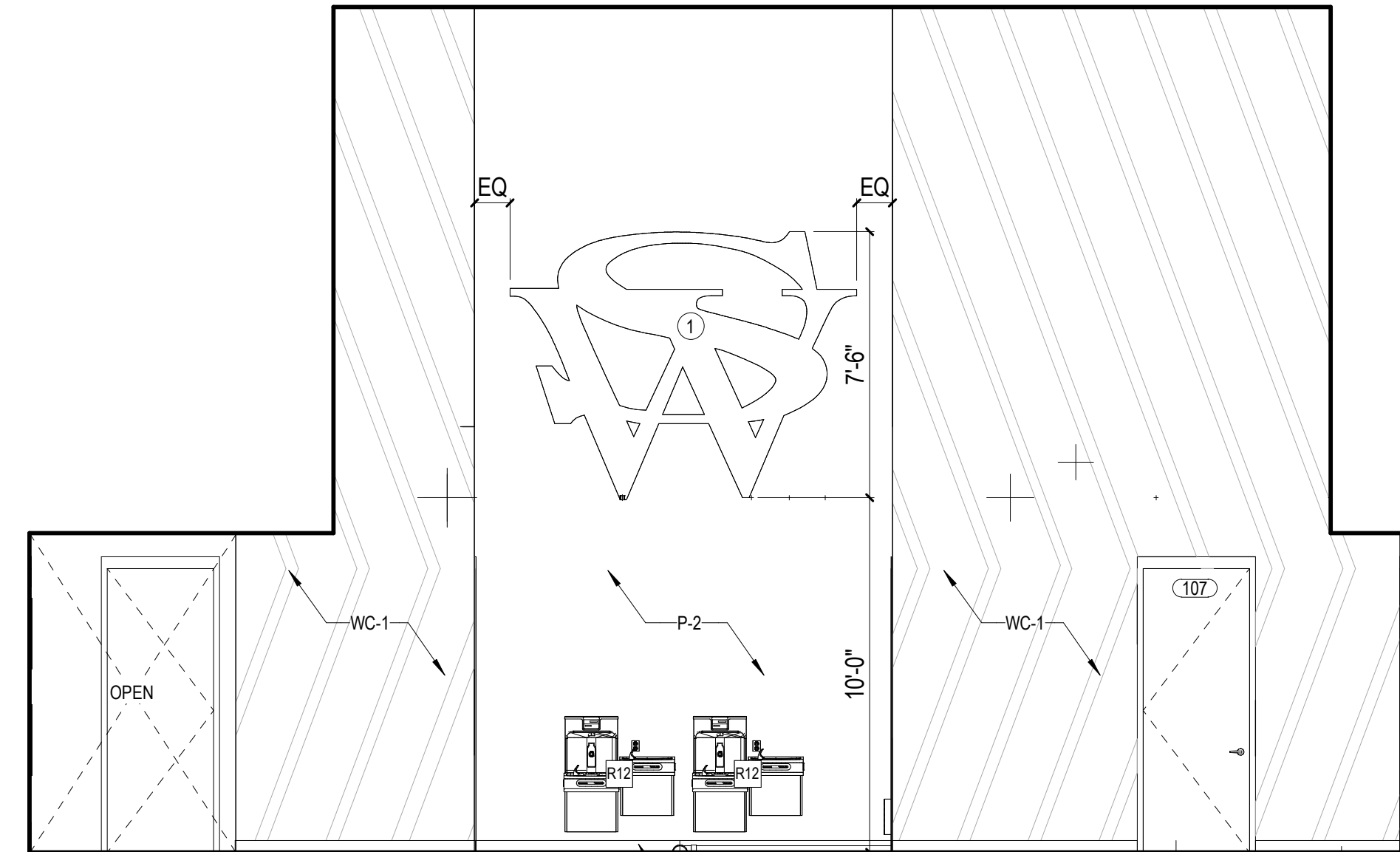
4 CIRCULATION 101 - WEST
SCALE: 1/4" = 1'-0"



3 CIRCULATION 101 - SOUTH
SCALE: 1/4" = 1'-0"



2 CIRCULATION 101 - EAST
SCALE: 1/4" = 1'-0"



1 CIRCULATION 101 - NORTH
SCALE: 1/4" = 1'-0"

EQUIPMENT SCHEDULE

MARK	DESCRIPTION	FURNISHED BY	INSTALLED BY
E1	BASKETBALL HOOP FORWARD FOLDING BACKSTOP	CONTR.	CONTR.
E2	MOTORIZED BATTING CAGE	CONTR.	CONTR.
E3	SCOREBOARD	CONTR.	CONTR.
E4	AUTOMATIC TELESCOPING WOODEN BLEACHERS, REAR FOLDING	CONTR.	CONTR.
E5	MOTORIZED ROLLING DIVIDER WALL	CONTR.	CONTR.
E6	REFRIGERATOR	CONTR.	CONTR.
E7	4' W X 4' H WHITEBOARD	CONTR.	CONTR.
E8	8' W X 4' H WHITEBOARD	CONTR.	CONTR.

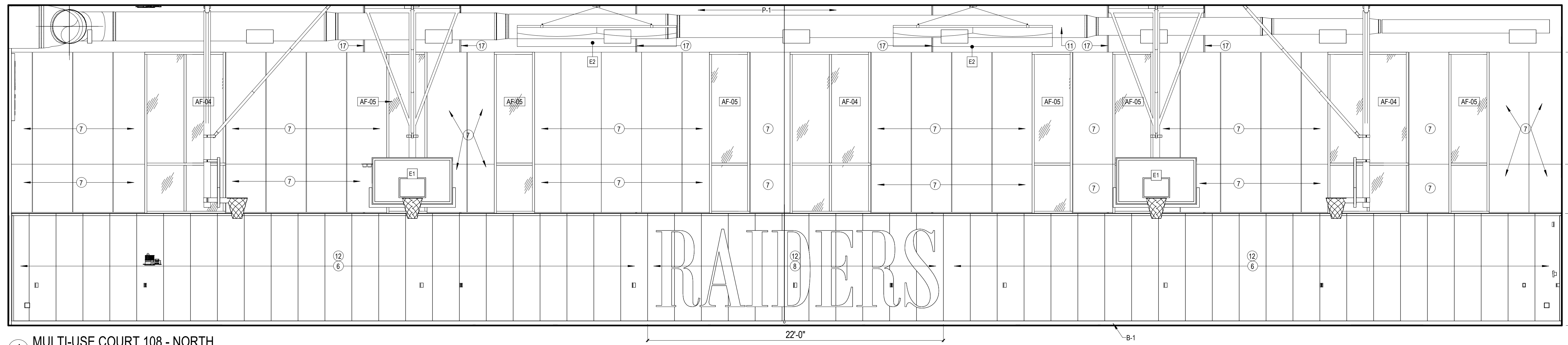


Matthew Williams

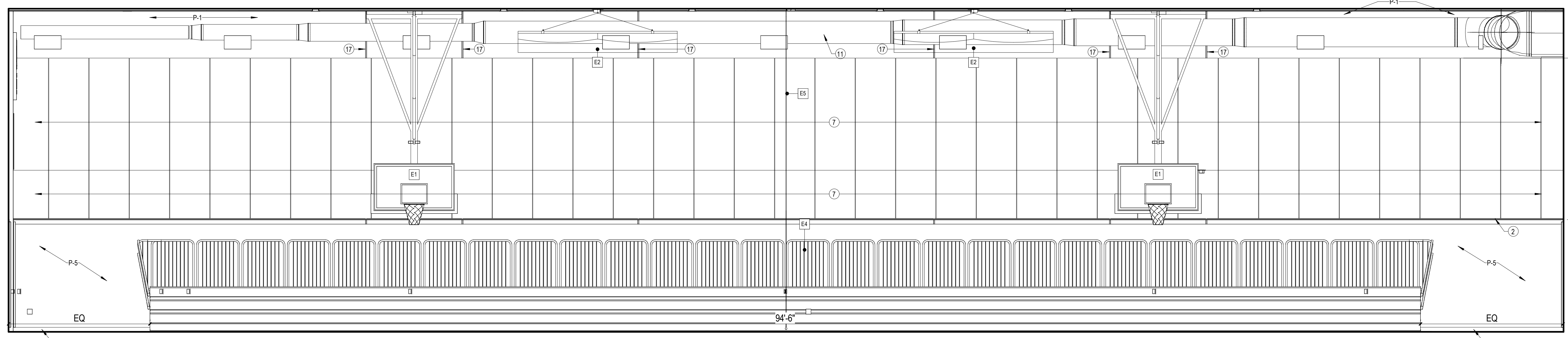
BID DOCUMENTS
3/12/2026
M/J JOB NO.
25048
DRAWN BY
MK

DRAWING NAME
INTERIOR
ELEVATIONS

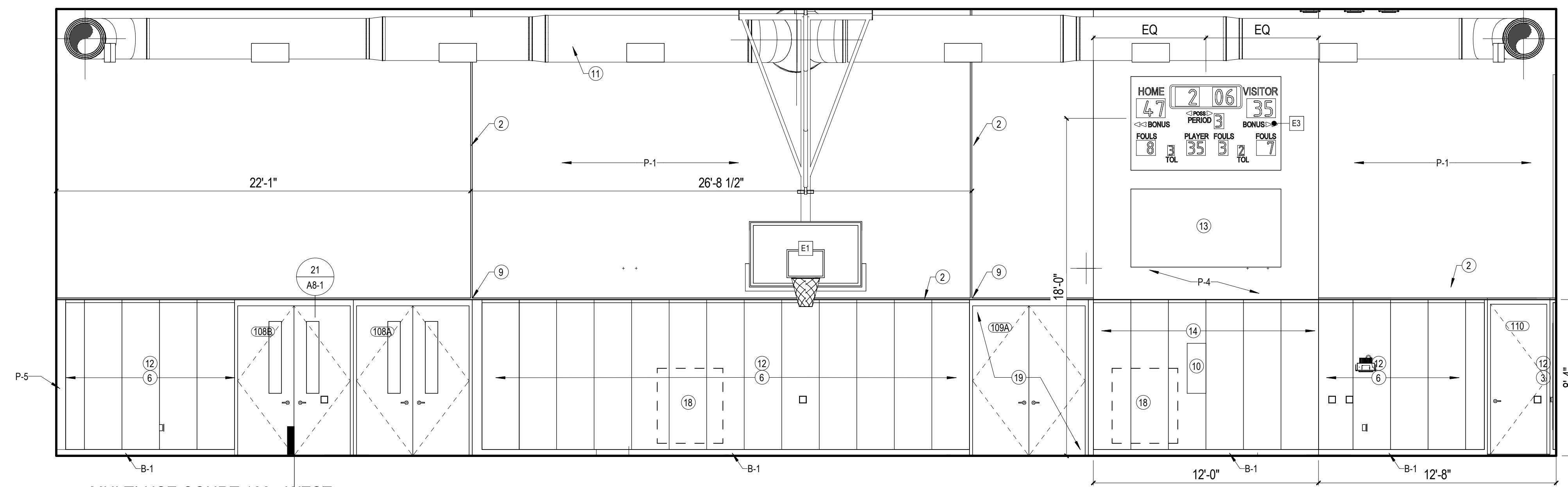
DRAWING NO.
A10-2



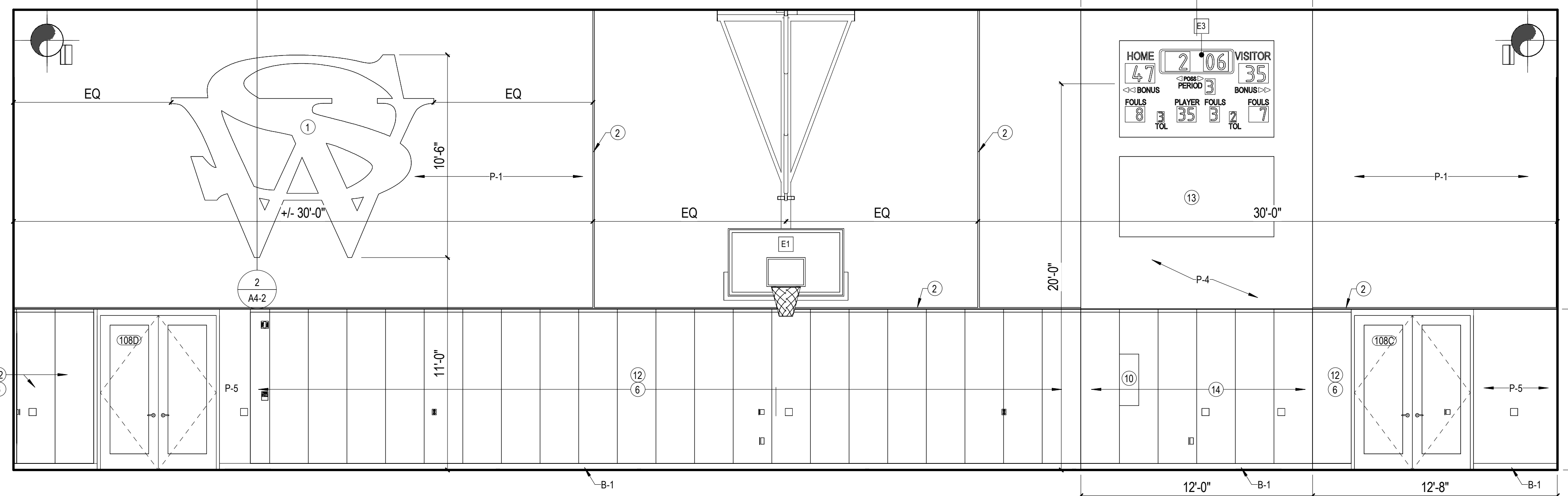
4 MULTI-USE COURT 108 - NORTH
SCALE: 1/4" = 1'-0"



3 MULTI-USE COURT 108 - SOUTH
SCALE: 1/4" = 1'-0"



2 MULTI-USE COURT 108 - WEST
SCALE: 1/4" = 1'-0"



1 MULTI-USE COURT 108 - EAST
SCALE: 1/4" = 1'-0"

ELEVATION NOTES - INTERIOR

- 1 ACRYLIC SIGNAGE, CUSTOM COLOR, SEE SPECIFICATION
- 2 DRYWALL CONTROL JOINT
- 3 6'-8" FRAMELESS MIRROR, MOUNT 1'-4" AFF.
- 4 FINISHED END
- 5 SIDESPLASH
- 6 8'-0" ATHLETIC WALL PADDING, MOUNT 4" AFF. COORDINATE LOCATION OF ALL DEVICES PRIOR TO CUTTING PADS
- 7 ACOUSTIC WALL PANELS
- 8 8'-0" ATHLETIC WALL PADDING WITH CUSTOM LOGO, MOUNT 4" AFF.
- 9 ALIGN CONTROL JOINT WITH DOOR
- 10 FIRE EXTINGUISHER CABINET, COORDINATE SIZE AND LOCATION PRIOR TO CUTTING ATHLETIC WALL PADDING
- 11 DUCTWORK, REFER TO MEP
- 12 P-5 FINISH TO 8'-4" AFF. MATCH COLOR OF WALL PADDING. FLAG BY OWNER
- 13 8'-0" ATHLETIC WALL PADDING, MOUNT 4" AFF. PADS TO MATCH P-4. PROVIDE RETURNS TO WALL
- 14 ALL CASEWORK TO BE LOOKING U.N.O.
- 15 BUILDING DEDICATION PLAQUE, REFER TO SPECS.
- 16 DRYWALL CONTROL JOINT, ALIGN WITH CEILING CONTROL JOINT
- 17 RETURN GRILLE, COORDINATE SIZE AND LOCATION PRIOR TO CUTTING ATHLETIC WALL PADDING. REFER TO MEP.
- 18 PROVIDE ATHLETIC WALL PADDING TO COVER SCHEDULED DOOR - AWP-1



REVISIONS
4 10/2/2026
Addendum 1

3/12/2026
SOUTHERN WELLS COMMUNITY SCHOOLS
25048 - RAIDERS ACTIVITY CENTER
9120 S 300 W
PONETO, IN 46781
BID DOCUMENTS

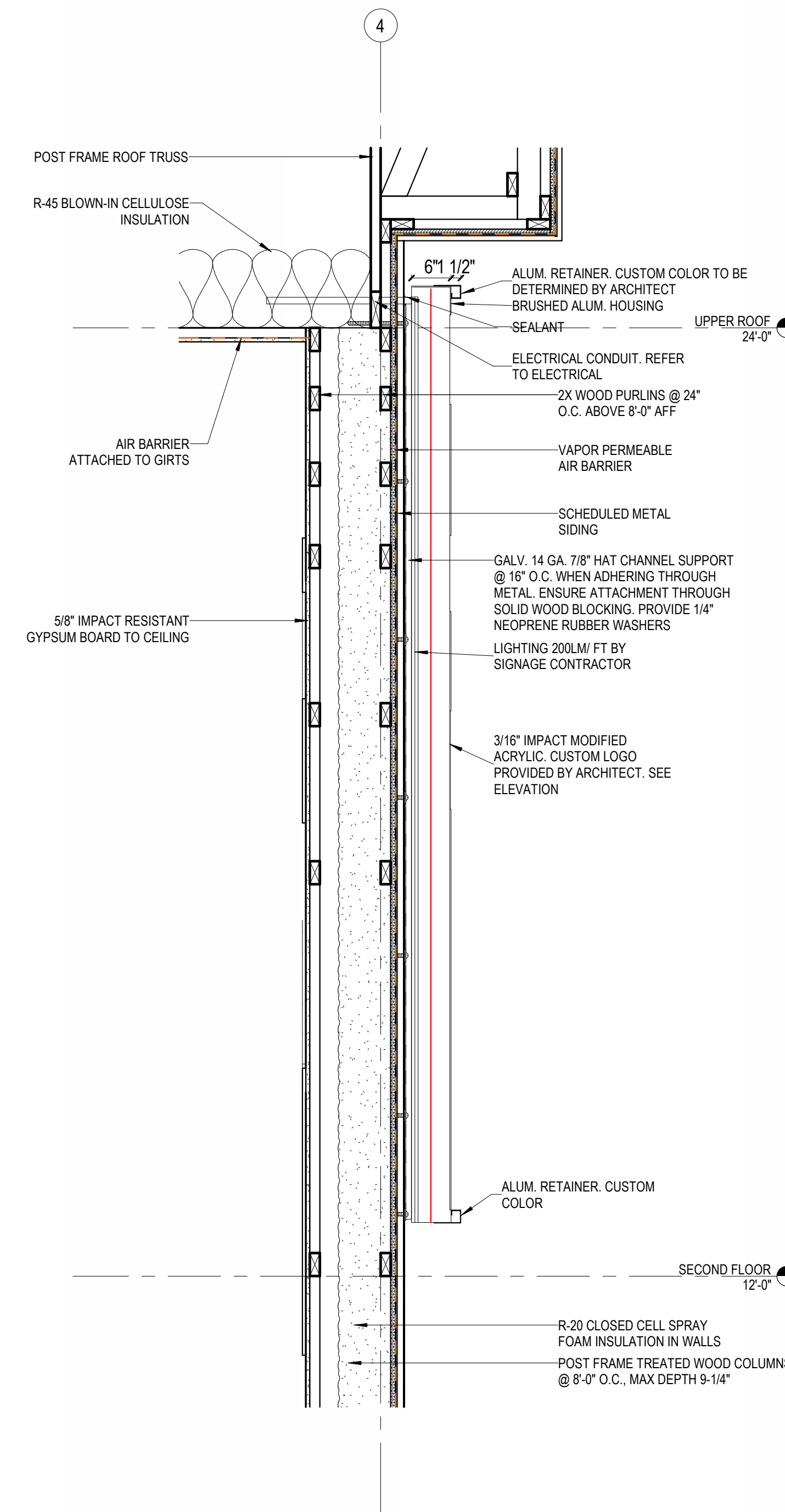


BID DOCUMENTS
3/12/2026
M/J JOB NO.
25048
DRAWN BY
MK
DRAWING NAME
INTERIOR
ELEVATIONS

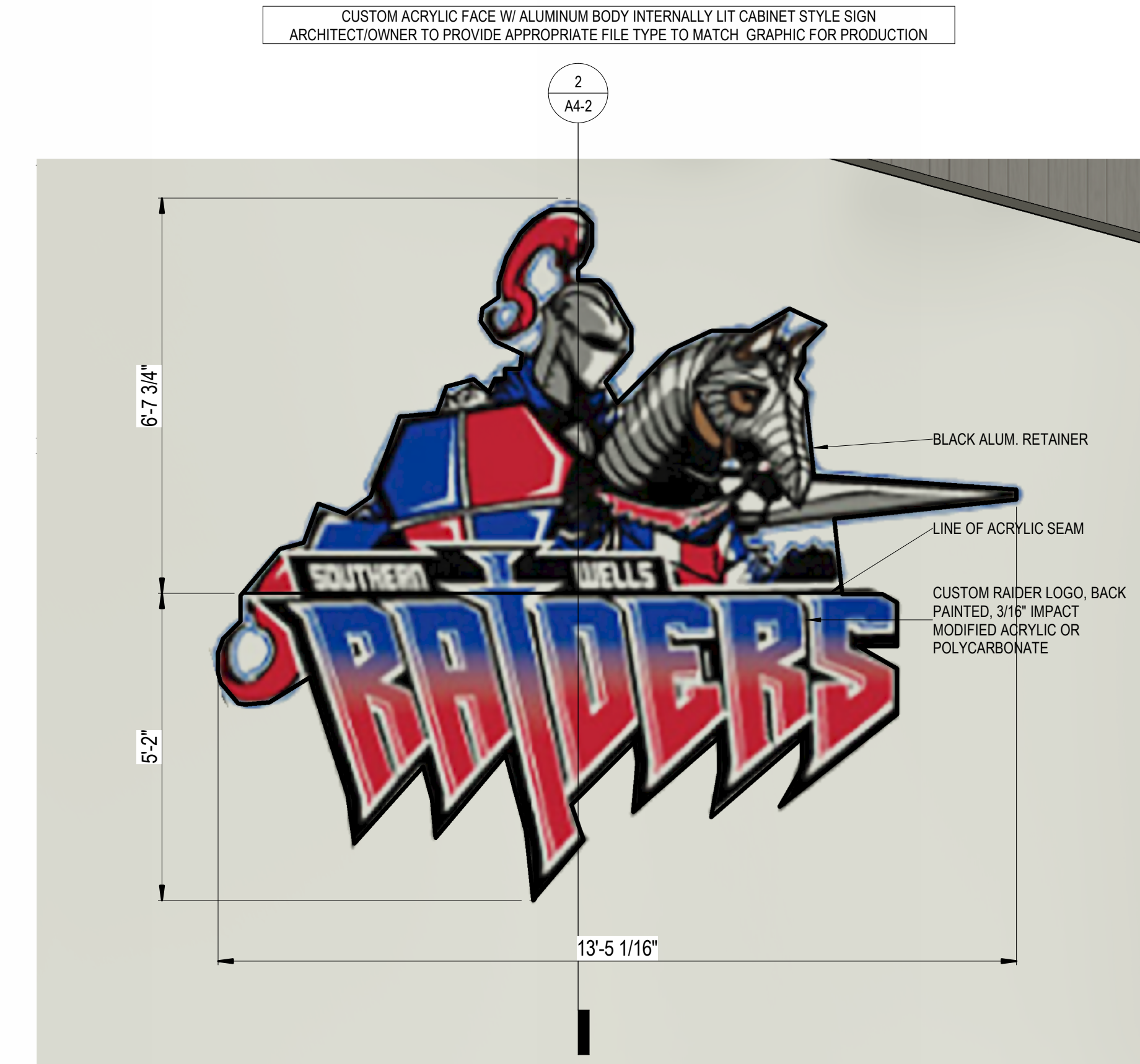
DRAWING NO.
A10-3

EQUIPMENT SCHEDULE

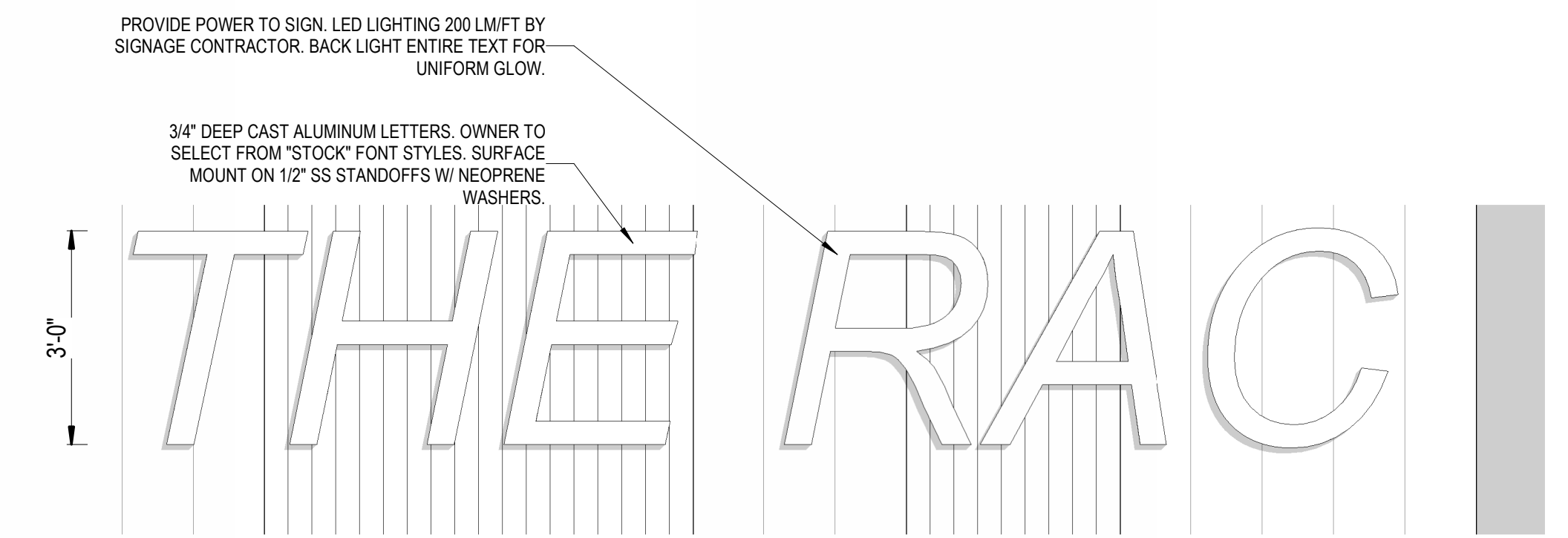
MARK	DESCRIPTION	FURNISHED BY	INSTALLED BY
E1	BASKETBALL HOOP FORWARD FOLDING BACKSTOP	CONTR.	CONTR.
E2	MOTORIZED BATTING CAGE	CONTR.	CONTR.
E3	SCOREBOARD	CONTR.	CONTR.
E4	AUTOMATIC TELESCOPING WOODEN BENCHES, REAR FOLDING	CONTR.	CONTR.
E5	MOTORIZED ROLLING DIVIDER WALL	CONTR.	CONTR.
E6	REFRIGERATOR	CONTR.	CONTR.
E7	4' W X 4' H WHITEBOARD	CONTR.	CONTR.
E8	8' W X 4' H WHITEBOARD	CONTR.	CONTR.



2 CUSTOM RAIDER SIGN SECTION
SCALE: 3/4" = 1'-0"

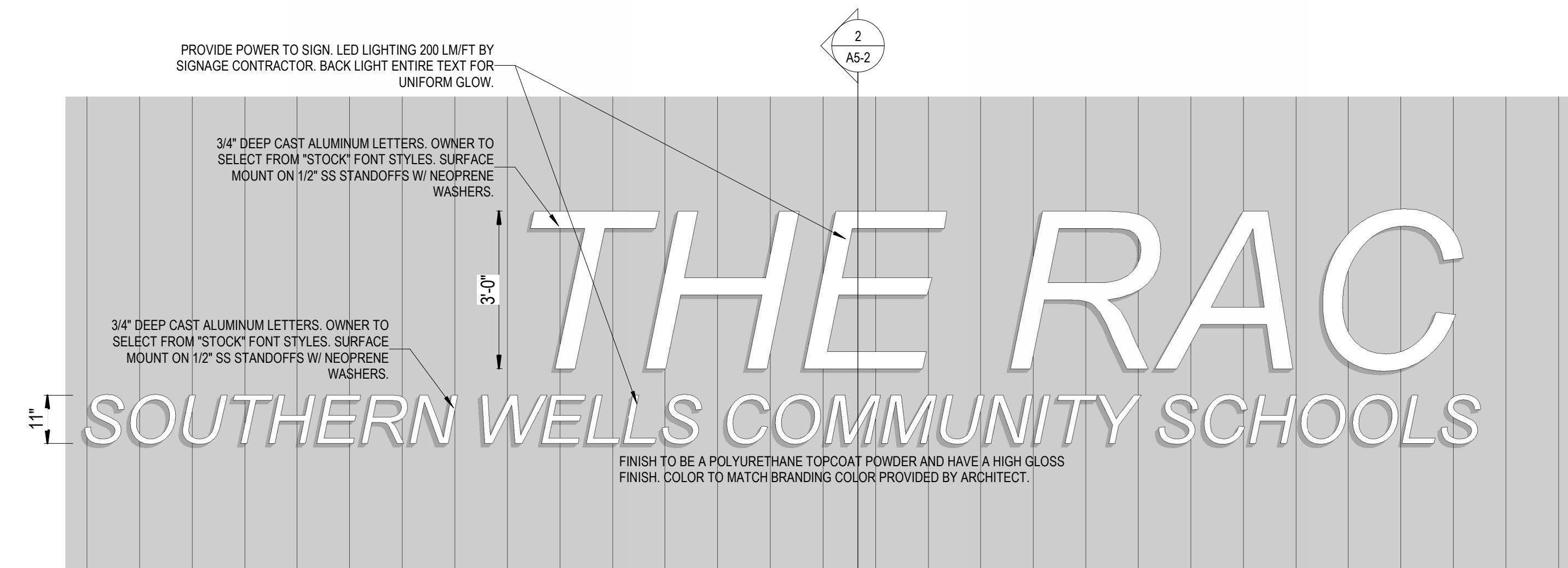


1 CUSTOM RAIDER SIGNAGE
SCALE: 1/2" = 1'-0"



3 EXTERIOR LETTER SIGNAGE 01
SCALE: 1/2" = 1'-0"

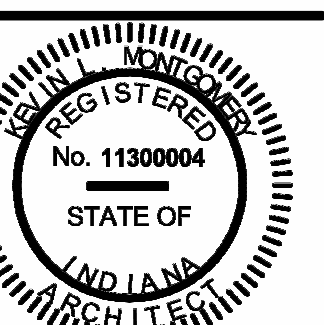
FINISH TO BE A POLYURETHANE TOPCOAT POWDER AND HAVE A HIGH GLOSS FINISH. COLOR TO MATCH BRANDING COLOR PROVIDED BY ARCHITECT.



4 EXTERIOR LETTER SIGNAGE 02
SCALE: 1/2" = 1'-0"

NO.	DATE	DESCRIPTION
1	4/23/2026	ADDendum 1

3/12/2026
SOUTHERN WELLS COMMUNITY SCHOOLS
25048 - RAIDERS ACTIVITY CENTER
9/20 S 300 W
PONECO, IN 46061
BID DOCUMENTS



Kevin Montgomery

BID DOCUMENTS
3/12/2026
K/M JOB NO.
25048
DRAWN BY
Author

DRAWING NAME
EXTERIOR SIGNAGE
AND DETAILS

DRAWING NO.
A4-2